

SURVEY/LAYOUT INFORMATION:

COORDINATE SYSTEM: MN COUNTY COORDINATE SYSTEM DATUM: ST. LOUIS TRANVERS MERCATOR 96 GEOID: G12B

USGS BENCHMARK HELD: AA9913



GENERAL PROJECT NOTES:

EXISTING CONDITIONS LEGEND:

 \Diamond O

ELECTRIC BOX

CATCH BASIN

ANCHOR

BENCH

SIGN

FLAG POLE

GATE POST

LARGE CLEAT

SMALL CLEAT

EXISTING BUILDING

EXISTING STORM EXISTING SANITARY

EXISTING WATERMAIN EXISTING OVERHEAD

EXISTING FIBEROPTIC EXISTING ELECTRIC

EXISTING MAJOR CONTOUR EXISTING MINOR CONTOUR

EXISTING GAS

FIRE HYDRANT

ELECTRICAL BOX

DECIDUOUS TREE/BUSH

CONIFEROUS TREE/BUSH

STORM MANHOLE

LIGHT POLE AND BASE

- COMPENSATION WILL BE ALLOWED FOR ITEMS THAT COULD HAVE BEEN IDENTIFIED BY A SITE VISIT, STUDYING THE TOPOGRAPHIC SURVEY, THOROUGHLY REVIEWING ALL PLANS AND REPORTS, AND ADDITIONAL INFORMATION REQUESTED FOR CLARIFICATION PRIOR TO
- 2. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL REQUIRED PERMITS PRIOR TO
- 3. EXISTING TOPOGRAPHIC AND BOUNDARY INFORMATION TAKEN FROM A SURVEY DATED 07/26/2016. SUPPLEMENTAL INFORMATION, GATHERED FROM SITE VISITS, RECORD DRAWINGS, PLANS OBTAINED FROM CLIENT, ARE SHOWN AS APPROXIMATE LOCATIONS ONLY AND SHALL BE FIELD VERIFIED PRIOR TO OR DURING CONSTRUCTION. CONTACT ENGINEER IMMEDIATELY IF ANY DISCREPANCIES ARE DISCOVERED.
- THE LOCATIONS OF ALL EXISTING UTILITIES SHOWN ON THIS PLAN HAVE BEEN DETERMINED FROM THE BEST INFORMATION AVAILABLE AND ARE GIVEN FOR THE CONVENIENCE OF THE CONTRACTOR. THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THEIR ACCURACY. PRIOR TO THE START OF ANY DEMOLITION ACTIVITY, THE CONTRACTOR SHALL NOTIFY THE UTILITY COMPANIES FOR ONSITE LOCATIONS OF EXISTING UTILITIES. THE CONTRACTOR IS REQUIRED TO TAKE DUE PRECAUTIONARY MEASURES TO PROTECT ANY EXISTING UTILITIES OR STRUCTURES LOCATED AT THE WORK SITE.
- 5. PROVIDE AND MAINTAIN TRAFFIC CONTROL DEVICES WHERE NECESSARY (INCIDENTAL). PLACEMENT OF THESE DEVICES SHALL BE APPROVED BY THE CITY AND ENGINEER PRIOR TO PLACEMENT. TRAFFIC CONTROL DEVICES SHALL MEET THE REQUIREMENTS OF THE MMUTCD, CURRENT ADDITION.
- CONTRACTOR SHALL MAINTAIN FULL ACCESS TO ADJACENT PROPERTIES DURING CONSTRUCTION AND TAKE ALL PRECAUTIONS NECESSARY TO AVOID PROPERTY DAMAGE TO
- 7. ALL CONSTRUCTION WORK SHALL BE COMPLETED WITHIN CITY APPROVED WORKING
- 8. INSTALL CONTROL FENCING AND BARRICADING AS NECESSARY TO PROTECT THE PUBLIC.

OWNERS

CITY OF DULUTH

ERIK BIRKELAND PROJECT MANAGER

218.730.4435

CHELLY TOWNSEND

DECC

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DESIGN TEAM

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ELECTRICAL ENGINEER

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Know what's below.

Call before you dig.

GOVERNING SPECIFICATIONS

THE 2016 EDITION OF THE MINNESOTA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR CONSTRUCTION AND THE 2016 VERSION OF THE DULUTH STANDARD CONSTRUCTION SPECIFICATIONS SHALL GOVERN.

THE LOCATIONS OF UNDERGROUND UTILITIES HAVE BEEN PROVIDED BY THE UTILITY OWNER, ARE SHOWN IN AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED. THE CONTRACTOR SHALL FIELD VERIFY EXACT LOCATION OF ALL UTILITIES BEFORE COMMENCING CONSTRUCTION PER STATE LAW.

JOB No: 161080 DATE: 09/22/2017 DRAWN BY: CJO designed by: ECR

DECC SEAWALL PROJECT

ST. LOUIS COUNTY DULUTH, MINNESOTA

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NOSEK		CIVIL		
STO GENERAL NOTES STO	COVER	TITLE SHEET		S0.0
CI.1 GENERAL NOTES SI.1	INDEX	PLAN INDEX		S0.1
S12 SIMALED QUARTITIES S13	G1.0	GENERAL NOTES		S1.0
S13 CMIL SHEET OVERMEN S13	G1.1	GENERAL NOTES		S1.1
C1.1 DOMOLITION PLAN - STATION 0 + 00 TO 4 + 90	G1.2	ESTIMATED QUANTITIES		S1.2
C1.2 DEMOLITION PLAN — STATION 4 + 90 TO 10 + 50 C1.3 DEMOLITION PLAN — STATION 10 + 50 TO 15 + 30 C1.4 DEMOLITION PLAN — STATION 15 + 30 TO 20 + 50 C1.5 DEMOLITION PLAN — STATION 20 + 50 TO 24 + 03 C2.1 CIVIL PLAN — STATION 20 + 50 TO 15 + 30 C2.2 CIVIL PLAN — STATION 20 + 50 TO 15 + 30 C2.3 CIVIL PLAN — STATION 10 + 50 TO 15 + 30 C2.4 CIVIL PLAN — STATION 10 + 50 TO 15 + 30 C2.5 CIVIL PLAN — STATION 20 + 50 TO 24 + 03 C2.6 CIVIL PLAN — STATION 20 + 50 TO 24 + 03 C2.7 CIVIL PLAN — STATION 20 + 50 TO 24 + 03 C2.8 CIVIL PLAN — STATION 20 + 50 TO 24 + 90 C3.1 CIVIL PLAN — STATION 20 + 50 TO 24 + 90 C3.2 CRADING PLAN — STATION 4 + 90 TO 10 + 50 C3.3 CRADING PLAN — STATION 4 + 90 TO 10 + 50 C3.4 CRADING PLAN — STATION 10 + 50 TO 15 + 30 C3.5 CRADING PLAN — STATION 10 + 50 TO 15 + 30 C3.6 CRADING PLAN — STATION 20 + 50 TO 24 + 03 C3.7 CRASS SECTIONS — STATION 20 + 50 TO 24 + 03 C3.8 CRADING PLAN — STATION 10 + 50 TO 15 + 30 C3.9 CRADING PLAN — STATION 10 + 50 TO 15 + 30 C3.1 CRASS SECTIONS — STATION 10 + 50 TO 12 + 50 C3.2 CRADING PLAN — STATION 10 + 50 TO 15 + 30 C3.3 CRADING PLAN — STATION 10 + 50 TO 12 + 50 C3.4 CRASS SECTIONS — STATION 10 + 00 TO 22 + 50 C3.5 CRADING PLAN — STATION 10 + 00 TO 22 + 50 C3.6 CRASS SECTIONS — STATION 10 + 00 TO 22 + 50 C3.7 CRASS SECTIONS — STATION 10 + 00 TO 12 + 50 C3.8 CRASS SECTIONS — STATION 10 + 00 TO 12 + 50 C3.9 CRASS SECTIONS — STATION 10 + 00 TO 12 + 50 C4.1 EROSION CONTROL PLAN — STATION 4 + 90 TO 10 + 50 C4.2 EROSION CONTROL PLAN — STATION 10 + 50 TO 15 + 30 C4.4 EROSION CONTROL PLAN — STATION 10 + 50 TO 15 + 30 C4.5 EROSION CONTROL PLAN — STATION 10 + 50 TO 15 + 30 C4.5 EROSION CONTROL PLAN — STATION 10 + 50 TO 15 + 30 E0.1	G1.3	CIVIL SHEET OVERVIEW		S1.3
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C1.4 DEMOLITION PLAN - STATION 15 + 30 TO 20 + 50	C1.2	DEMOLITION PLAN - STATION 4 + 90 TO 10 + 50		S1.5
C1.5 DEMOLITION PLAN - STATION 20 + 50 TO 24 + 03 C2.1 CML PLAN - STATION 0 + 00 TO 4 + 90 S2.1 C2.2 CML PLAN - STATION 10 + 50 TO 15 + 30 C2.3 CML PLAN - STATION 10 + 50 TO 15 + 30 C2.4 CML PLAN - STATION 10 + 50 TO 15 + 30 C2.5 CML PLAN - STATION 10 + 50 TO 20 + 50 C2.6 CML PLAN - STATION 20 + 50 TO 24 + 03 C2.6 CML PLAN - STATION 20 + 50 TO 24 + 03 C3.1 GRADING PLAN - STATION 20 + 50 TO 24 + 03 C3.2 GRADING PLAN - STATION 10 + 00 TO 4 + 90 C3.2 GRADING PLAN - STATION 10 + 50 TO 15 + 30 C3.3 GRADING PLAN - STATION 10 + 50 TO 15 + 30 C3.4 GRADING PLAN - STATION 10 + 50 TO 24 + 03 C3.5 GRADING PLAN - STATION 10 + 50 TO 24 + 03 C3.6 CROSS SECTIONS - STATION 20 + 50 TO 24 + 03 C3.7 CROSS SECTIONS - STATION 0 + 00 TO 3 + 50 C3.8 CROSS SECTIONS - STATION 11 + 00 TO 12 + 50 C3.9 CROSS SECTIONS - STATION 11 + 00 TO 12 + 50 C3.1 EROSION CONTROL PLAN - STATION 10 + 50 TO 12 + 50 C3.2 GROSS SECTIONS - STATION 11 + 00 TO 17 + 50 C3.3 GROSS SECTIONS - STATION 11 + 00 TO 17 + 50 C3.4 CROSS SECTIONS - STATION 11 + 00 TO 17 + 50 C4.1 EROSION CONTROL PLAN - STATION 10 + 50 TO 15 + 30 C4.2 EROSION CONTROL PLAN - STATION 10 + 50 TO 15 + 30 C4.3 EROSION CONTROL PLAN - STATION 10 + 50 TO 15 + 30 C4.4 EROSION CONTROL PLAN - STATION 10 + 50 TO 15 + 30 C4.5 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03	C1.3	DEMOLITION PLAN - STATION 10 + 50 TO 15 + 30		S1.6
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C3.7 CROSS SECTIONS - STATION 4 + 00 TO 8 + 50 C3.8 CROSS SECTIONS - STATION 9 + 50 TO 12 + 50 S5.3 C3.9 CROSS SECTIONS - STATION 13 + 00 TO 17 + 50 S5.4 C3.10 CROSS SECTIONS - STATION 18 + 00 TO 22 + 50 C4.1 EROSION CONTROL PLAN - STATION 0 + 00 TO 4 + 90 C4.2 EROSION CONTROL PLAN - STATION 4 + 90 TO 10 + 50 C4.3 EROSION CONTROL PLAN - STATION 10 + 50 TO 15 + 30 M1.1 C4.4 EROSION CONTROL PLAN - STATION 15 + 30 TO 20 + 50 C4.5 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 E0.1	C3.5	GRADING DETAILS - STATION 20 + 50 TO 24 + 03		S5.0
C3.8 CROSS SECTIONS - STATION 9 + 50 TO 12 + 50 C3.9 CROSS SECTIONS - STATION 13 + 00 TO 17 + 50 S5.4 C3.10 CROSS SECTIONS - STATION 18 + 00 TO 22 + 50 C4.1 EROSION CONTROL PLAN - STATION 0 + 00 TO 4 + 90 C4.2 EROSION CONTROL PLAN - STATION 4 + 90 TO 10 + 50 C4.3 EROSION CONTROL PLAN - STATION 10 + 50 TO 15 + 30 M1.1 C4.4 EROSION CONTROL PLAN - STATION 15 + 30 TO 20 + 50 C4.5 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 E0.1	C3.6	CROSS SECTIONS - STATION 0 + 00 TO 3 + 50		S5.1
C3.9 CROSS SECTIONS - STATION 13 + 00 TO 17 + 50 C3.10 CROSS SECTIONS - STATION 18 + 00 TO 22 + 50 C4.1 EROSION CONTROL PLAN - STATION 0 + 00 TO 4 + 90 C4.2 EROSION CONTROL PLAN - STATION 4 + 90 TO 10 + 50 C4.3 EROSION CONTROL PLAN - STATION 10 + 50 TO 15 + 30 M1.1 C4.4 EROSION CONTROL PLAN - STATION 15 + 30 TO 20 + 50 C4.5 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 E0.1	C3.7	CROSS SECTIONS - STATION 4 + 00 TO 8 + 50		S5.2
C3.10 CROSS SECTIONS - STATION 18 + 00 TO 22 + 50 C4.1 EROSION CONTROL PLAN - STATION 0 + 00 TO 4 + 90 C4.2 EROSION CONTROL PLAN - STATION 4 + 90 TO 10 + 50 C4.3 EROSION CONTROL PLAN - STATION 10 + 50 TO 15 + 30 M1.1 C4.4 EROSION CONTROL PLAN - STATION 15 + 30 TO 20 + 50 C4.5 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 E0.1	C3.8	CROSS SECTIONS - STATION 9 + 50 TO 12 + 50		S5.3
C4.1 EROSION CONTROL PLAN - STATION 0 + 00 TO 4 + 90 C4.2 EROSION CONTROL PLAN - STATION 4 + 90 TO 10 + 50 C4.3 EROSION CONTROL PLAN - STATION 10 + 50 TO 15 + 30 C4.4 EROSION CONTROL PLAN - STATION 15 + 30 TO 20 + 50 C4.5 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 E0.1	C3.9	CROSS SECTIONS - STATION 13 + 00 TO 17 + 50		S5.4
C4.2 EROSION CONTROL PLAN — STATION 4 + 90 TO 10 + 50 C4.3 EROSION CONTROL PLAN — STATION 10 + 50 TO 15 + 30 C4.4 EROSION CONTROL PLAN — STATION 15 + 30 TO 20 + 50 C4.5 EROSION CONTROL PLAN — STATION 20 + 50 TO 24 + 03 E0.1	C3.10	CROSS SECTIONS - STATION 18 + 00 TO 22 + 50		S5.5
C4.3 EROSION CONTROL PLAN - STATION 10 + 50 TO 15 + 30 C4.4 EROSION CONTROL PLAN - STATION 15 + 30 TO 20 + 50 C4.5 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 E0.1	C4.1	EROSION CONTROL PLAN - STATION 0 + 00 TO 4 + 90		
C4.4 EROSION CONTROL PLAN - STATION 15 + 30 TO 20 + 50 C4.5 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 E0.1	C4.2	EROSION CONTROL PLAN - STATION 4 + 90 TO 10 + 50		M1.0
C4.5 EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03 E0.1	C4.3	EROSION CONTROL PLAN - STATION 10 + 50 TO 15 + 30		M1.1
	C4.4	EROSION CONTROL PLAN - STATION 15 + 30 TO 20 + 50		
C4.6 EROSION CONTROL DETAILS E1.1	C4.5	EROSION CONTROL PLAN - STATION 20 + 50 TO 24 + 03		E0.1
	C4.6	EROSION CONTROL DETAILS		E1.1

	STRUCTURAL / MARINE
S0.0	STRUCTURAL / MARINE NOTES
S0.1	STRUCTURAL / MARINE NOTES
S1.0	PROJECT OVERVIEW PLAN
S1.1	DOCK SITE PLAN - STATION 0 + 00 TO 3 + 50 & INNER END OF MINNESOTA SLIP
S1.2	DOCK SITE PLAN - STATION 3 + 50 TO 6 + 30
S1.3	DOCK SITE PLAN - STATION 6 + 30 TO 12 + 08
S1.4	DOCK SITE PLAN - STATION 12 + 08 TO 16 + 00
S1.5	DOCK SITE PLAN - STATION 16 + 00 TO 20 + 21
S1.6	DOCK SITE PLAN - STATION 20 + 21 TO 22 + 25
S1.7	DOCK SITE PLAN - STATION 22 + 25 TO 24 + 03
S2.0	DOCK WALL SECTIONS
S2.1	DOCK WALL SECTIONS
S2.2	DOCK WALL SECTIONS
S2.3	DOCK WALL REHAB SECTION & DETAILS
S2.4	DOCK WALL REHAB LADDER SECTION & DETAILS
S3.0	DOCK WALL DETAIL & LOWER CONCRETE PLATFORM PLAN & SECTIONS
S3.1	DOCK WALL DETAILS & SECTIONS
S3.2	DOCK WALL DETAILS & SECTIONS
S4.0	GENERAL DOCK WALL SECTIONS AND DETAILS
S4.1	GENERAL DOCK WALL SECTIONS AND DETAILS
S4.2	GENERAL BOLLARD SECTIONS AND DETAILS
S4.3	GENERAL BOLLARD SECTIONS AND DETAILS
S5.0	GUARDRAIL PLAN - STATION 6 + 00 TO 12 + 00
S5.1	GUARDRAIL PLAN - STATION 12 + 00 TO 16 + 00
S5.2	GUARDRAIL PLAN - STATION 16 + 00 TO 20 + 25
S5.3	GUARDRAIL PLAN - STATION 20 + 25 TO 23 + 45
S5.4	GENERAL DETAILS
S5.5	GENERAL DETAILS
	MECHANICAL / PLUMBING
M1.0	DOCKSIDE WATER PLAN - STATION 5 + 00 TO 14 + 00
M1.1	DOCKSIDE WATER PLAN - STATION 14 + 00 TO 18 + 00
	ELECTRICAL
E0.1	TITLE SHEET
E1.1	ELECTRICAL OVERALL DEMOLITION PLAN
E1.2	OVERALL ELECTRICAL SITE PLAN
E1.3	ENLARGED PLAN STA 0 + 00 TO 5 + 00
E1.4	ENLARGED PLAN STA 5 + 00 TO 12 + 50
E1.5	ENLARGED PLAN STA 12 + 50 TO 18 + 00
E1.6	ENLARGED PLAN STA 18 + 00 TO 22 + 50
E2.1	DETAIL SHEET
E3.1	SCHEDULES



REV. BY:					
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- CONTRACTOR IS RESPONSIBLE FOR SUBMITTING A "SITE SAFETY AND SITE CONTINGENCY PLAN" TO ADDRESS THE REQUIREMENTS OF 29 CFR 1910.120
- CONTRACTOR MUST SUBMIT A SCHEDULE INDICATING WORK SEQUENCE UNDER THE FOLLOWING GUIDELINES: COORDINATE THE SCHEDULE TO ALLOW SUFFICIENT TIME FOR REQUIRED TESTING, INSPECTIONS, AND INSTALLATION OF WORK
- COORDINATE WITH CONTINUATION OF OWNERS ON SITE OPERATIONS.
 - SUBMIT PRODUCT DATA FOR ALL MATERIALS REQUIRED FOR A COMPLETE INSTALLATION PRIOR TO INSTALLATION DEMONSTRATING THAT MATERIALS MEET THE SPECIFICATIONS.
- PROVIDE PLANS INDICATING LOCATION OF SALVAGEABLE ITEMS, LOCATIONS AND CONSTRUCTION OF BARRICADES, FENCES, AND TEMPORARY WORK.
- 4. SUBMIT PRE-DEMOLITION PHOTOGRAPHS OR VIDEO BEFORE BEGINNING WORK
- SUBMIT COPIES OF ALL PERMITS RECEIVED ALONG WITH ANY SPECIAL CONDITIONS OR REQUIREMENTS OF COMPLIANCE.
- 6. SUBMIT THE NAME AND PHONE NUMBER OF THE SWPPP COORDINATOR AND FILL OUT THE SWPPP PLAN SHEET WITH SAME.
- SUBMIT THE NAME AND TELEPHONE NUMBER OF A PROJECT SUPERINTENDENT WHO WILL BE ON SITE DURING REGULAR WORK HOURS AND ALSO AVAILABLE ON CALL 24 HOURS A DAY FOR THE DURATION OF THE PROJECT. THIS PERSON SHALL BE THE CONTACT FOR MAINTENANCE OF THE BARRICADES, LIGHTING, AND SAFETY.
- SUBMIT TEST REPORTS FROM THE INDEPENDENT TESTING FACILITY FOR MATERIALS AND COMPACTION SHOWING COMPLIANCE WITH THE
- MATERIAL SUBMITTALS AND SHOP DRAWINGS INCLUDE, BUT ARE NOT LIMITED TO THE FOLLOWING:
- THE CONTRACTOR AND BITUMINOUS ASPHALT PROVIDER SHALL JOINTLY PROVIDE CERTIFICATES CERTIFYING THAT MATERIALS
- COMPLY WITH SPECIFICATION REQUIREMENTS THE BITUMINOUS MIX PLANT SHALL HAVE, ON FILE, A REPORT BY AN APPROVED TESTING LABORATORY THAT INDICATES THE
- PROPORTIONS OF MATERIALS USED IN EACH TYPE OF BITUMINOUS PROVIDED AND THE TEMPERATURE OF THE MIX. SUBMIT SUBSTANTIATING DATA FOR CONCRETE MIX DESIGN TO THE OWNER AND INDEPENDENT TESTING AGENCY NOT LESS THAN TWO WEEKS PRIOR TO CONCRETE PLACEMENT.
- SAMPLES FROM OFF SITE BORROW SHALL BE OBTAINED WITH SUFFICIENT TIME TO PERMIT TESTING, REVIEW AND APPROVAL PRIOR TO DELIVERY TO SITE.
- GRADATIONS FOR ALL GRANULAR AND AGGREGATE MATERIALS USED DURING THE PROJECT.
- CASTINGS AND MANHOLE SCHEDULES FOR ALL STORM AND SANITARY STRUCTURES, HYDRANTS, VALVES, AND ALL UTILITY APPURTENANCES
- ALL PIPE MATERIALS
- PAVIEMENT MARKING PAINT

GENERAL NOTES:

- ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE CURRENT EDITION OF THE MINNESOTA DEPARTMENT OF TRANSPORTATION "STANDARD SPECIFICATIONS FOR CONSTRUCTION" CONCURRENT WITH THE PERMIT DATE.
- . BY SUBMISSION OF BID, THE CONTRACTOR ACKNOWLEDGES THAT THEY HAVE THOROUGHLY EXAMINED THE LOCATION OF THE WORK TO BE PERFORMED, AND IS FAMILIAR WITH LOCAL CONDITIONS, AND HAS THE RESOURCES TO LAY OUT AND CONSTRUCT THESE PLANS.
- ALL WORK AND MATERIALS WHICH DO NOT CONFORM TO THE SPECIFICATIONS AND/OR PLANS ARE SUBJECT TO REMOVAL AND REPLACEMENT AT THE CONTRACTOR'S EXPENSE. THE QUANTITIES AND SITE CONDITIONS DEPICTED IN THESE PLANS ARE FOR INFORMATIONAL PURPOSES ONLY AND ARE SUBJECT TO ERROR AND OMISSION. THE CONTRACTOR SHALL CONFIRM TO THEMSELVES AS TO ACTUAL QUANTITIES AND SITE CONDITIONS PRIOR TO BIDDING THE WORK FOR THE CONSTRUCTION COVERED BY THESE PLANS. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES OR SAFETY PRECAUTIONS OR PROGRAMS UTILIZED IN CONNECTION WITH THE WORK. THE ENGINEER OR OWNER SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION.
- CONTRACTOR SHALL ACQUIRE ALL NECESSARY PERMITS BEFORE BEGINNING CONSTRUCTION.
- ANY DIFFERENCES BETWEEN THE PLANS AND SPECIFICATIONS OR QUESTIONS REGARDING INTERPRETATION OF THE PLANS AND SPECIFICATIONS SHALL BE RESOLVED BY THE ENGINEER OR OWNER. THE CONTRACTOR WILL NOT BE PERMITTED TO TAKE ADVANTAGE OF ANY ERRORS OR OMISSIONS IN THE PLANS AND SPECIFICATIONS. THE GENERAL INTENT AND MEANING WILL GOVERN. THE ENGINEER OR OWNER WILL PROVIDE FULL INSTRUCTIONS WHEN CHANGED CONDITIONS, ERRORS OR OMISSIONS ARE DISCOVERED BY THE CONTRACTOR.
- THE CONTRACTOR IS REQUIRED TO TAKE DUE PRECAUTIONARY MEASURES TO PROTECT THE UTILITY LINES SHOWN AND ANY OTHER LINES NOT OF RECORD OR NOT SHOWN ON THESE PLANS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING GOPHER 811
- THOUGH SUCH WORK MAY NOT BE SPECIFICALLY BE NOTED ON THESE PLANS. THE CONTRACTOR SHALL FURNISH AND INSTALL A SUPPLEMENTARY OR MISCELLANEOUS FITTINGS, APPURTENANCES AND DEVICES INCIDENTAL TO OR NECESSARY FOR A SOUND, SECURE, AND COMPLETE INSTALLATION.
- THE CONTRACTOR SHALL PRESERVE AND PROTECT ALL SURVEY CONTROL STAKES SET FOR LINE, GRADE, OR CONTROL IN THEIR ORIGINAL LOCATIONS. ANY EXPENSES INCURRED IN REPLACING ANY SURVEY STAKES WHICH THE CONTRACTOR OR CONTRACTORS SUB-CONTRACTORS MAY HAVE FAILED TO PRESERVE SHALL BE THE FINANCIAL RESPONSIBILITY OF THE CONTRACTOR.
-). CONTRACTOR SHALL AT ALL TIMES COORDINATE WORK WITH OTHER CONTRACTORS INVOLVED WITH ONGOING CONSTRUCTION OF THIS PROJECT.
- 10. ALL CONSTRUCTION REMOVAL ITEMS SHALL BE DISPOSED OF AT AN APPROVED WASTE SITE.
- 11. ALL MATERIALS USED AND ALL WORK DONE BY THE CONTRACTOR SHALL BE SUBJECT AT ALL TIME TO THE INSPECTION, TESTING, AND APPROVAL OF THE OWNER OR OWNER'S REPRESENTATIVE AND THE ENGINEER.
- 12. ALL CONTRACTOR AND SUBCONTRACTOR PERSONNEL, VEHICLES, EQUIPMENT, AND MATERIALS SHALL REMAIN WITHIN THE LIMITS OF CONSTRUCTION AND WITHIN THE SITE ACCESS AND HAUL ROUTES.
- 13. DUMP TRUCKS SHALL USE LOAD COVER AND SHALL BE LOADED BY THE CONTRACTOR SUCH THAT NO SPILLAGE OCCURS DURING TRANSIT.
- 14. CONTRACTOR IS RESPONSIBLE FOR THE MAINTENANCE, REPAIR, AND RESTORATION OF HAUL ROADS.
- 15. OPEN TRENCHES AND EXCAVATIONS AT THE CONSTRUCTION SITE SHALL BE PROMINENTLY MARKED WITH BARRICADES.
- 16. THE CONTRACTOR SHALL NOTIFY THE OWNER AT LEAST 72 HOURS MINIMUM IN ADVANCE OF THE DATE WHEN UTILITIES ARE PROPOSED TO BE SHUT DOWN OR DISCONNECTED.
- 17. ACCESS TO ALL FIRE HYDRANTS AND STAND PIPES SHALL BE MAINTAINED AT ALL TIMES.

MATERIALS:

1. CONCRETE MATERIALS:

- 1.1. AIR-ENTRAINING ADMIXTURE SHALL BE PER MNDOT SPECIFICATION 3113.
- 1.2. CONCRETE CURBING SHALL BE PER MNDOT SPECIFICATION 2531.
- 1.2.1. USE CONCRETE MIX NO. 3F52 PER MNDOT SPECIFICATION 2461 FOR HAND PLACEMENT OF CONCRETE FOR CURBING.
- 1.2.2. USE CONCRETE MIX NO. 3F32 PER MNDOT SPECIFICATION 2461 FOR MACHINE PLACEMENT OF CONCRETE FOR CURBING. 1.3. CONCRETE SIDEWALK SHALL BE PER MNDOT 2521.
- 1.3.1. USE CONCRETE MIX NO. 3F52 PER MNDOT SPECIFICATION 2461 FOR PLACEMENT OF CONCRETE FOR SIDEWALK.
- USE CONCRETE MIX NO. 3F52EX OR 3F53EX PER MNDOT SPECIFICATION 2461 FOR PLACEMENT OF CONCRETE FOR SIDEWALK WITH EXPOSED AGGREGATE.
- 1.4. CONCRETE PAVEMENT SHALL BE PER MNDOT SPECIFICATION 2301.
- 1.4.1. HAVE A MINIMUM 4,000 POUNDS PER SQUARE INCH (PSI) COMPRESSIVE STRENGTH AT 28 DAYS AND A MODULUS OF RUPTURE OF AT LEAST 600PSI.
- 1.4.2. HAVE 3/4 INCH MAXIMUM COARSE AGGREGATE PER MNDOT SPECIFICATION 3137 AND ASTM C33.
- 1.4.3. FINE AGGREGATE SHALL BE PER MNDOT SPECIFICATION 3126.
- FINE AGGREGATE SHALL BE WASHED FREE OF SILT, CLAY, LOAM, FRIABLE OR SOLUBLE MATERIALS.
- FINE AGGREGATE SHALL BE GRADED IN ACCORDANCE WITH ANSI/ASTM C136 AND MNDOT 3126 WITH LESS THAN 2.5 PERCENT PASSING NO. 200 SIEVE.
- 1.4.4. HAVE TYPE 1 PORTLAND CEMENT MEETING THE REQUIREMENTS OF ASTM C150.
- 1.4.5. PROVIDE 6.0 PERCENT, PLUS OR MINUS ONE PERCENT AIR CONTENT AIR ENTRAINMENT PER ASTM C260.
- PROVIDE 0.5:1.0 MINIMUM CEMENT PLUS AIR ENTRAINMENT TO WATER RATIO.
- 1.4.7. HAVE ONE INCH TO THREE INCHES SLUMP.
- 1.4.8. HAVE BETWEEN 530 POUNDS TO 850 POUNDS CEMENT PER CUBIC YARD OF CONCRETE.

2. ASPHALT MATERIALS:

- 2.1. THE BITUMINOUS TACK COAT SHALL BE RC LIQUID ASPHALT OR EMULSIFIED ASPHALT AND SHALL MEET THE REQUIREMENTS OF MNDOT SPECIFICATIONS 2357 & 3151.
- 2.2. FINE AGGREGATE SHALL BE PER MNDOT SPECIFICATION 2360.
- 2.2.1. FINE AGGREGATE SHALL BE FREE OF SILT, CLAY, LOAM, FRIABLE OR SOLUBLE MATERIALS.
- 2.2.2. FINE AGGREGATE SHALL BE GRADED IN ACCORDANCE WITH ANSI/ASTM C136 AND MNDOT 2360.
- FINE AGGREGATE CAN BE NATURAL RIVER OR BANK SAND.
- 2.3. THE BITUMINOUS PAVEMENT SHALL BE PRODUCED IN ACCORDANCE WITH MNDOT SPECIFICATION 2360 PLANT MIXED ASPHALT PAVEMENT.
- 2.4. THE BITUMINOUS MIX SHALL BE AS SHOWN ON THE PLANS.
- 2.5. MIX PLANT AND MIX DESIGN TO BE MNDOT CERTIFIED.

3. AGGREGATE MATERIALS:

- 3.1. AGGREGATE SHALL MEET THE REQUIREMENTS OF MNDOT 3138, FOR CLASS 5 AGGREGATE.
- 3.2. GRANULAR MATERIAL SHALL MEET THE REQUIREMENTS OF MNDOT SPECIFICATION 3149
- 3.3. SELECT GRANULAR MATERIAL SHALL MEET THE REQUIREMENTS OF MNDOT SPECIFICATION 3149
- 3.4. MODIFIED SELECT GRANULAR MATERIAL SHALL MEET THE REQUIREMENTS OF MNDOT SPECIFICATION 3149
- 3.5. TOPSOIL SHALL MEET THE REQUIREMENTS OF MNDOT SPECIFICATION 3877.

4. WATER UTILITIES: 4.0 MECHANICAL SPECIFICATIONS GOVERN IF ANY CONFLICT.

- 4.1. DUCTILE IRON PIPE
- 4.1.1. PIPE THREE TO TWELVE INCHES INSIDE DIAMETER SHALL COMPLY WITH ANSI A-21.51, CLASS 52 4.1.2. PIPE LARGER THAN TWELVE INCHES DIAMETER SHALL BE CLASS 51
- 4.1.3. PIPE SHALL HAVE WORKING PRESSURE OF NOT LESS THAN 150 PSI UNLESS OTHERWISE SHOWN OR SPECIFIED
- 4.2. COPPER TUBING
- ALL FITTINGS FOR COPPER TUBING SHALL BE CAST BRASS, HAVING UNIFORMITY IN WALL THICKNESS AND STRENGTH AND SHALL BE FREE OF DEFECTS AFFECTING SERVICEABILITY.
- 4.2.2. ALL FITTINGS TO BE FLARED OR COMPRESSION TYPE

4.3. FIRE HYDRANTS

- 4.3.1. FIRE HYDRANTS SHALL BE TYPE, SIZE, AND CONSTRUCTION AS REQUIRED BY THE CITY, TOWNSHIP, TRIBE, OR APPLICABLE REGULATORY ENTITIY(S)
- 4.3.2. FIRE HYDRANTS SHALL CONFORM TO THE APPLICABLE REQUIREMENTS OF AWWA C-502

5. SANITARY SEWER UTILITIES: NA

6. STORM SEWER UTILITIES:

- 6.1. REINFORCED CONCRETE PIPE (RCP) AND APRON SECTION
- 6.1.1. PIPE SHALL MEET THE REQUIREMENTS OF MNDOT SPECIFICATION 3236 AND MNDOT STANDARD DETAILS
- 6.1.2. APRON SECTIONS SHALL BE TIED BACK A MINIMUM OF THREE PIPE JOINTS, PIPE TIES SHALL BE PER MNDOT DETAIL 3145E
- 6.1.3. THE LENGTHS OF STORM PIPES INDICATED ON PLANS ARE APPROXIMATE FROM END TO END OF PIPE, AND INCLUDED APRONS AND OTHER SPECIAL SECTIONS.
- 6.1.4. RIPRAP PLACED AT APRONS SHALL CONFORM TO MNDOT SPECIFICATION 2511 AND 3601
- 6.1.5. PROVIDE GEOTEXTILE FABRIC UNDER RIPRAP IN ACCORDANCE WITH MNDOT SPECIFICATION 2511 AND 3733
- 6.2. PRECAST CONCRETE MANHOLE/CATCH BASINS
- 6.2.1. ALL STORM STRUCTURES SHALL BE PRECAST REINFORCED CONCRETE AS SHOWN ON THE PLANS AND DETAILS
- 6.2.2. FOLLOW CITY, COUNTY, TOWNSHIP, TRIBAL, OR ANY APPLICABLE REGULATORY ENTITY(S) STANDARDS AND SPECIFICATION WHEN IN PUBLIC RIGHT-OF-WAY.

7. EROSION AND SEDIMENT CONTROL:

- 7.1. SILT FENCE SHALL BE PER MNDOT SPECIFICATION 3886
- 7.2. TEMPORARY ROCK CONSTRUCTION ENTRANCE SHALL BE COARSE AGGREGATE IN ACCORDANCE WITH MNDOT 3137 CLASS 1 OR CLASS
- 7.3. EROSION CONTROL BLANKETS SHALL BE PER MNDOT SPECIFICATION 3885

7.4. SEEDING:

- 7.4.1. SEED MIXTURE, UNLESS INDICATED OTHERWISE ON PLANS, TO BE AS FOLLOWS:
- 7.4.1.1. FOR LAWN TYPE AREAS: MNDOT MIXTURE NO. 25-131
- 7.4.1.2. FOR PRAIRIE GRASS SEEDED AREAS: MNDOT MIXTURE NO. 35-241 7.4.1.3. FOR POND EROSION CONTROL AREAS: MNDOT MIXTURE NO. 33-261 OR 33-361
- 7.4.1.4. FOR TEMPORARY SEEDING: MNDOT MIXTURE NO. 22-111
- 7.4.2. MULCH PER MNDOT SECTION 3882, TYPE 1.
- 7.4.3. FERTILIZER SHALL BE PER RECOMMENDATION OF THE MANUFACTURER OF THE SPECIFIED SEED MIXTURE
- 7.4.4. TOPSOIL SHALL MEET THE REQUIREMENTS OF MNDOT SPECIFICATION 3877
- WATER MUST BE CLEAN, FRESH, AND FREE OF SUBSTANCE OR MATTER THAT COULD INHIBIT VIGOROUS GROWTH OF THE GRASS.

8. SIGNAGE:

- 8.1. SIGN MATERIALS SHALL BE ACCORDANCE WITH MNDOT SPECIFICATION 3352 AND THE CURRENT VERSION OF MN MUTCO
- 8.1.1. SHEET SIGN PANELS OF ALUMINUM ALLY 6061-T6 OR 5052-H38 CONFORMING TO ASTM B209

8.2. POST 8.2.1. SHALL BE IN ACCORDANCE WITH MNDOT SPECIFICATION 3401 AND THE CURRENT VERSION OF MN MUTCD

FIELD QUALITY CONTROL:

1. CONCRETE MATERIALS:

- 1.1. FIELD TESTING AND INSPECTION SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY EMPLOYED BY THE OWNER. THE
- TESTING FIRM SHALL PREPARE CYLINDERS AND PERFORM SLUMP AND AIR ENTRAINMENT TESTS IN ACCORDANCE WITH ACI 301.
- 1.2. THE TESTING FIRM SHALL PERFORM SEVEN DAY COMPRESSION TESTS AND 28 DAY COMPRESSION TESTS. 1.3. FIVE CONCRETE TEST CYLINDERS SHALL BE TAKEN FOR EVERY 75 OR LESS CUBIC YARDS OF EACH CLASS OF CONCRETE PLACED EACH
- 1.4. ONE ADDITIONAL TEST CYLINDER SHALL BE TAKEN DURING COLD WEATHER (BELOW 40°F) AND CURED ON SITE UNDER THE SAME
 - CONDITIONS AS THE CONCRETE IT REPRESENTS.
- 1.5. ONE SLUMP TEST SHALL BE TAKEN FOR EACH SET OF TEST CYLINDERS TAKEN.
- 1.6. THE TESTING FIRM TO MAINTAIN RECORDS OF PLACED CONCRETE ITEMS.
- 1.7. THE TESTING FIRM TO RECORD DATE, LOCATION OF POUR, QUANTITY, AIR TEMPERATURE AND TEST SAMPLES TAKEN.

- THE FOLLOWING METHODS OF TESTING BITUMINOUS COURSES ARE ACCEPTABLE:
- 2.0.1. PORTABLE NUCLEAR DENSITY METHOD:
- 2.0.2. A PORTABLE NUCLEAR DENSITY DEVICE SHALL BE USED WHILE THE MIXTURE IS BEING PLACED AND COMPACTED.
- 2.0.3. THE ROLLING PATTERN SHALL BE ESTABLISHED TO OBTAIN THE SPECIFIED COMPACTION LEVEL.
- 2.1. THIS IS THE PREFERRED METHOD OF TESTING. 2.1.1. CORE SAMPLES:
 - CUT SAMPLES FROM ANY COURSE OR FINISHED PAVEMENT, NOT TO EXCEED FIVE IN NUMBER, FROM ANY DAY'S RUN FOR TESTS OF DENSITY AND COMPOSITION.
 - THE SAMPLES SHALL BE TAKEN AT POINTS DESIGNATED BY THE OWNER BY SAWING WITH A POWER DRIVEN MASONRY SAW OR DIAMOND CORE DRILL.
 - THE SAMPLES SHALL BE SUFFICIENTLY LARGE ENOUGH TO MEET THE NEEDS OF THE TESTING LABORATORY. THE SURFACES FROM WHICH SAMPLES ARE TAKEN SHALL BE RESTORED BY THE CONTRACTOR NO LATER THAN THE NEXT
- SUCCEEDING DAY OF PLANT OPERATION.
- THE SAMPLES SHALL BE PAID FOR BY THE CONTRACTOR.
- 2.1.2. THIS METHOD SHALL BE USED IF THE PORTABLE NUCLEAR DENSITY METHOD INDICATES THAT THE LIFT DOES NOT MEET THE MINIMUM SPECIFIED DENSITY
- THIS METHOD SHALL BE CARRIED OUT IN ACCORDANCE WITH MNDOT 2360.
- COMPACTED THICKNESS FOR EACH LIFT AND TOTAL THICKNESS SHALL BE WITHIN 1/4 INCH AS SHOWN ON THE PLANS. TEST FINISHED SURFACE OF EACH ASPHALT COURSE FOR SMOOTHNESS. USING A TEN FOOT STRAIGHT EDGE APPLIED AT RIGHT ANGLES IN PAVED AREAS.
- 2.1.4. CHECK SURFACE AREAS AT INTERVALS AS DIRECTED BY THE OWNER'S AUTHORIZED REPRESENTATIVE.
- 2.1.5. MAXIMUM VARIATION OF FLATNESS IS 1/4 INCH MEASURED WITH A TEN FOOT STRAIGHT EDGE.

2.2. FLOOD TEST:

- 2.2.1. FLOOD TEST SHALL BE WITNESSED BY OWNER'S REPRESENTATIVE AND/OR ENGINEER.
- AFTER COMPLETION, FLOOD THE ENTIRE ASPHALT PAVED AREAS WITH WATER BY USE OF A TANK TRUCK OR HOSES.
- IF A DEPRESSION IS FOUND WHERE WATER PONDS TO A DEPTH OF MORE THAN 1/8 INCH, FILL DEPRESSIONS OR OTHERWISE CORRECT TO PROVIDE PROPER DRAINAGE.
- 2.2.4. FEATHER AND SMOOTH THE EDGES OF FILL SO THAT THE JOINT BETWEEN THE FILL AND THE ORIGINAL SURFACE IS INVISIBLE. 2.2.5. IF TESTS INDICATE THAT THE WORK DOES NOT MEET THE SPECIFIED REQUIREMENTS, REMOVE THE WORK, REPLACE AND RETEST
- AT NO ADDITIONAL COST TO THE OWNER. 2.2.6. TAKE ASPHALT MIXTURE SAMPLES AND PERFORM TESTS IN ACCORDANCE WITH ASPHALT INSTITUTE MS-2.

- 2.3.1. MINIMUM ACCEPTABLE DENSITY OF IN-PLACE COURSE MATERIAL IS 95 PERCENT OF THE RECORDED LABORATORY SPECIMEN
- 2.3.2. IN-PLACE COMPACTED THICKNESS WILL NOT BE ACCEPTED IF EXCEEDING THE FOLLOWING ALLOWABLE VARIATION FROM THICKNESS
- BASE COURSE: MINUS 1/4 INCH.

TOTAL THICKNESS: MINUS 1/4 INCH

- SURFACE COURSE: MINUS 1/4 INCH.
- 3. TRENCHING, BACKFILL AND GRADING

INDICATED.

- 3.1. COMPACTION TESTING FREQUENCY
- 3.1.1. PARKING LOTS: ONE PER LIFT PER 5,000 SQUARE FEET BUT NOT LESS THAN THREE PER LIFT TOTAL.
- 3.1.2. ROADWAYS, DRIVES, AND TRAILS:
- ONE PER LIFT PER LANE AT FIFTY FOOT INTERVALS BUT NOT LESS THAN TWO PER LIFT TOTAL. 3.1.3. STRUCTURES:
- 3.1.3.1. ONE PER LIFT PER 2,500 SQUARE FEET BUT NOT LESS THAN THREE PER LIFT TOTAL.
- 3.1.4. FOUNDATION TRENCHES: 3 1 4 1 ONE PER LIFT AT 25 FOOT INTERVALS
- 3.1.5. LAWNS, OPEN AREAS: 3.1.5.1. ONE PER LIFT PER 10,000 SQUARE FEET BUT NOT LESS THAN THREE PER LIFT TOTAL.
- 3.2. COMPACTION SHALL BE NOT LESS THAN THE FOLLOWING PERCENTAGES OF MAXIMUM DRY DENSITY AS DETERMINED BY THE STANDARD
- PROCTOR TEST (ASTM D698), OR EQUIVALENT DENSITY STANDARD DETERMINED BY OTHER METHOD(S).
- 3.2.1. ALL SUBSOIL, EXCEPT THE UPPER THREE FEET AND EMBANKMENTS FOR STRUCTURES AND PAVEMENTS: 95 PERCENT. 3.2.2. EMBANKMENTS FOR STRUCTURES AND PAVEMENTS, EXCEPT THE UPPER ONE FOOT AND SUBSOIL BENEATH PAVEMENTS AND
- 3.2.3. UPPER ONE FOOT OF EMBANKMENTS FOR STRUCTURES AND ALL SURFACE COMPACTION BENEATH FOOTINGS, STRUCTURES AND

PAVEMENTS: 100 PERCENT

3.3. TOLERANCES

SLABS ONE FOOT TO THREE FEET IN DEPTH: 98 PERCENT

- 3.3.1. PONDS AND BASINS
- 3.3.1.1. SUBGRADE AND FINISHED SURFACES: 0.10 FEET. THICKNESS OF SOIL LINERS: 0.05 FEET.
- THICKNESS OF SAND CUSHIONS: 0.10 FEET. 3.3.1.4. THICKNESS OF DRAINAGE LAYER: 0.10 FEET.
- 3.3.2. DITCHES, SWALES, AND DIVERSIONS
- SUBGRADE SURFACES: 0.10 FEET.
- 3.3.2.2. FINISHED SURFACES: 0.05 FEET. DEVIATION FROM DESIGN SLOPE: NOT GREATER THAN 0.2 PERCENT IN 25 FEET. 3.3.2.3.
- 3.3.3. STRUCTURES AND SLABS ON GRADE
- SUBGRADE SURFACES: MINUS 0.10 FEET TO PLUS 0.05 FEET. THICKNESS OF SAND COVER: 0.05 FEET. 3.3.3.2.
- 3.3.4. ASPHALT AND EXTERIOR CONCRETE PAVEMENTS

SUBGRADE SURFACES: 0.10 FEET.

- 3.3.4.1. SUBGRADE SURFACES: MINUS 0.10 FEET TO PLUS 0.05 FEET. THICKNESS OF GRANULAR MATERIALS: 0.05 FEET.
- DEVIATION FROM DESIGN SLOPE: NOT GREATER THAN 0.2 PERCENT IN FIFTY FEET. 3.3.4.3. 3.3.5. LAWNS AND OPEN AREAS

3.3.5.1.

3.3.5.2. THICKNESS OF TOPSOIL: 0.05 FEET. 3.3.5.3. DEVIATION FROM DESIGN SLOPE: NOT GREATER THAN 0.4 PERCENT IN FIFTY FEET.

AWALL PROJ

JOB No: 161080 DATE: 09/22/2017 DRAWN BY: CJO DESIGNED BY: ECR SHEET:

- 1.1. APPLY BITUMINOUS TACK COAT ONLY WHEN THE AMBIENT TEMPERATURE IS AT LEAST FIFTY DEGREES FAHRENHEIT AND WHEN THE
- TEMPERATURE HAS NOT BEEN BELOW THIRTY-FIVE DEGREES FAHRENHEIT FOR TWELVE HOURS IMMEDIATELY PRIOR TO APPLICATION. 1.2. DO NOT APPLY MATERIALS WHEN THE SUBBASE OR BASE SURFACE IS WET OR CONTAINS AN EXCESS OF MOISTURE WHICH WOULD PREVENT
- 1.3. PLACE ASPHALT PAVING ONLY WHEN ATMOSPHERIC TEMPERATURE IS ABOVE 45 DEGREE FAHRENHEIT, WHEN THE UNDERLYING BASE IS DRY AND WHEN WEATHER IS NOT RAINY.
- 1.4. THE CONTRACTOR SHALL PROTECT ALL ADJACENT CONCRETE SURFACES FROM CHIPPING, STAINING, DISCOLORATION, AND ANY OTHER DEFECTS OR DAMAGE DURING ASPHALT PAVING OPERATIONS.
- 2. PLACING THE MIX MULTIPLE COURSES:
- 2.1. PLACE ASPHALT MIXTURE ON PREPARED SURFACES, SPREAD AND STRIKE-OFF USING PAVING MACHINE.
- 2.2. INACCESSIBLE AND SMALL AREAS MAY BE PLACED BY HAND.
- 2.3. PLACE EACH COURSE AT THICKNESS SO THAT WHEN COMPACTED IT WILL CONFORM TO THE INDICATED GRADE, CROSS—SECTION, FINISH THICKNESS, TOLERANCE AND DENSITY INDICATED.
- 2.4. UNLESS OTHERWISE DIRECTED, BEGIN PLACING BASE COURSE AT HIGH SIDE OF SECTION ON ONE-WAY SLOPE.

UNIFORM DISTRIBUTION OF BITUMINOUS TACK COAT AND THE REQUIRED COMPACTION OF ASPHALT PAVING.

- 2.5. AFTER FIRST STRIP HAS BEEN PLACED AND ROLLED, PLACE SUCCEEDING STRIPS.
- 2.6. COMPLETE BASE COURSES FOR A SECTION BEFORE PLACING WEARING COURSES.
- 2.7. PLACE MIXTURE IN CONTINUOUS OPERATION AS PRACTICABLE.
- 2.8. PLACE TACK COAT BEFORE PLACING WEAR COURSE.
- 2.9. BASE COURSE SHALL BE SWEPT AS REQUIRED BEFORE PLACING WEAR COURSE.
- 2.10. THE HEATED SCREED OR STRIKE-OFF ASSEMBLY SHALL PRODUCE A FINISHED SURFACE OF THE REQUIRED EVENNESS AND TEXTURE WITHOUT TEARING. SHOVING OR GOUGING.
- 2.11. HEATED SCREED EXTENSIONS ARE REQUIRED IF THE PAVING WIDTH ON EITHER SIDE OF THE PAVER IS GREATER THAN THE BASIC SCREED. 2.12. STRIKE-OFF ONLY EXTENSIONS ASSEMBLIES ARE NOT ALLOWED FOR WEAR COURSE PAVING.
- 2.13. SPREAD, TAMP AND FINISH MIXTURE USING HAND GUIDED MECHANICAL TOOLS IN AREAS WHERE MACHINE SPREADING IS NOT POSSIBLE, AS
- ACCEPTABLE TO THE OWNER'S AUTHORIZED REPRESENTATIVE. 2.14. PLACE MIXTURE AT A RATE THAT WILL ENSURE HANDLING AND COMPACTION BEFORE MIXTURE BECOMES COOLER THAN ACCEPTABLE WORKING
- 2.15. GRADUALLY MAKE JOINTS BETWEEN OLD AND NEW PAVEMENTS, OR BETWEEN SUCCESSIVE DAY'S WORK, TO ENSURE A CONTINUOUS BOND
- BETWEEN ADJOINING WORK. 2.16. CONSTRUCT JOINTS TO HAVE SAME TEXTURE, DENSITY AND SMOOTHNESS AS ADJACENT SECTIONS OF ASPHALT COURSE.
- 2.17. CLEAN CONTACT SURFACES FREE OF SAND, DIRT OR OTHER OBJECTIONABLE MATERIAL AND APPLY TACK COAT.
- 2.18. OFFSET TRANSVERSE JOINTS IN SUCCEEDING COURSES NOT LESS THAN FIVE FEET.
- 2.19. CUT BACK EDGE OF PREVIOUSLY PLACED COURSE TO EXPOSE AN EVEN, VERTICAL SURFACE FOR FULL COURSE THICKNESS.
- 2.20. OFFSET LONGITUDINAL JOINTS IN SUCCEEDING COURSES NO LESS THAN SIX INCHES.
- 2.21. WHEN THE EDGES OF LONGITUDINAL JOINTS ARE IRREGULAR, HONEYCOMBED, OR INADEQUATELY COMPACTED, CUT BACK UNSATISFACTORY SECTION TO EXPOSE AN EVEN, VERTICAL SURFACE FOR THE FULL COURSE THICKNESS.
- 3. COMPACTING THE MIX 3.1. PROVIDE SUFFICIENT NUMBER OF ROLLERS TO OBTAIN THE REQUIRED MINIMUM PAVEMENT DENSITY OF NINETY-FIVE PERCENT OF THE RECORDED LABORATORY SPECIMEN DENSITY.
- 3.2. BEGIN ROLLING OPERATIONS AS SOON AFTER PLACING MIX WHEN THE MIXTURE WILL BEAR WEIGHT OF ROLLER WITHOUT DISPLACEMENT. 3.3. DO NOT PERMIT HEAVY EQUIPMENT, INCLUDING ROLLERS, TO STAND ON FINISHED SURFACE BEFORE IT HAS THOROUGHLY COOLED OR SET.
- 3.4. COMPACT MIXTURE WITH HOT HAND TAMPERS OR VIBRATING PLATE COMPACTORS IN AREAS INACCESSIBLE TO ROLLERS.
- 3.5. ROLL TO SLIGHTLY DIFFERENT LENGTHS ON ALTERNATE ROLLER RUNS.
- 3.6. DO NOT ROLL CENTERS OF SECTION FIRST UNDER ANY CIRCUMSTANCES.
- 3.7. ACCOMPLISH BREAKDOWN OR INITIAL ROLLING IMMEDIATELY FOLLOWING ROLLING OF TRANSVERSE AND LONGITUDINAL JOINTS AND OUTSIDE EDGE.
- 3.8. OPERATE ROLLERS AS CLOSE AS POSSIBLE TO PAVEMENT OR PAVERS WITHOUT CAUSING DISPLACEMENT.
- 3.9. CHECK CROWN, GRADE AND SMOOTHNESS AFTER BREAKDOWN ROLLING.
- 3.10. REPAIR DISPLACED AREAS BY LOOSENING AT ONCE WITH LUTES OR RAKES AND FILLING, IF REQUIRED, WITH HOT LOOSE MATERIAL BEFORE
- 3.11. FOLLOW BREAKDOWN ROLLING WITH SECOND ROLLING AS SOON AS POSSIBLE, WHILE MIXTURE IS HOT AND IN CONDITION FOR COMPACTION. 3.12. CONTINUE SECOND ROLLING UNTIL MIXTURE HAS BEEN THOROUGHLY COMPACTED.
- 3.13. A PNEUMATIC TIRED ROLLER SHALL BE USED FOR FINISH ROLLING.
- 3.14. PERFORM FINISH ROLLING WHILE MIXTURE IS STILL WARM ENOUGH FOR REMOVAL OF ROLLER MARKS.
- 3.15. CONTINUE ROLLING UNTIL ROLLER MARKS ARE ELIMINATED AND COURSE HAS ATTAINED SPECIFIED DENSITY. 3.16. REMOVE AND REPLACE DEFECTIVE AREAS.
- 3.16.1. CUT-OUT AND FILL WITH FRESH, HOT ASPHALT CONCRETE.
- 3.16.2. REMOVE DEFICIENT AREAS FOR FULL DEPTH OF COURSE. 3.17. CUT SIDES PERPENDICULAR AND PARALLEL TO DIRECTION OF TRAFFIC WITH EDGES VERTICAL.
- 3.18. COMPACT BY ROLLING TO SPECIFIED SURFACE DENSITY AND SMOOTHNESS.
- 3.19. BITUMINOUS PATCHING OF THE BASE COURSE SHALL BE COMPLETED AT LEAST 24 HOURS PRIOR TO THE PLACEMENT OF THE WEAR
- COURSE IN ORDER TO PROVIDE BASE COURSE STABILITY BY ALLOWING THE BASE COURSE TO COOL TO AMBIENT TEMPERATURE. 3.20. APPLY TACK COAT TO EXPOSED SURFACES BEFORE PLACING NEW ASPHALT CONCRETE MIXTURE.
- 4. MAINTENANCE HOLE AND GATE VALVE PROTECTION
- 4.1. COVER MAINTENANCE HOLES, CATCH BASINS AND GATE VALVES WITHIN THE SURFACE TO BE SEALED TO PROHIBIT THE BITUMINOUS MATERIAL

6. CLEANING

- FROM BEING PLACED THEREON.
- 4.2. CLEAN THE SURFACE OF THESE STRUCTURES FOLLOWING THE APPLICATION OF THE COVER AGGREGATE. 5. ADJUSTING CASTINGS
- 5.1. CASTINGS SHALL BE RAISED AFTER THE BITUMINOUS BASE COURSE IS PLACED AND PRIOR TO INSTALLING THE WEAR COURSE.
- 5.2. CASTINGS FOR CATCH BASINS IN CURB SHALL BE ADJUSTED WHEN THE CONCRETE CURB AND GUTTER IS CONSTRUCTED.
- 5.3. THE BITUMINOUS BASE SHALL BE SAWCUT AROUND THE COVER PLATE.
- 5.4. THE BITUMINOUS AGGREGATE AND COVER PLATE SHALL BE REMOVED.
- 5.5. ALL EXCAVATIONS SHALL BE OVERSIZED ON ALL SIDES, AT LEAST ONE FOOT GREATER THAN THE ITEM TO BE ADJUSTED TO PROVIDE FOR MECHANICAL COMPACTION OF BACKFILLED MATERIALS.
- 5.6. EXCAVATION AND PATCH WORK SHALL BE CIRCULAR IN NATURE TO AVOID SQUARE CORNERS AND REDUCE PAVEMENT CRACKING.
- 5.7. THE CASTING SHALL BE SET TO FINAL GRADE USING ADJUSTING RINGS AND MORTAR.
- 5.8. THE AGGREGATE BASE SHALL BE PLACED TO THE DESIGN DEPTH AROUND THE CASTING.
- 5.9. THE MIX SHALL BE PLACED AND TAMPED AROUND THE CASTING TO BRING THE GRADE UP TO THE SURFACE OF THE BITUMINOUS BASE.
- 5.10. THE FINAL GRADE OF CASTINGS IN PAVED AREAS SHALL BE 1/4 INCH TO 1/2 INCH BELOW THE TOP OF THE COMPLETED WEAR COURSE. 5.11. THE CASTINGS SHALL BE SET TO THE CONTOUR OF THE FINISHED SURFACE SO THAT THE REQUIRED TOLERANCE IS UNIFORM AROUND THE
- CIRCUMFERENCE OF THE CASTING. 5.12. THE TOLERANCE SHALL BE MEASURED AT THE IMMEDIATE EDGE OF THE CASTING AND NO "STRAIGHTEDGE" MEASUREMENTS SHALL BE
- 5.13. A PLYWOOD TEMPLATE, 1/2 INCH THICK, SHALL BE FASTENED TO THE TOP OF ALL NON-ADJUSTABLE CASTINGS DURING PLACING AND
- ROLLING OF THE WEAR COURSE TO ENSURE THAT THE REQUIRED TOLERANCES ARE MET. 5.14. ALL FINAL ADJUSTMENTS TO THE ADJUSTABLE CASTINGS SHALL BE MADE BY MEANS OF THE CASTING ADJUSTMENT BOLTS.
- 5.15. ALL CASTINGS, WHICH DO NOT MEET THE REQUIRED TOLERANCES, SHALL BE REMOVED AND RE-ADJUSTED AT THE CONTRACTOR'S EXPENSE.
- 6.1. AFTER COMPLETION OF PAVING OPERATIONS, CLEAN SURFACES OF EXCESS OR SPILLED ASPHALT MATERIALS TO THE SATISFACTION OF THE OWNER.
- 6.2. IMMEDIATELY AFTER PLACEMENT OF ASPHALTIC PAVING, PROVIDE TRAFFIC CONES, BARRICADES AND OTHER DEVICES NEEDED TO PROTECT PAVEMENT FROM MECHANICAL INJURY FOR A MINIMUM OF 24 HOURS.

CONCRETE PLACEMENT:

- 1. ENVIRONMENTAL REQUIREMENTS
- 1.1. DURING COLD WEATHER, PLACE AND PROTECT CONCRETE IN ACCORDANCE WITH ACI 306.
- 1.2. DURING HOT WEATHER, PLACE AND PROTECT CONCRETE IN ACCORDANCE WITH ACI 305
- 1.3. DO NOT PLACE CONCRETE IN RAIN, SLEET, OR SNOW WITHOUT PROVIDING ADEQUATE PROTECTION.
- 1.4. CLEANING OF EQUIPMENT AND DISPOSAL OF EXCESS CONCRETE SHALL BE PER BEST MANAGEMENT PRACTICES (BMPS).
- 2. CASTINGS 2.1. CASTINGS IN CONCRETE AREAS SHALL BE RAISED PRIOR TO THE PLACEMENT OF CONCRETE
- 2.2. CASTINGS FOR CATCH BASINS IN CURB SHALL BE ADJUSTED WHEN THE CONCRETE AND GUTTER IS CONSTRUCTED 2.3. THE FINAL GRADE OF CASTINGS IN PAVED AREAS SHALL BE 1/4 INCH TO 1/2 BELOW THE TOP OF THE COMPLETED CONCRETE.
- 3. FORMING 3.1. PLACE AND SECURE FORMS TO CORRECT LOCATION, DIMENSION, AND PROFILES
- 3.2. ASSEMBLE FORMWORK TO PERMIT EASY STRIPING AND DISMANTLING WITHOUT DAMAGING CONCRETE
- 3.3. PLACE JOINT FILLER IN VERTICAL POSITION, IN STRAIGHT LINES. SECURE TO FORMWORK DURING CONCRETE PLACEMENT 3.4. INSTALL FORMS TO ALL CONTINUOUS PROGRESS OF WORK AND SO THAT FORMS CAN REMAIN IN PLACE AT LEAST 24 HOURS AFTER CONCRETE PLACEMENT.

- 4.1. PLACE REINFORCEMENT AS INDICATED ON THE PLANS.
- 4.2. INTERRUPT REINFORCEMENT AT EXPANSION JOINTS, UNLESS NOTED OTHERWISE.
- 4.3. PLACE DOWELS AND REINFORCEMENT TO ACHIEVE PAVEMENT AND CURB ALIGNMENT AS DETAILED ON THE PLANS.
- 4.4. PROVIDE DOWELED JOINTS 24 INCHES ON CENTER AT INTERRUPTIONS OF CONCRETE WITH ONE END OF DOWEL SET IN CAPPED SLEEVE TO ALLOW LONGITUDINAL MOVEMENT.

5. MAINTENANCE HOLE AND GATE VALVE PROTECTION

- 5.1. COVER MAINTENANCE HOLES, CATCH BASINS AND GATE VALVES LYING WITHIN THE SURFACE TO BE SEALED TO AS TO PROHIBIT THE CONCRETE MATERIAL FROM BEING PLACED THEREON.
- 5.2. CLEAN THE SURFACE OF THESE STRUCTURES FOLLOWING THE APPLICATION OF THE COVER AGGREGATE.

- 6.1. PLACE CONCRETE IN ACCORDANCE WITH ACI 301 AND COMPLY WITH THE REQUIREMENTS FOR MIXING AND PLACING CONCRETE AS HEREIN SPECIFIED.
- 6.2. ENSURE REINFORCEMENT, INSERTS, EMBEDDED PARTS AND FORMED JOINTS ARE NOT DISTURBED DURING CONCRETE PLACEMENT.
- 6.3. INSTALL SAFETY STAIR NOSINGS TO BE CAST INTO ALL EXTERIOR POURED CONCRETE STAIR TREADS.
- 6.4. PLACE CONCRETE CONTINUOUSLY BETWEEN PREDETERMINED CONSTRUCTION JOINTS.
- 6.5. DO NOT BREAK OR INTERRUPT SUCCESSIVE POURS SUCH THAT COLD JOINTS OCCUR.
- 6.6. PLACE CONCRETE TO PATTERN INDICATED ON THE PLANS OR AS DIRECTED BY THE OWNER'S AUTHORIZED REPRESENTATIVE. 6.7. PLACE CONCRETE BY METHODS THAT PREVENT SEGREGATION OF THE MIX.
- 6.8. CONSOLIDATE CONCRETE ALONG THE FACE OF THE FORMS AND ADJACENT TO TRANSVERSE JOINTS WITH INTERNAL VIBRATOR.
- 6.9. KEEP VIBRATOR AWAY FROM JOINT ASSEMBLIES, REINFORCEMENT AND SIDE FORMS.

- 7.1. EXPANSION JOINTS SHALL BE FILLED WITH 1/2 INCH THICK PREFORMED JOINT FILLER MATERIAL AND SHALL BE INSTALLED IN THE FOLLOWING LOCATIONS
- 7.1.1. CURB AND GUTTER
- 7.1.1.1. AT THE BEGINNING AND END OF ALL CURVED SECTIONS
- AT SIXTY FEET MAXIMUM SPACING
- AT TEN FEET EACH SIDE OF CATCH BASINS
- 7.1.2. SIDEWALK AT THE THE BEGINNING AND END OF ALL CURVED SECTIONS
- 7.1.2.2. AT SIXTY FEET MAXIMUM SPACING
- 7.1.2.3. AT DRIVEWAYS
- 7.1.2.4. AT CURBS 7.1.3. WHERE ALL NEW CONCRETE SURROUNDS, ADJOINS OR ABUTS ANY EXISTING OR NEW FIXED OBJECTS, SUCH AS FIRE
- 7.2. CONTRACTION JOINTS
- 7.2.1. SHALL BE 1/3 OR MORE THE DEPTH OF THE CONCRETE CURBING SHALL BE PROVIDED WITH CONTRACTION JOINTS TEN FEET ON CENTER MINIMUM OR AS INDICATED ON THE PLANS
- SIDEWALKS SHALL BE PROVIDED WITH CONTRACTION JOINTS MATCHING THE WIDTH OF THE SIDEWALK OR AS INDICATED ON THE PLANS

HYDRANTS, VALVE BOXES, MAINTENCE HOLES, LIGHT POLES, CURBS, WALKS, BUILDING FACE OR OTHER RIGID STRUCTURES.

TRUCK PARKING PADS, APRONS, AND DRIVEWAYS SLABS SHALL BE CUT TO UNIFORMS SECTIONS NOT GREATER THAN EIGHT 7.2.4.

7.2.5. ALL JOINTS SHALL BE VERTICAL AND STRAIGHT.

- 8. FINISHING 8.1. AFTER STRIKING OFF AND CONSOLIDATING CONCRETE, SMOOTH SURFACES BY SCREEDING AND FLOATING
- 8.2. AFTER FLOATING AND BEFORE FINAL FINISHING, CHECK THE CONCRETE WITH A TEN FOOT STEEL STRAIGHTEDGE TO ENSURE THERE IS NO VARIATION GREATER THAN 3/16 INCH FROM THE STRAIGHTEDGE
- 8.3. WORK EDGES OF SLABS, GUTTERS, BACK TOP EDGE OF CURBS AND FORMED JOINTS WITH AN EDGING TOOL, AND ROUND TO 1/2 INCH RADIUS, UNLESS OTHER WISE INDICATED
- 8.4. AFTER COMPLETION OF FLOATING AND WHEN EXCESS MOISTURE OR SURFACE SHEEN HAS DISAPPEARED, COMPLETE TROWLING AND
- FINISH SURFACES AS FOLLOWS: 8.4.1. MEDIUM BROOM CURBS AND GUTTERS
- 8.4.2. MEDIUM BROOM PERPENDICULAR TO SLOPE TRUCK PARKING PADS, APRONS, AND DRIVEWAYS
- 8.4.3. LIGHT BROOM SIDEWALKS AND STAIRS 8.4.4. MEDIUM BROOM PEDESTRIAN RAMPS

9. CONCRETE CURING

- 9.1. CURING SHALL BE PERFORMED BY APPLYING A MEMBRANE CURING COMPOUND TO THE EXPOSED SURFACE OF THE CONCRETE WITHIN ONE HOUR OF FINISHING THE CONCRETE
- 9.2. THE CURING COMPOUND SHALL BE APPLIED BY AN APPROVED AIRLESS SPRAYING MACHINE AT THE APPROXIMATE RATE OF ONE GALLON OF COMPOUND PER 150 SQUARE FEET OF SURFACE CURING AREA
- 9.3. IN ALL CASES, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE CONCRETE FROM EXCESSIVE HOT OR COLD TEMPERATURES, AND PEDESTRIANS DURING THE CURE PERIOD.

JOB No: 161080 DATE: 09/22/2017 DRAWN BY: CJO DESIGNED BY: ECR

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EM NO.	DESCRIPTION	ESTIMATED BASE QUANTITY	ESTIMATED ALT 1 QUANTITY	ESTIMATED ALT 2 QUANTITY	ESTIMATED DEDUCT 2 QUANTITY	UNIT	MN DOT SPEC#	BID UNIT PRICE	BASE BID PRICE	ALT 1 BID PRICE	ALT 2 BID PRICE	DEDUCT 2 BID PRICE
1	MOBILIZATION	1	0	0	0	LS	-					
2	TRAFFIC CONTROL	1	0	0	0	LS	-					
3	MAINTENANCE AND RESTORATION OF HAUL ROADS	1	0	0	0	LS	-					
5	TREE REMOVAL	2	8	0	0	EA	2101.507					
6	REMOVE CURB & GUTTER	0	1167	0	0	LF	2104.501					
7	SAWCUT CONCRETE WALK (FULL DEPTH)	40	4	0	0	LF	2104.511					
8	SAWCUT BITUMINUOUS PAVEMENT (FULL DEPTH)	0	97	0	0	LF	2104.513					
9	SAWCUT CONCRETE PAVEMENT (FULL DEPTH)	0	1036	0	0	LF	2104.511					
10	REMOVE BITUMINOUS PAVEMENT	0	32) 0	0	SY	2104.505					
11	REMOVE CONCRETE PAVEMENT	0	1394	0	0	SY	2104.505					
12	PAVEMENT MARKING REMOVAL	0	2	0	0	LS	2102					
13	BUILDING REMOVAL	1	0	0	0	EA	2103					
14		17084	461	380	0	SF	2104.503					
15	REMOVE WALKS REMOVE SHIP CLEAT AND IRVIN BOLLARDS	14 / 8	0	300	0		2104.509					
16	REMOVE STORM PIPE	20	21	0	0	EA	2104.501					
			21	0	0	LF 	2104.509					
17	REMOVE STORM STRUCTURE	0	0			EA	2104.504					
18	REMOVE WATER LIVER ANT	22	0	0	0	LF	2104.509					
19	REMOVE WATER HYDRANT	0	0	0	0	EA	2104.523					
20	SALVAGE ITEMS FOR STORAGE (BENCH, BELL, ANCHOR, ETC)	1	0	0	0	LS	2104.523 3352					
21	REMOVE AND RE-INSTALL SIGN	9	1	0	0	EA	2506					
22	ADJUST FRAME AND RING CASTING	A	2	0	0	EA	2104					
23	REMOVE ELECTRICAL APPURTENANCES	1	0	0	0	LS	2104					
24	REMOVE TELEPHONE APPURTENANCES	1	0	0	0	LS	2104.509					
25	REMOVE LIGHT POLE	1	0	0	0	LS	2104.509					
26	REMOVE AND REPLACE STEEL PLATE	2	0	0	0	EA	2104.509					
27	REMOVE CABLE FENCE	705	0	0	0	LF						
28	REMOVE PIPE FENCE	480		0	0	LF	2104.501					
29	CURB & GUTTER (B618)	0	1164	0	0	LF	2531					
30	8" WIDE CURB	0	0	0	0	LF	2531					
31	PEDESTRIAN RAMP	0	6	0	0	EA	2301					
32	CONCRETE WALK (4")	14074	1240	10700	0	SF	2531					
33	TRUNCATED DOME (WARNING PANEL)	0	48	0	0	SF	2531					
34	BITUMINOUS WEARING COURSE MIXTURE (3")	0	200	0	0	TON	2357, 3151					
35	REGULATORY SIGN (STOP, YIELD, ETC.)	0	1	0	0	EA	3352, 3401					
36	AGGREGATE BASE COURSE	75	249	223	0	CY	2105					
37	PEDESTRIAN CROSSING PAVEMENT MARKING	0	4	0	0	EA	2582, 2568					
38	STORM SEWER STRUCTURES (MH'S, CB'S)	0	1	0	0	EA	2506					
39	36" RCP STORM SEWER	0	17	0	0	LF	3235					
40	CONNECT TO EXISTING DRAINAGE STRUCTURE	0	1	0	0	EA	3145E					
41	PLANTER BOX-RETAINING WALL-STIARS	25	0	0	0	CY	-					
42	PAVEMENT SYMBOL (TRAFFIC ARROW)	0	8	0	0	EA	2582					
43	BIKE LANE STRIPE (6" SOLID WHITE)	0	1193	0	0	LF	2582					
44	BIKE LANE PAVEMENT MARKINGS	0	1	0	0	EA EA	2582					
45	SEDIMENT CONTROL LOG TYPE MNDOT	2167	0	0	0	LF	3897 2573.533					
	SOD SEDIMENT CONTROL LOG TYPE MINDOT	1172	0	0	670		3878					
46						SY	2574					
47	INLET PROTECTION NOTE: CONTRACTOR SHALL DETERMINE EARTH	2	0	0	0	EA						

 DECC SEAWALL PROJECT
 DATE:
 REV:
 DESCRIPTION

 CITY OF DULUTH
 10/25
 1
 BIDDING CLARIFICATION

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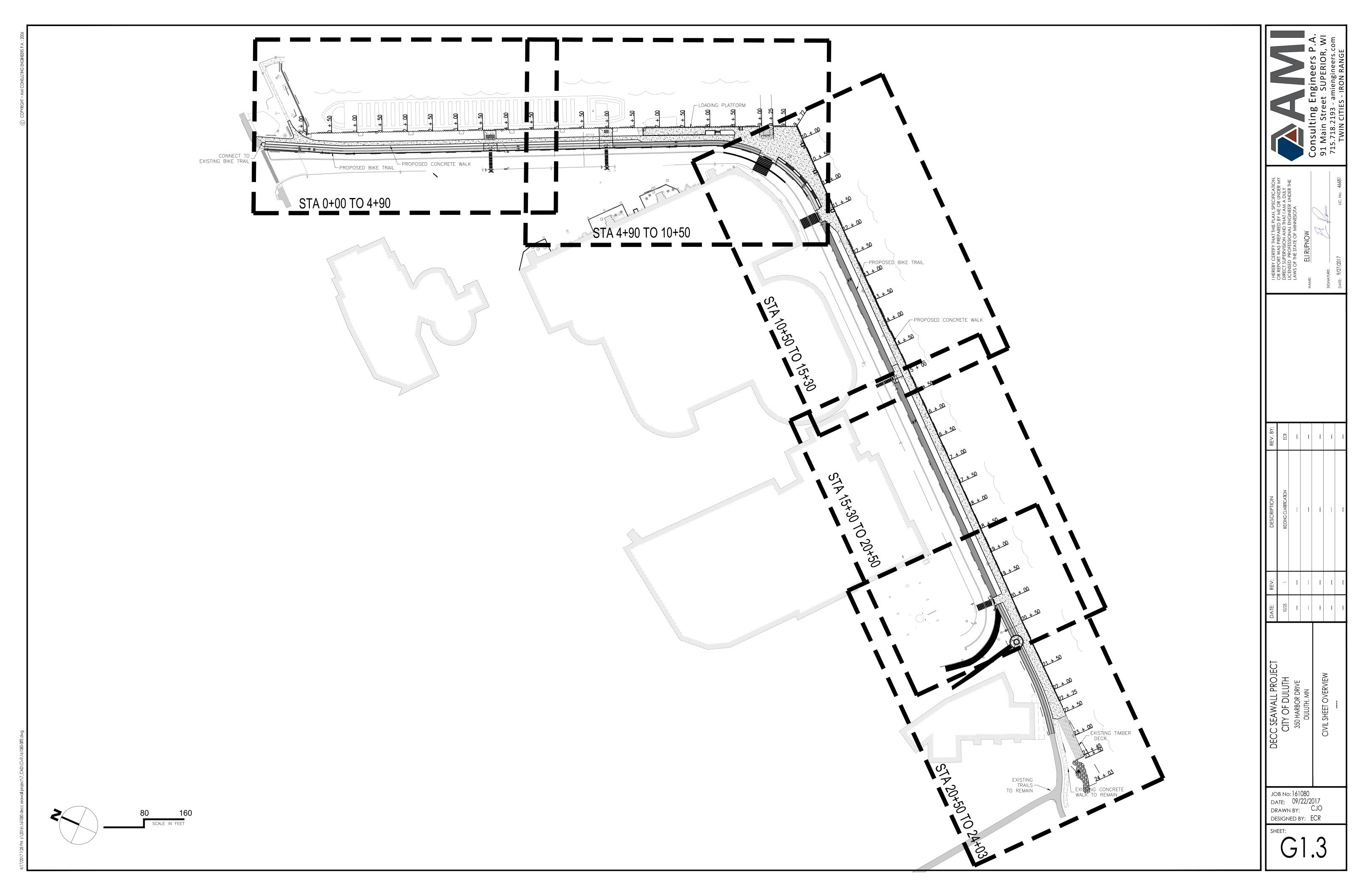
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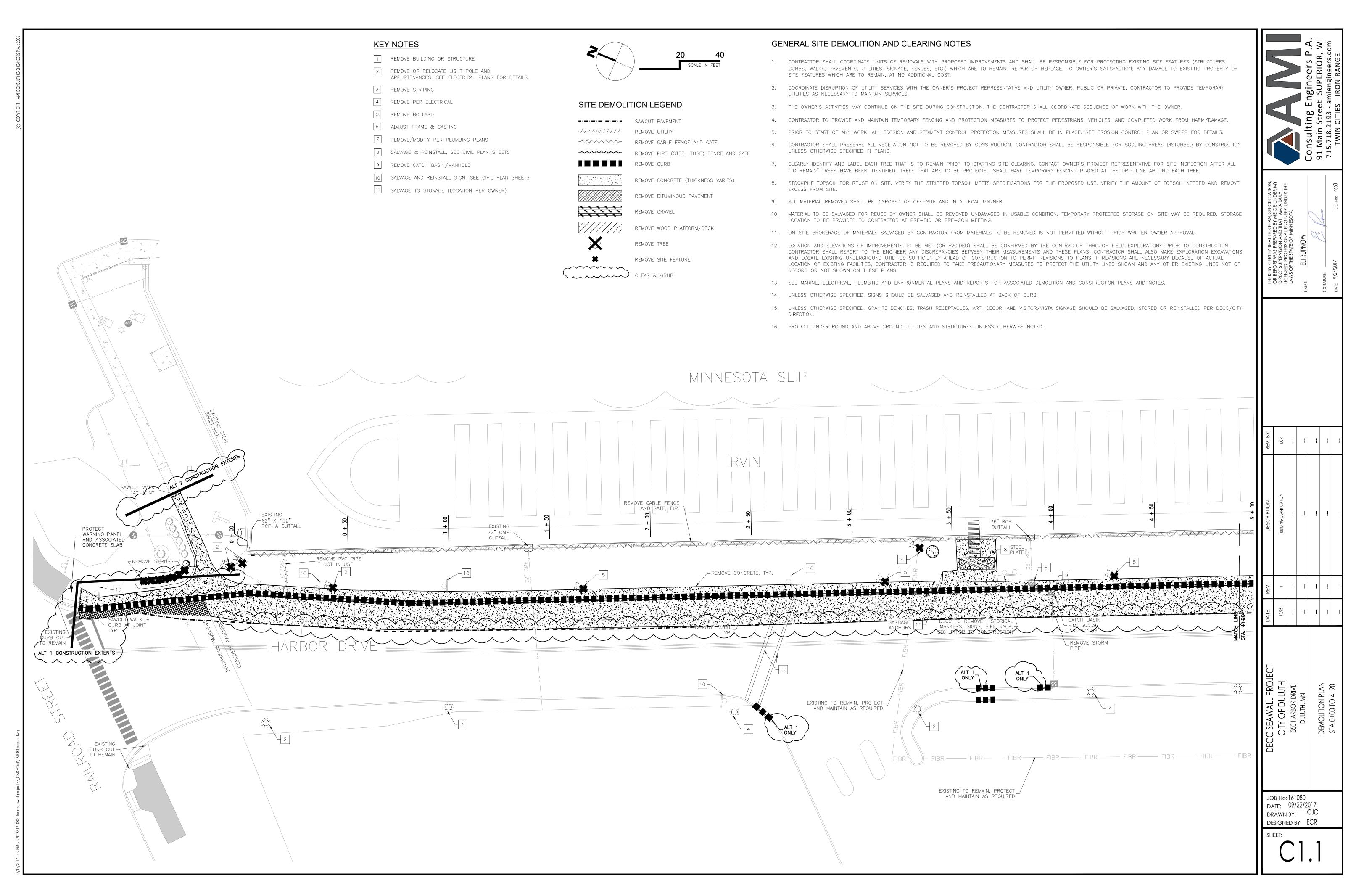
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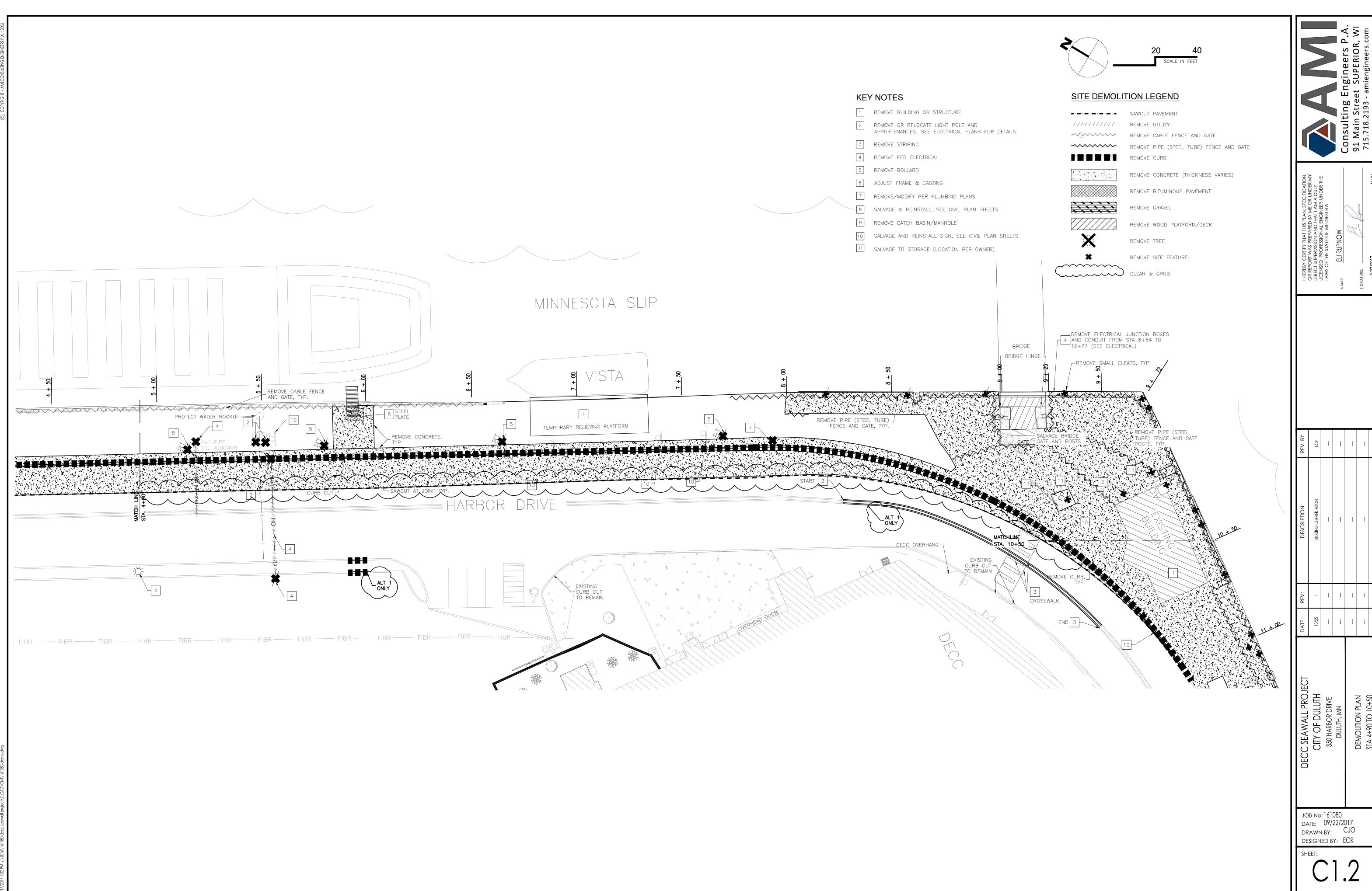
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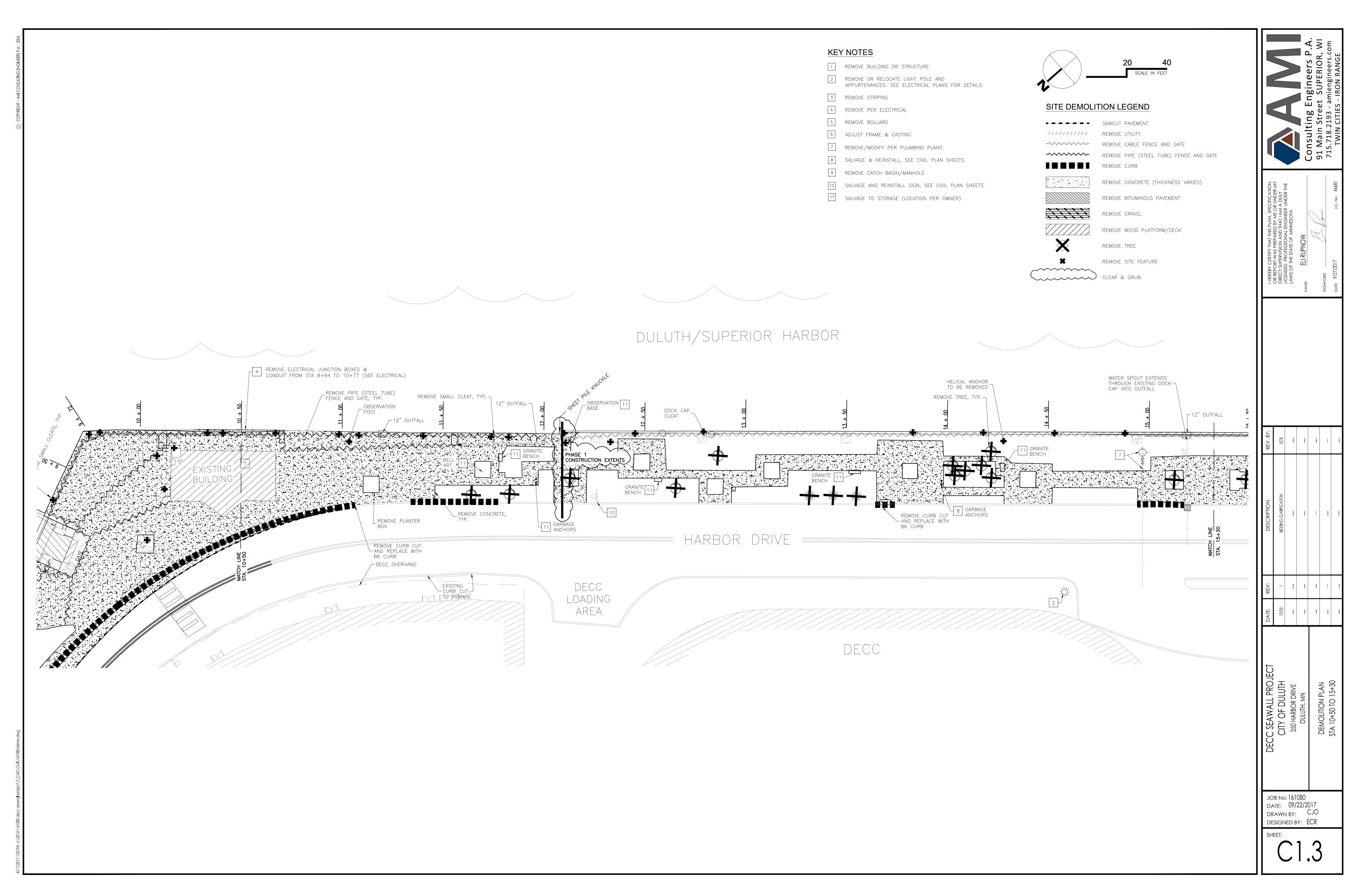
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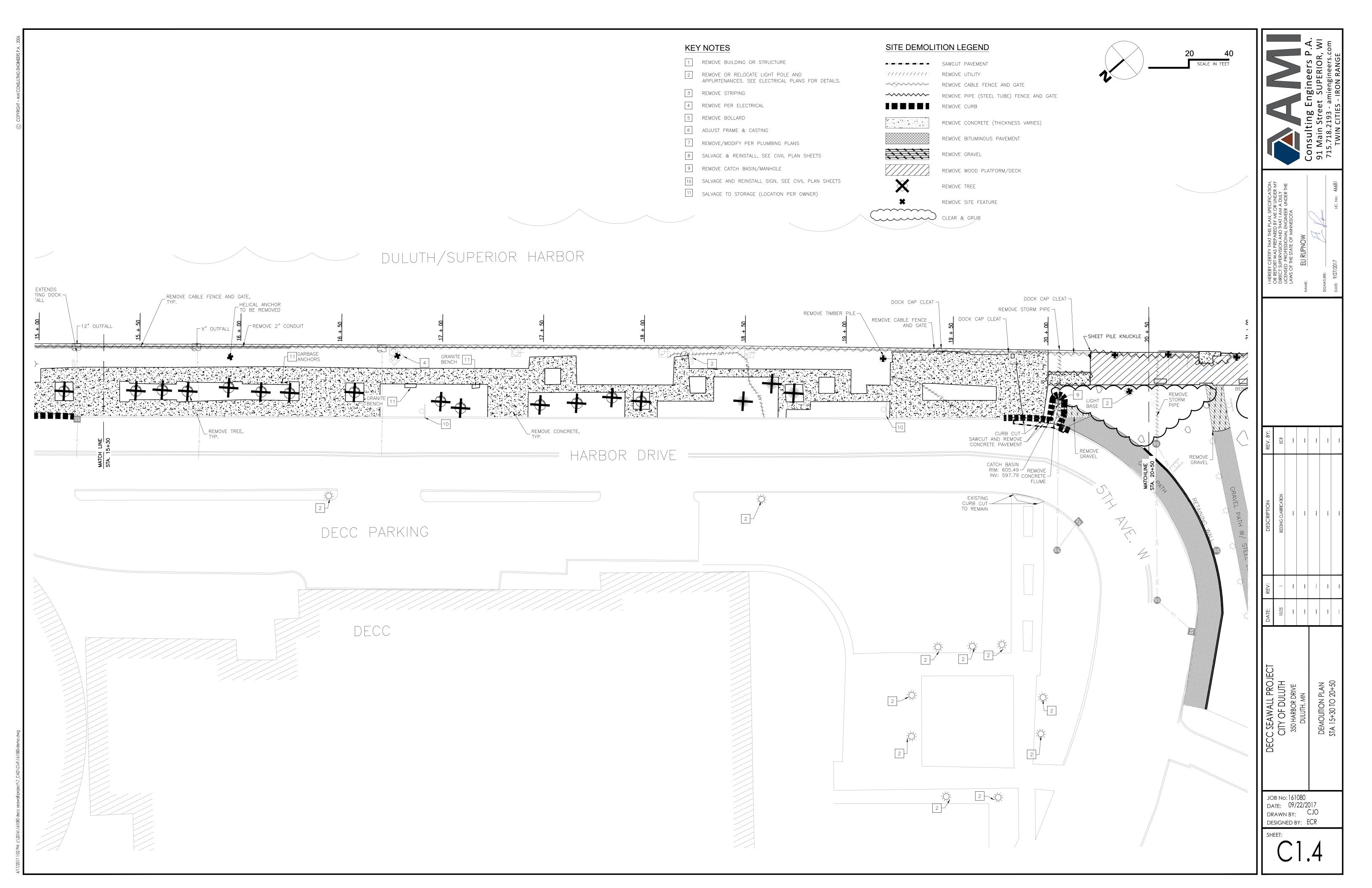
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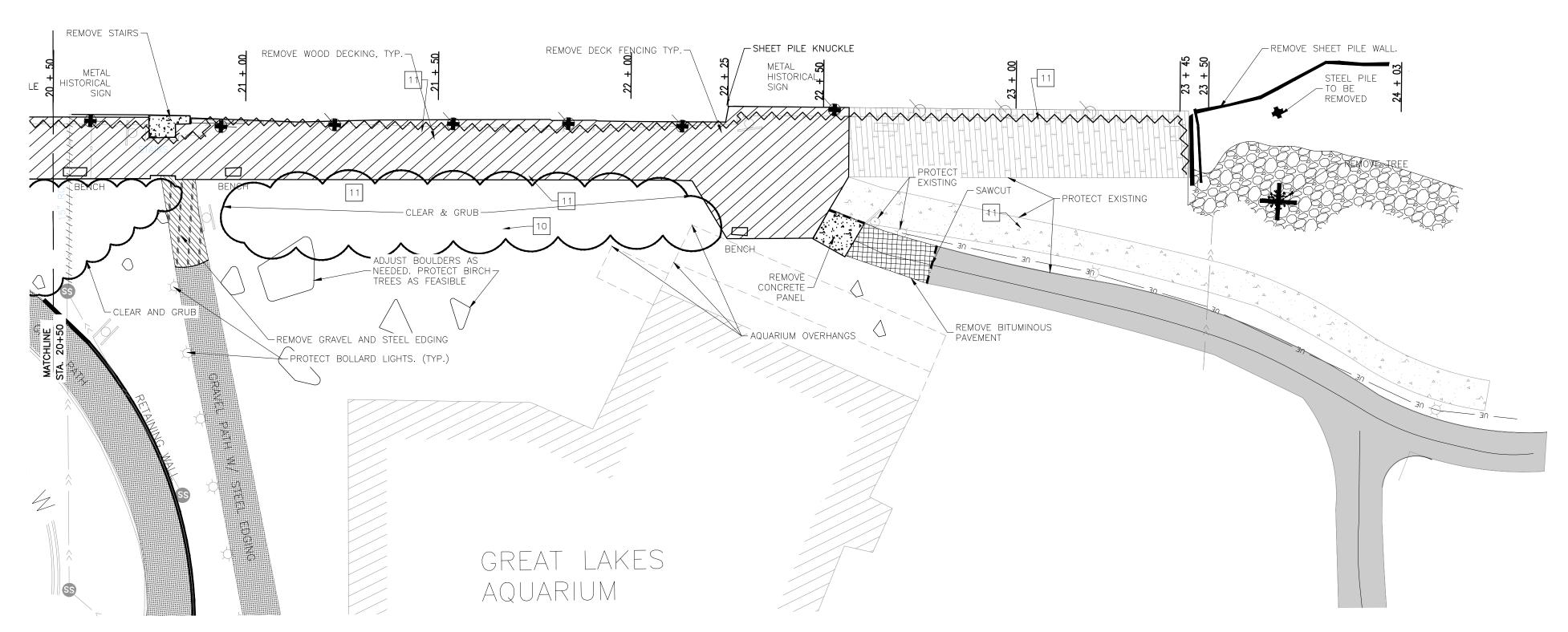






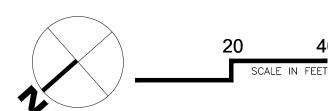


DULUTH/SUPERIOR HARBOR



KEY NOTES

- 1 REMOVE BUILDING OR STRUCTURE
- 2 REMOVE OR RELOCATE LIGHT POLE AND APPURTENANCES. SEE ELECTRICAL PLANS FOR DETAILS.
- AFFUNILINANCES. SEE ELECTRICAL
- 3 REMOVE STRIPING
 4 REMOVE PER ELECTRICAL
- 5 REMOVE BOLLARD
- 6 ADJUST FRAME & CASTING
- 7 REMOVE/MODIFY PER PLUMBING PLANS
- 8 SALVAGE & REINSTALL, SEE CIVIL PLAN SHEETS
- 9 REMOVE CATCH BASIN/MANHOLE
- 10 SALVAGE AND REINSTALL SIGN, SEE CIVIL PLAN SHEETS
- 11 SALVAGE TO STORAGE (LOCATION PER OWNER)



SITE DEMOLITION LEGEND

SAWCUT PAVEMENT

'/'/'/'/'/'/

REMOVE UTILITY

REMOVE CABLE FENCE AND GATE

REMOVE PIPE (STEEL TUBE) FENCE AND GATE

REMOVE CURB

REMOVE CONCRETE (THICKNESS VARIES)

REMOVE BITUMINOUS PAVEMENT

REMOVE GRAVEL

REMOVE WOOD PLATFORM/DECK
REMOVE TREE

REMOVE SITE FEATURE

CLEAR & GRUB

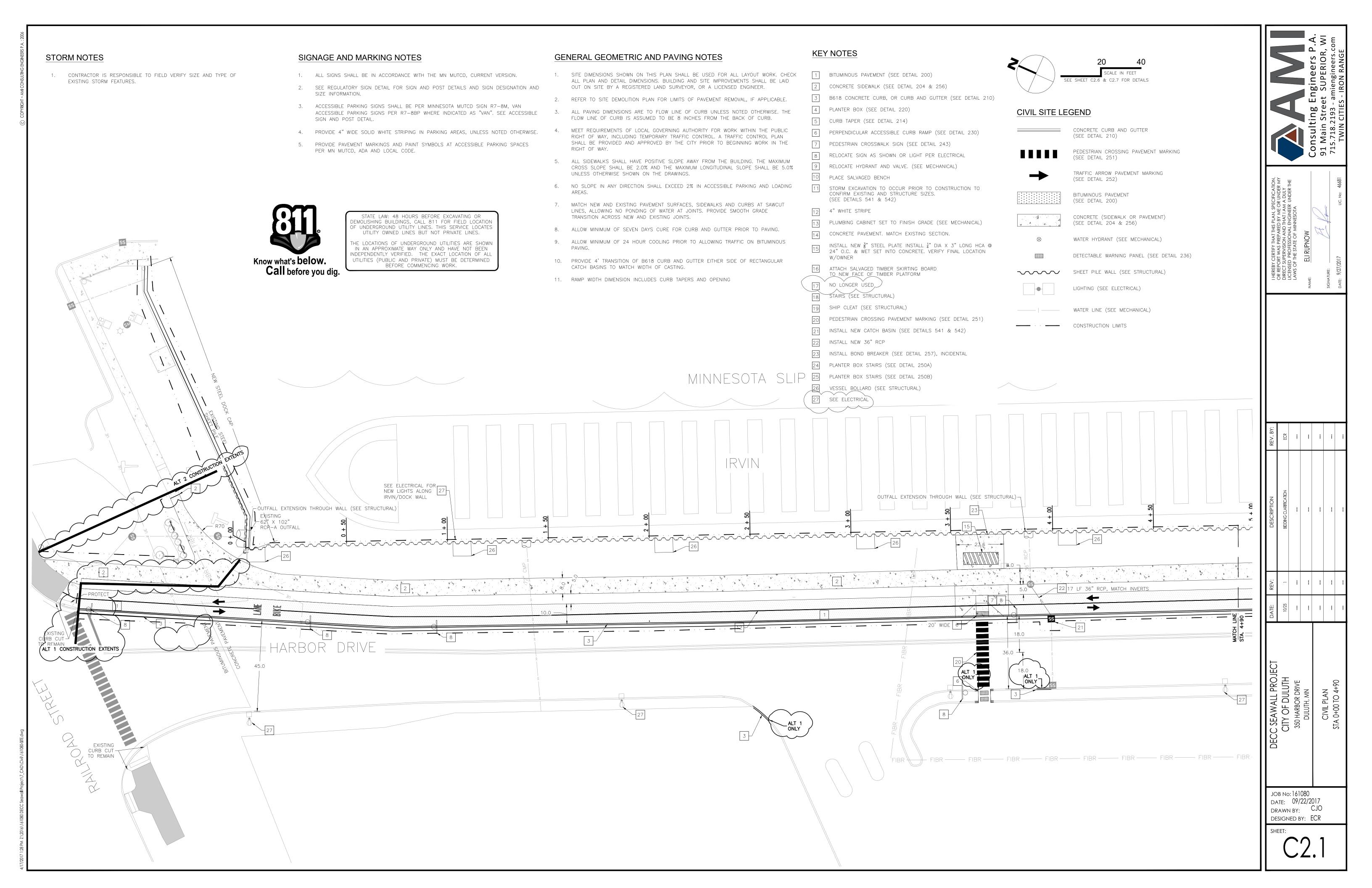
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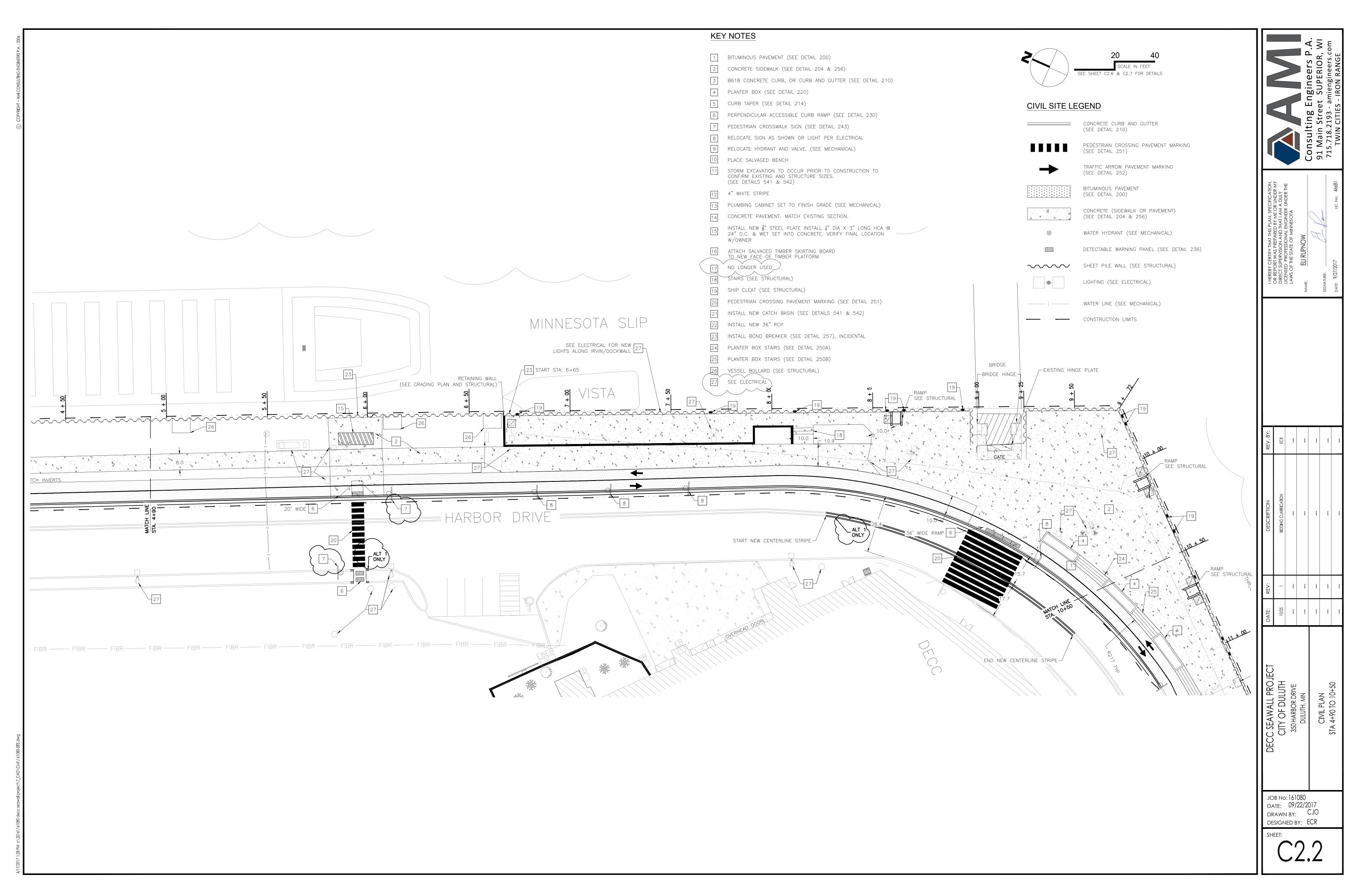
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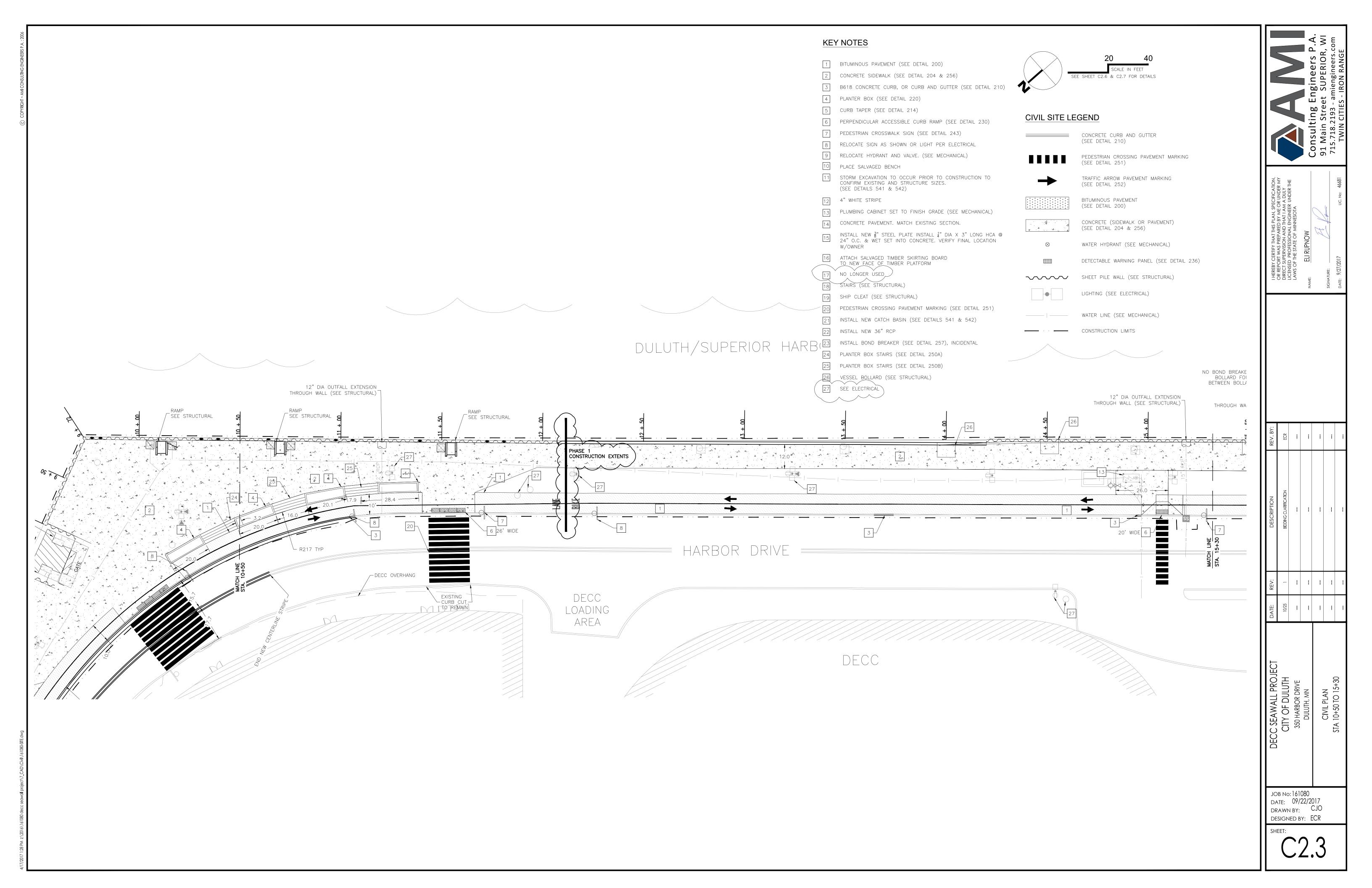
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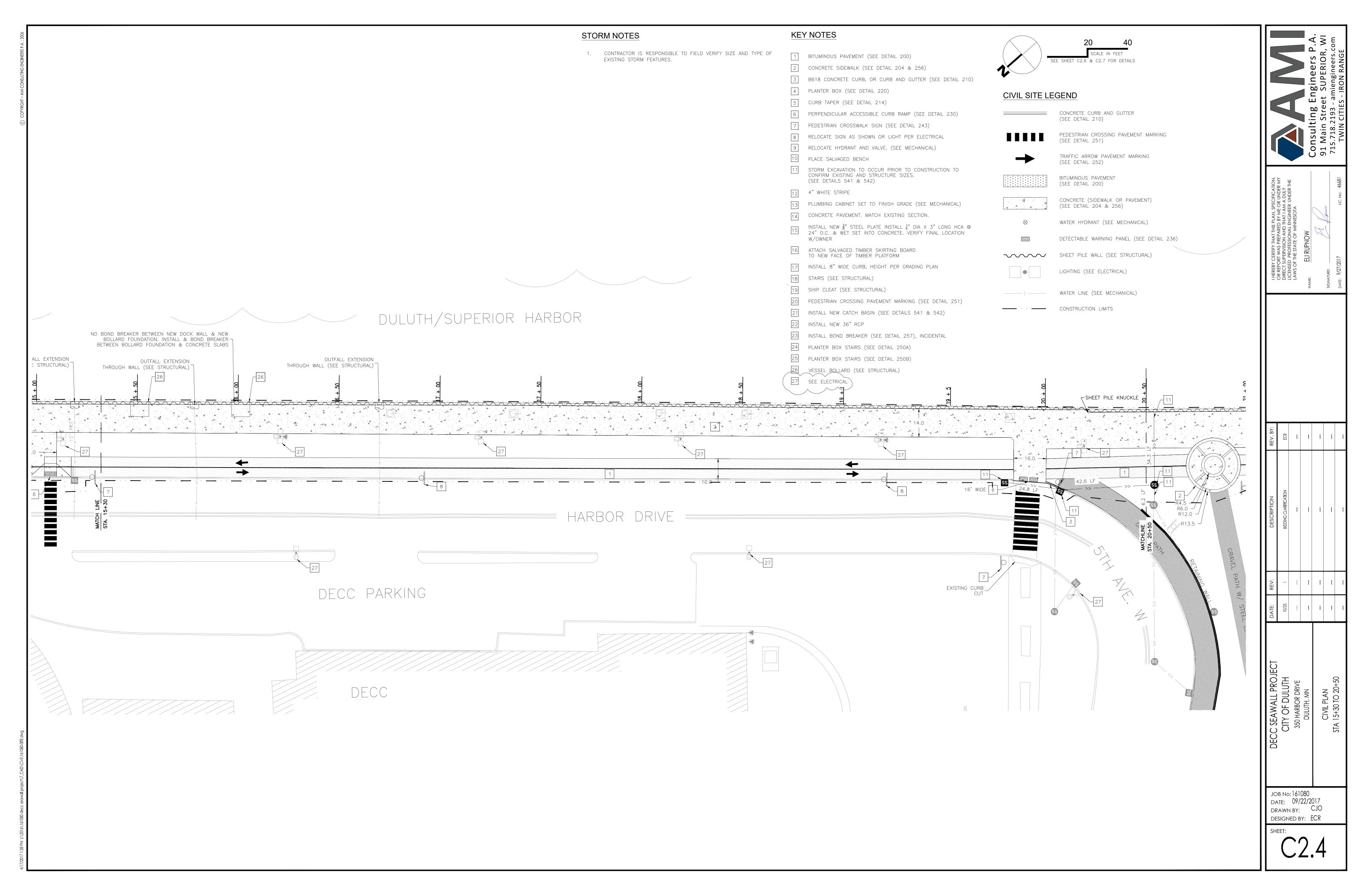
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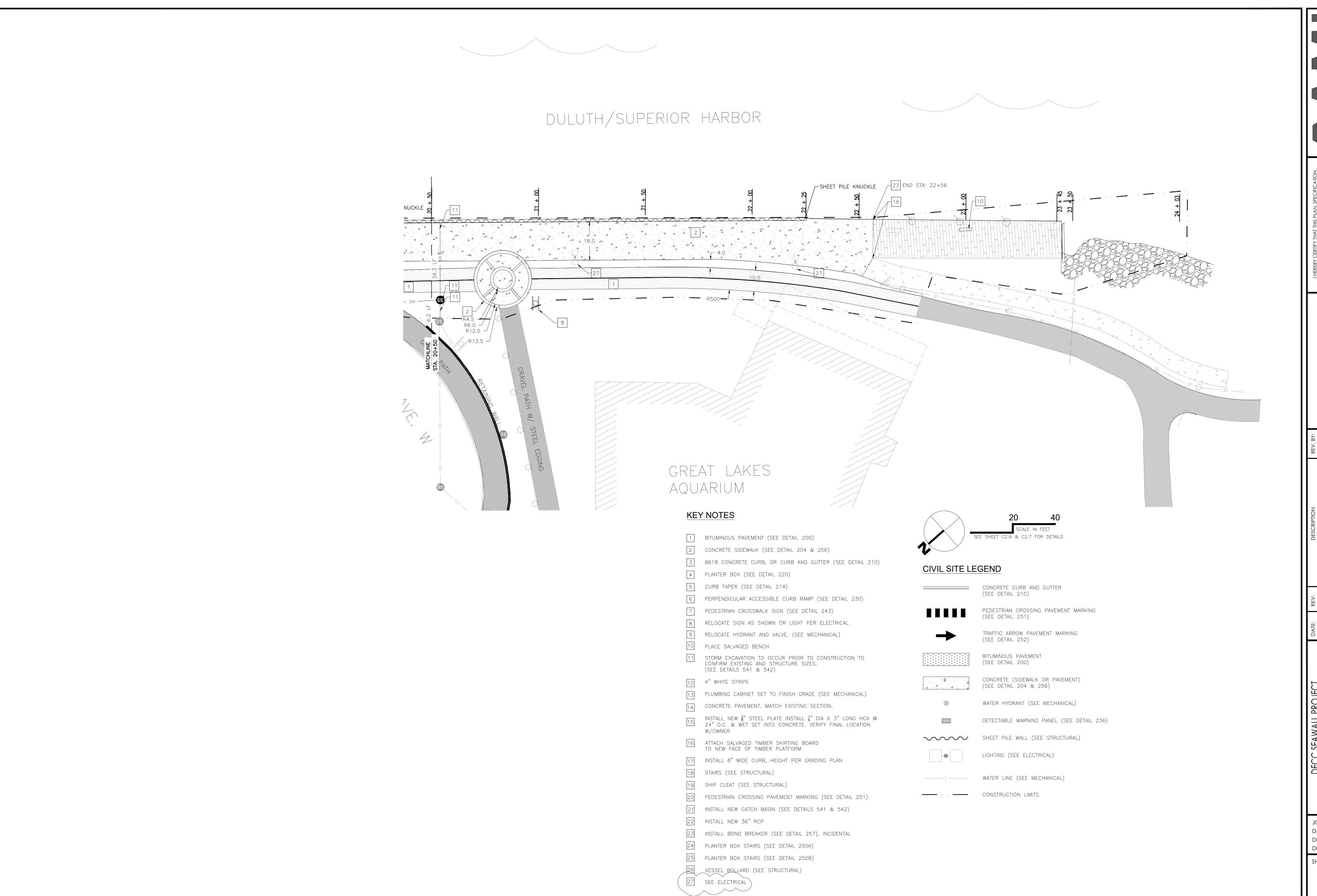
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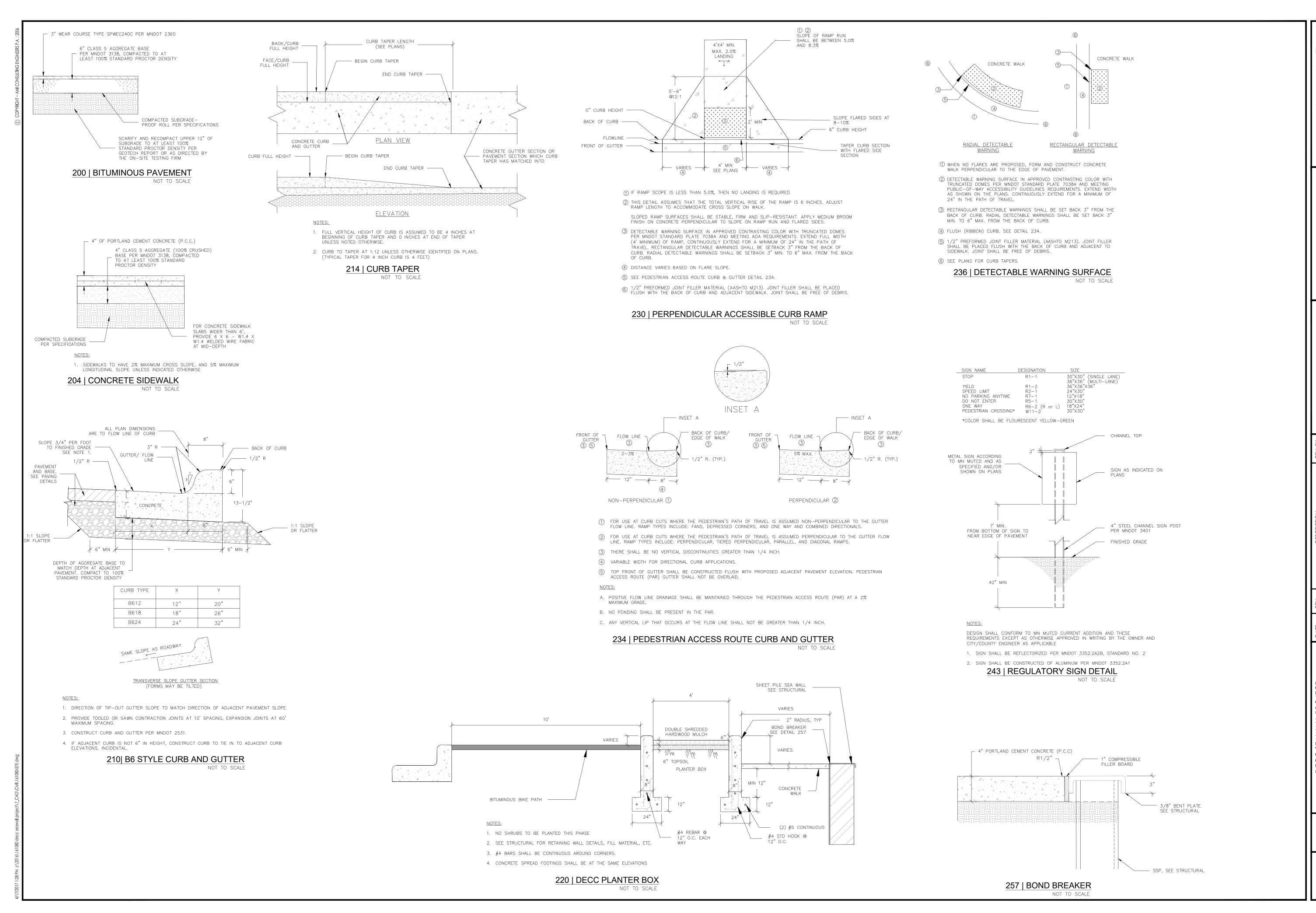
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NAME:

ELI RUPNOW

SIGNATURE:

CITY OF DULUTH

350 HARBOR DRIVE

DULUTH, MN

CIVIL PLAN DETAILS

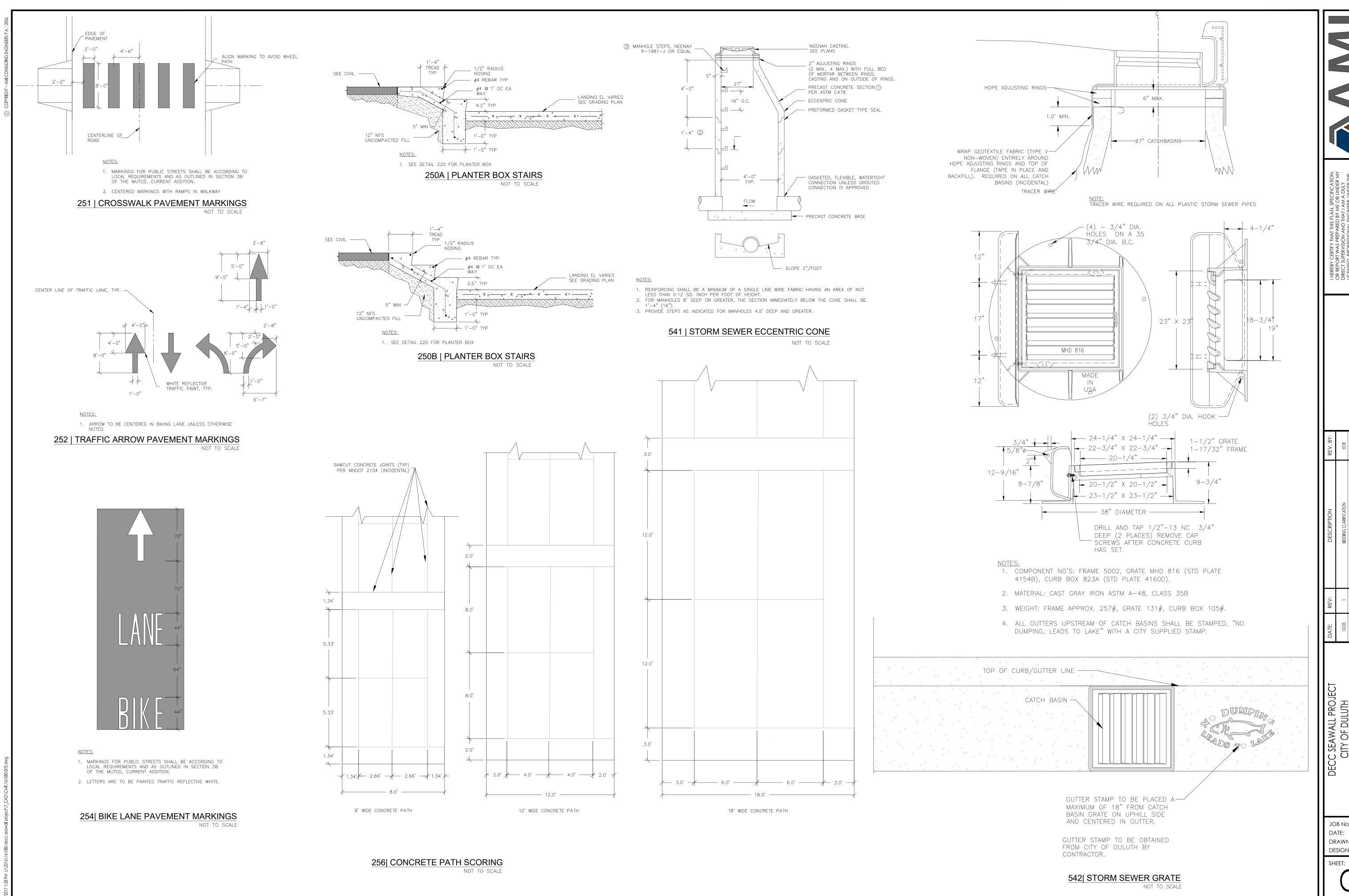
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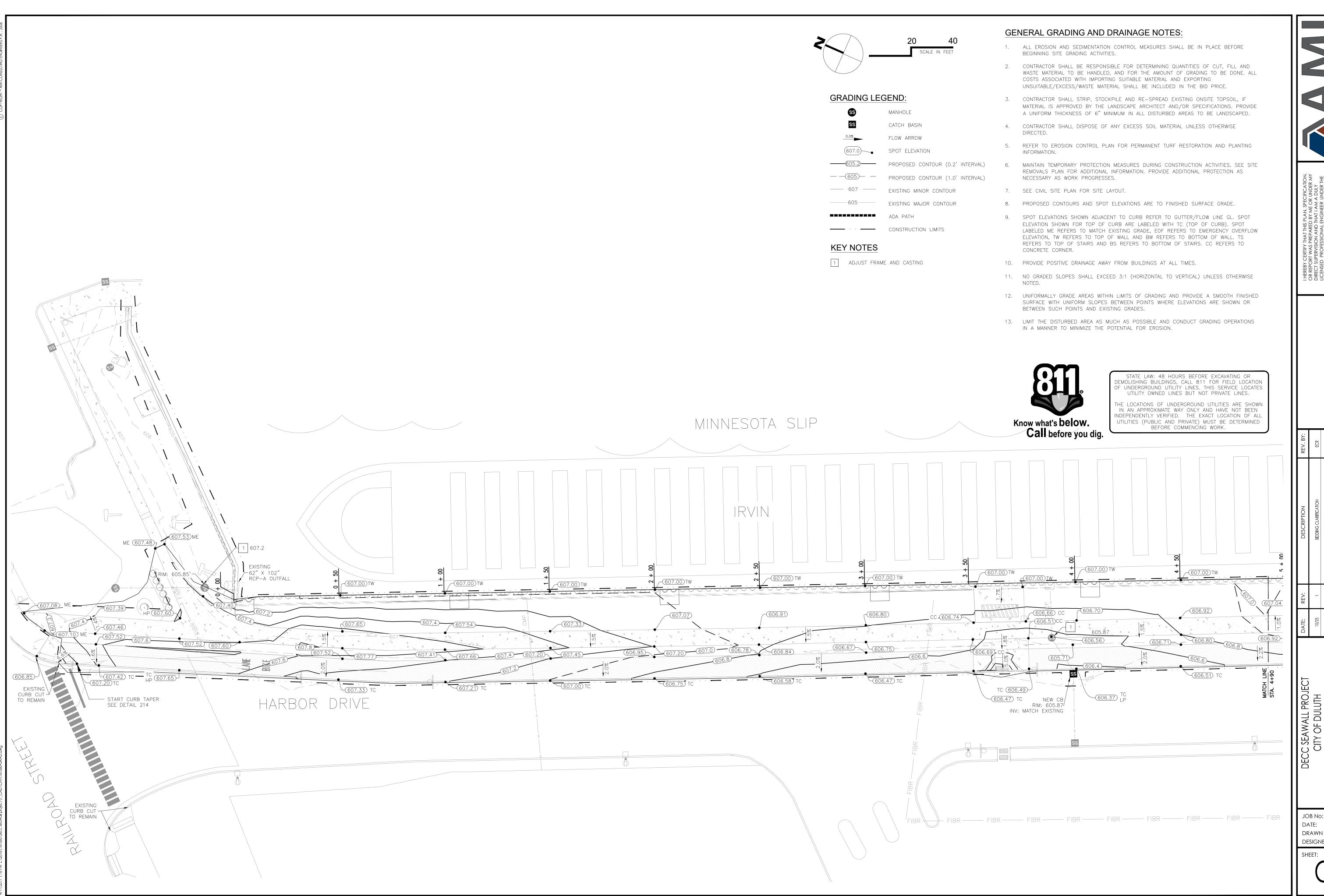
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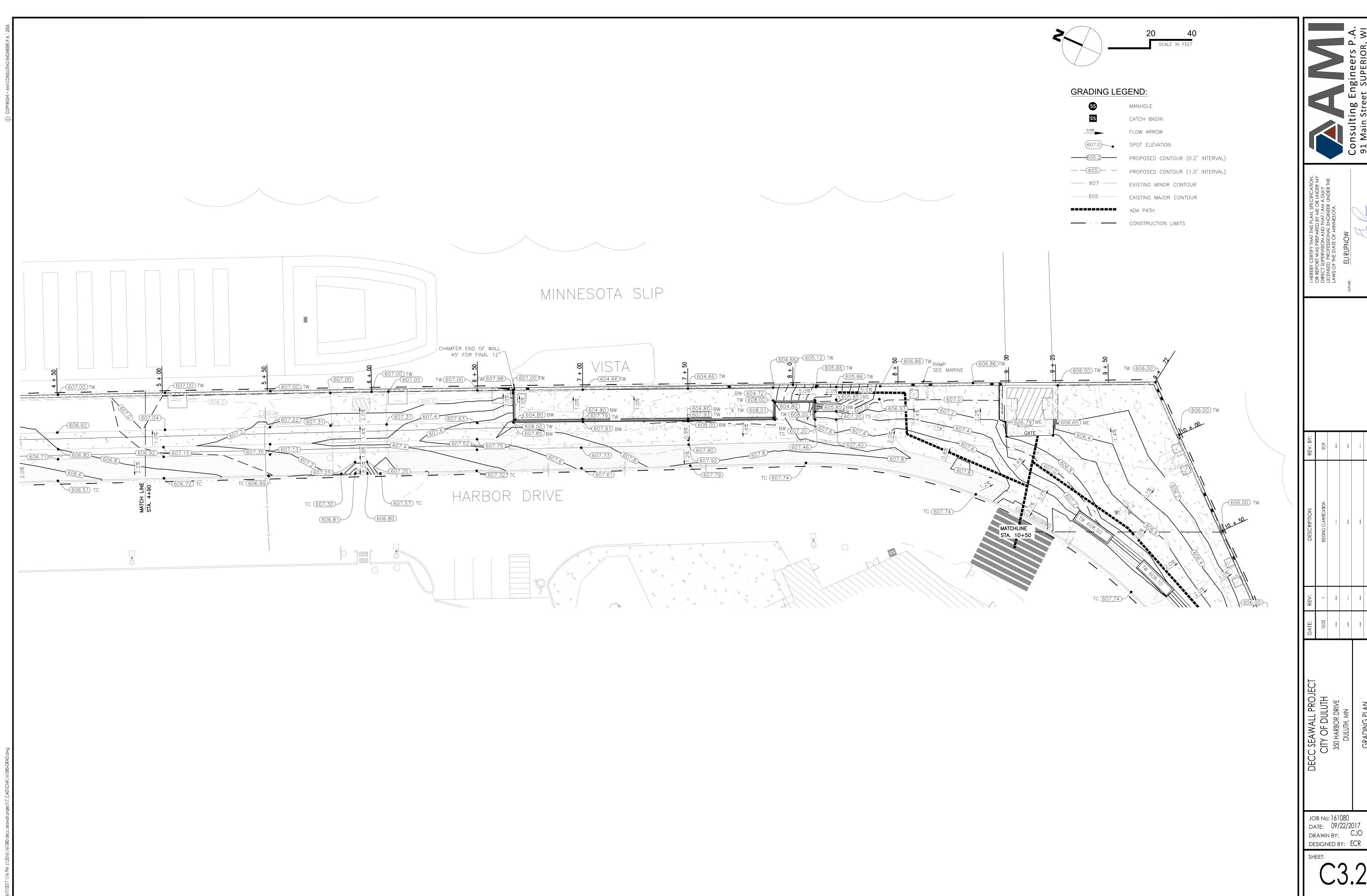
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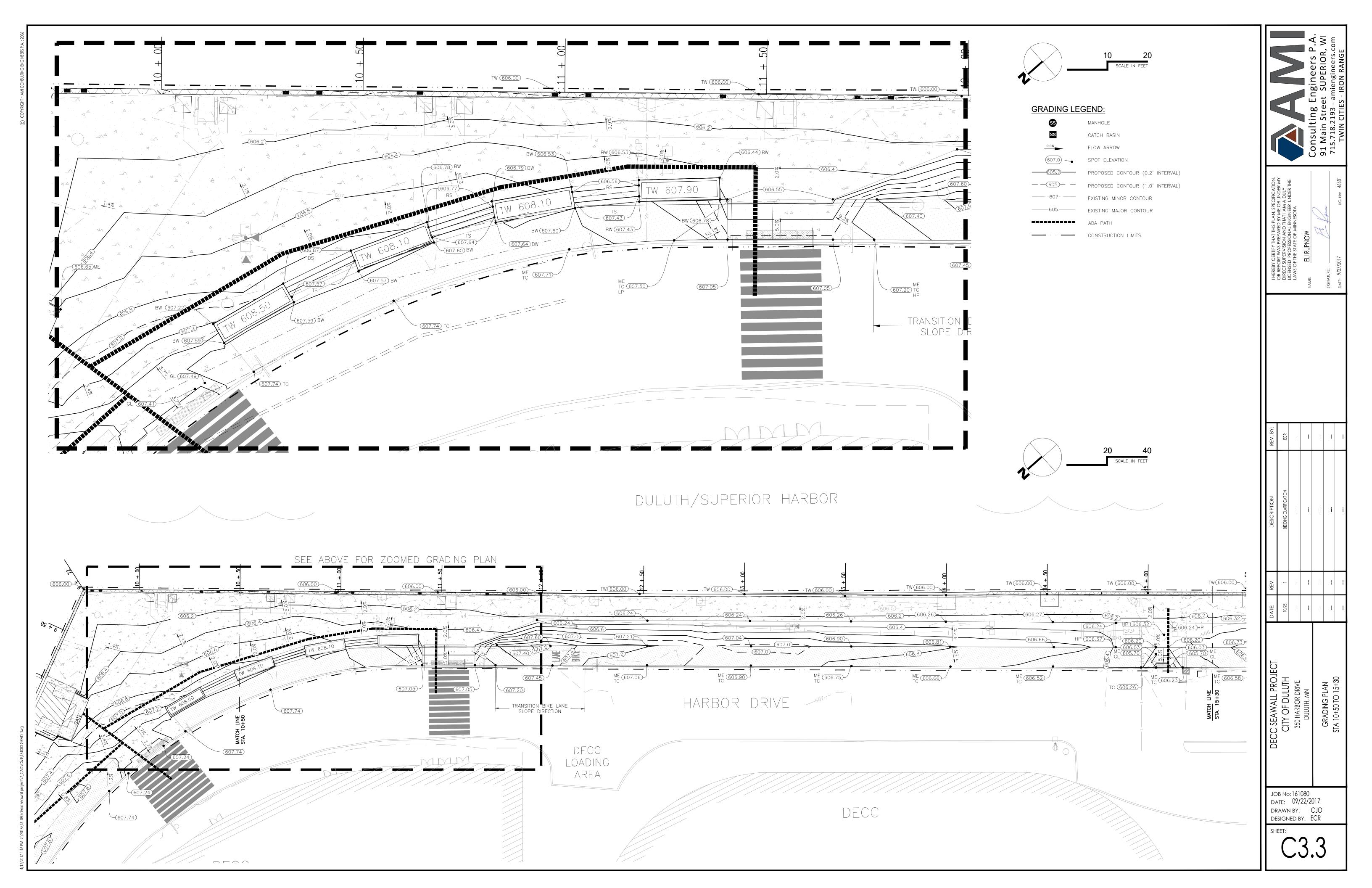
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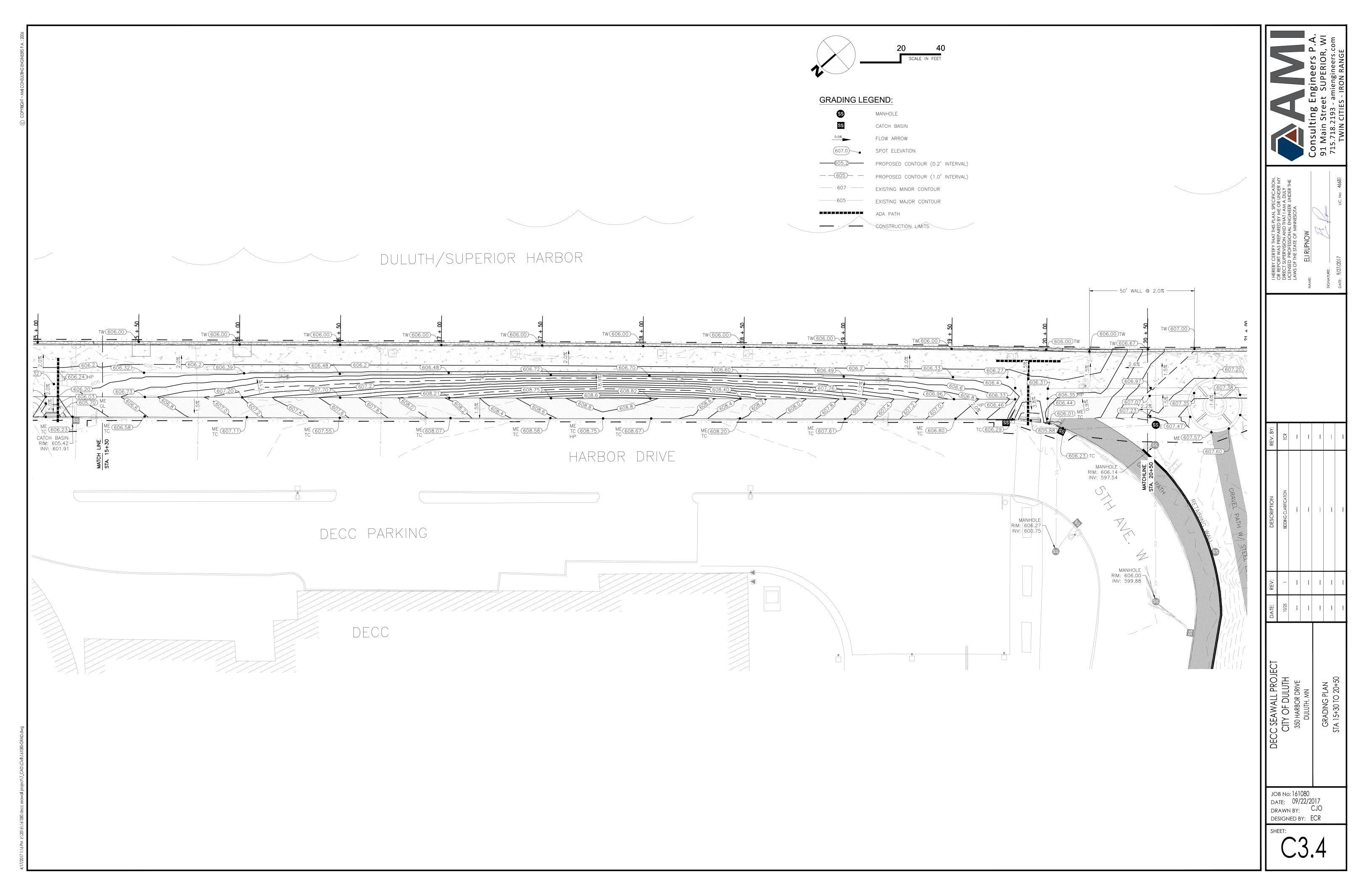
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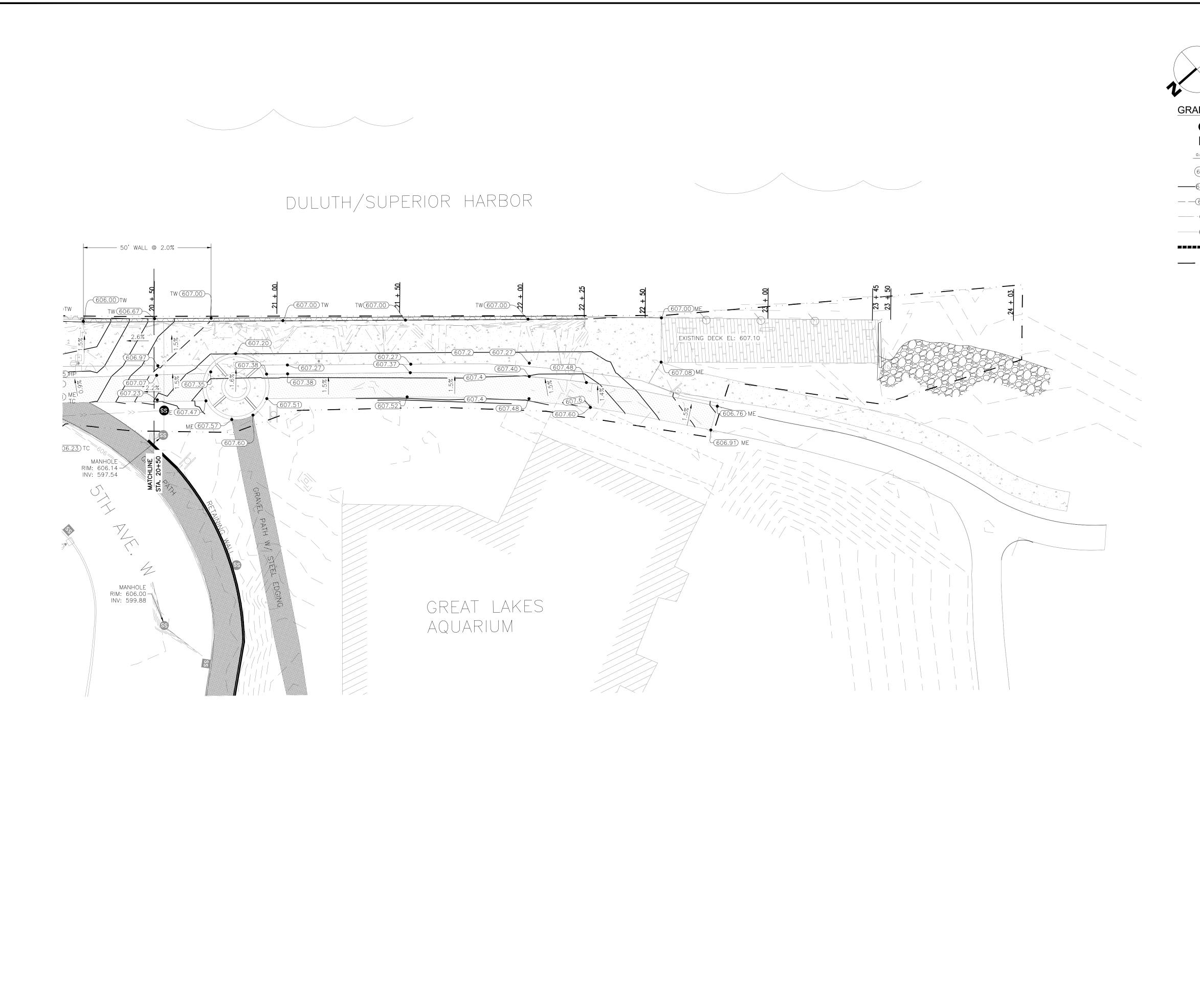
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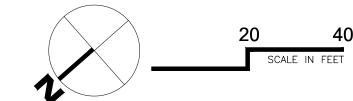


GRADING PLAN STA 4+90 TO 10+50









GRADING LEGEND:

SS MANHOLE

SS CATCH BASIN

FLOW ARROW

607.0 SPOT ELEVATION

PROPOSED CONTOUR (0.2' INTERVAL)

PROPOSED CONTOUR (1.0' INTERVAL)

PROPOSED CONTOUR

EXISTING MINOR CONTOUR

ADA PATH

---- CONSTRUCTION LIMITS

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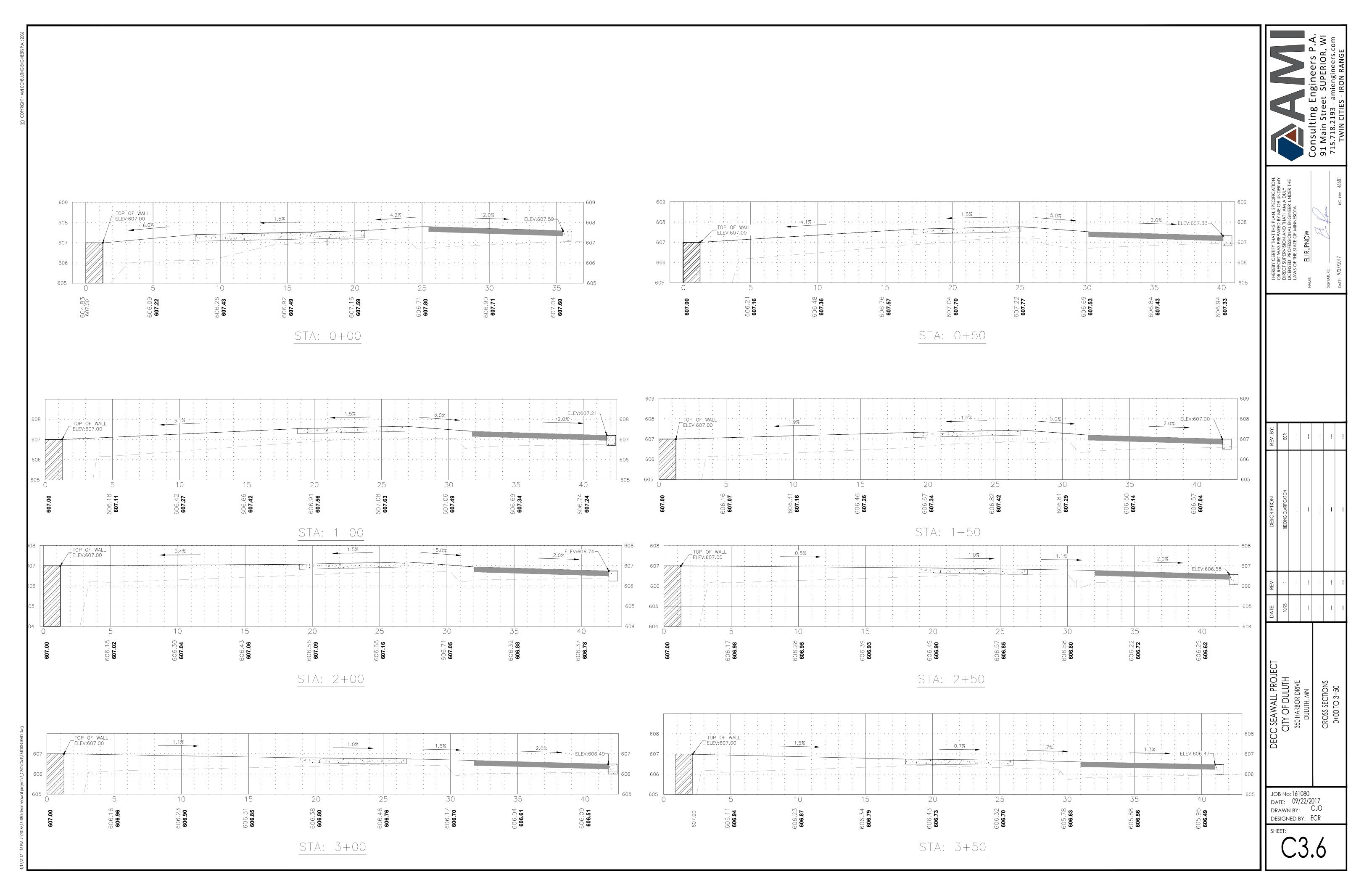
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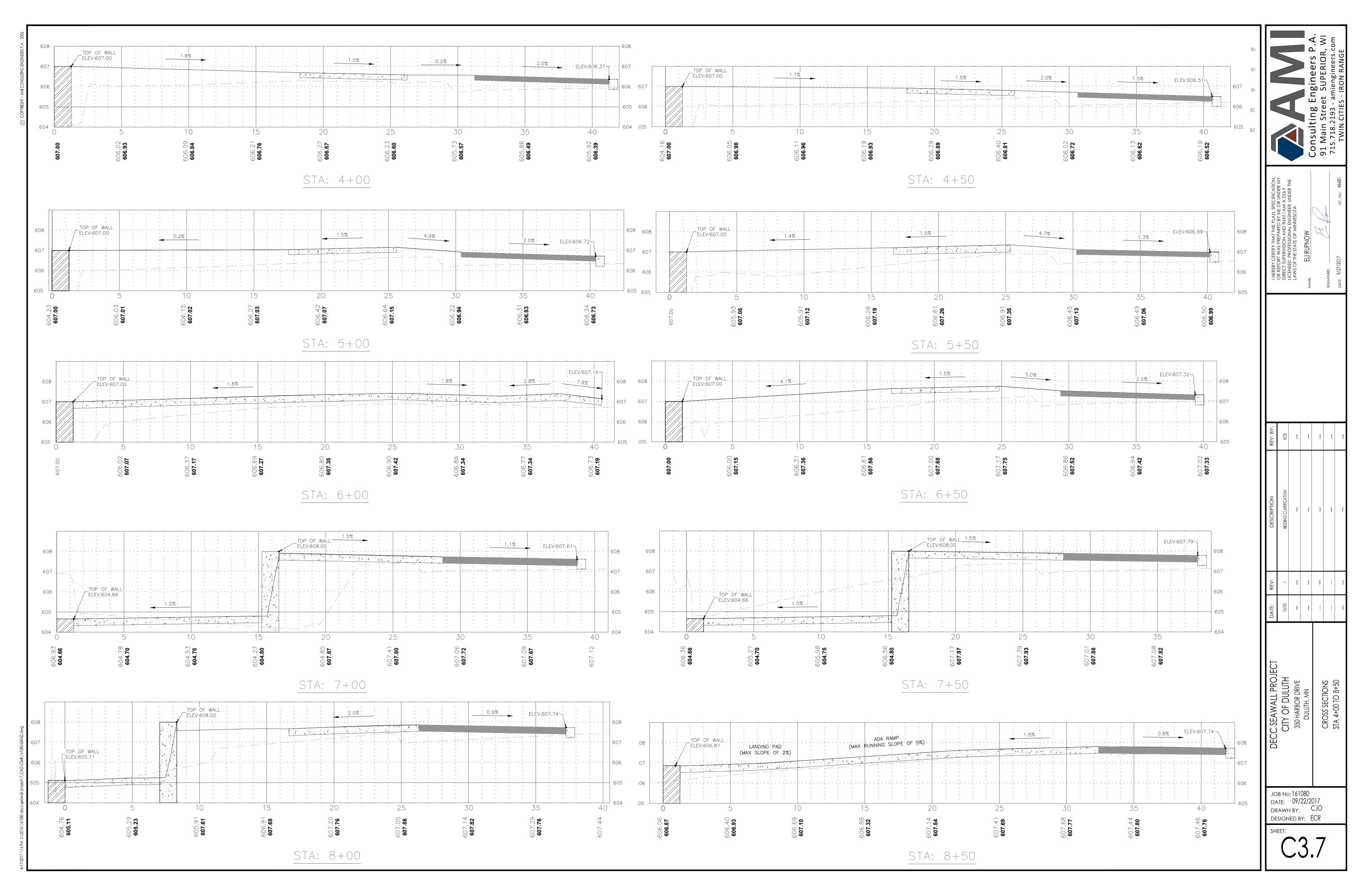
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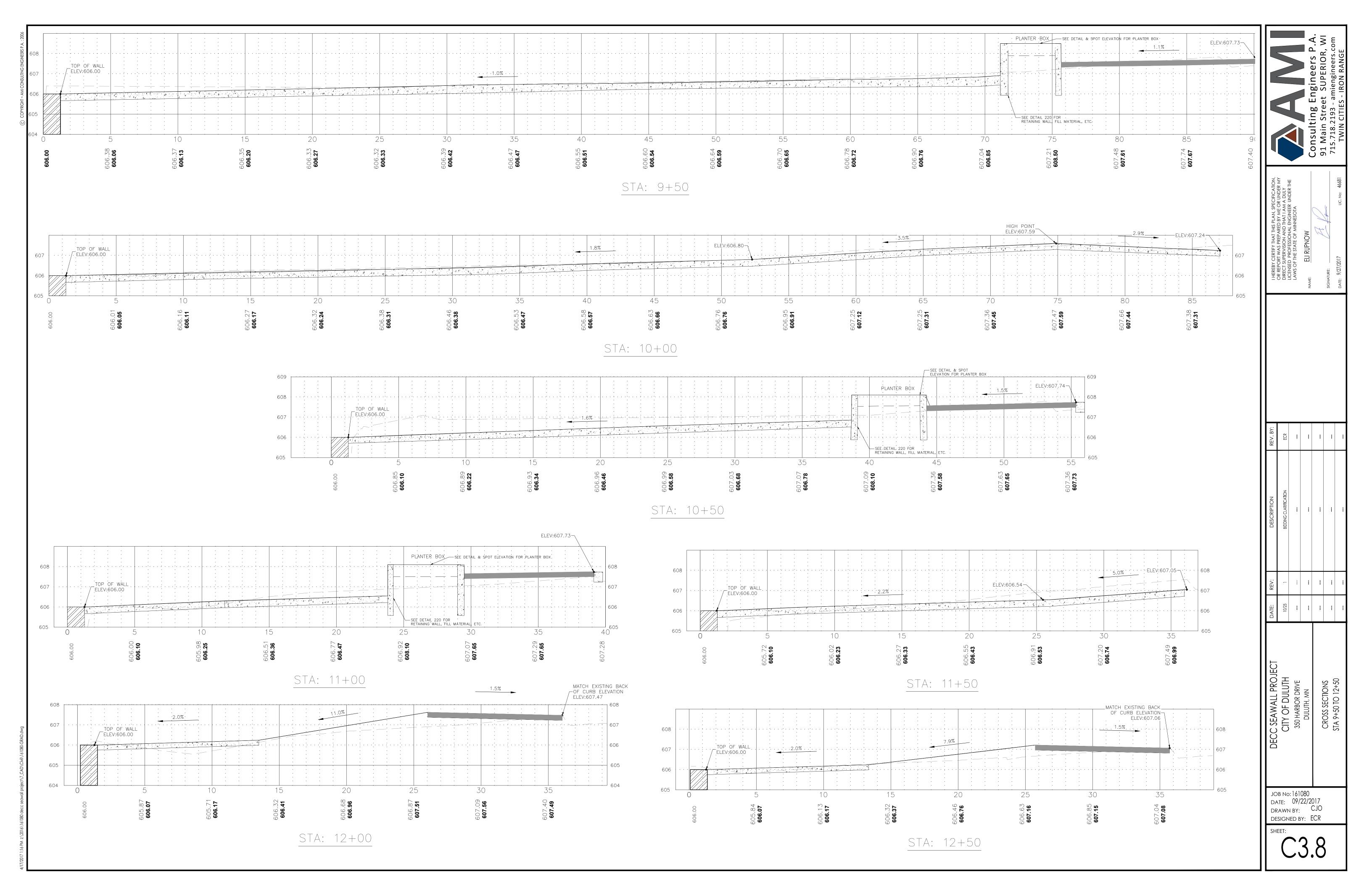
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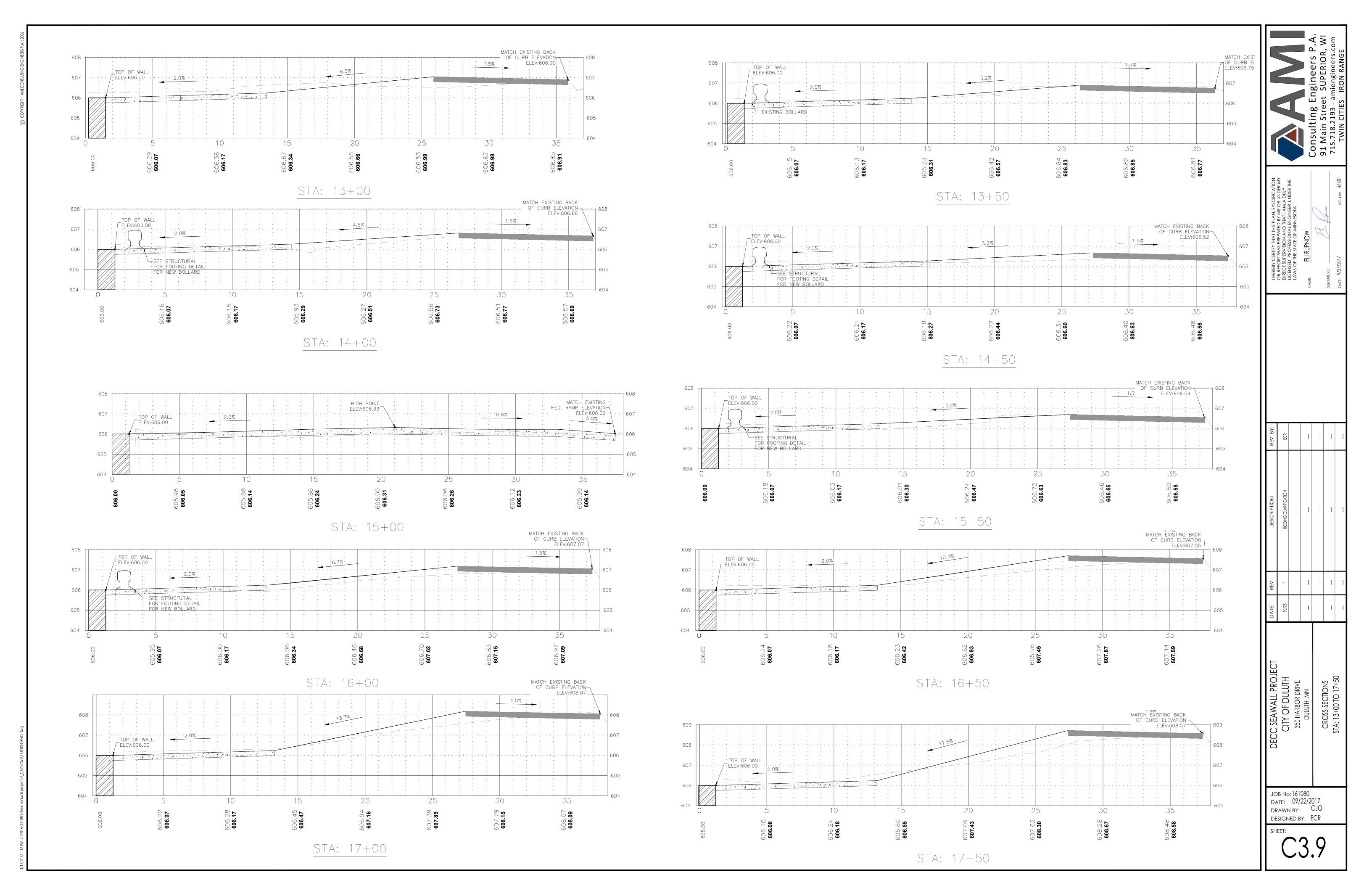
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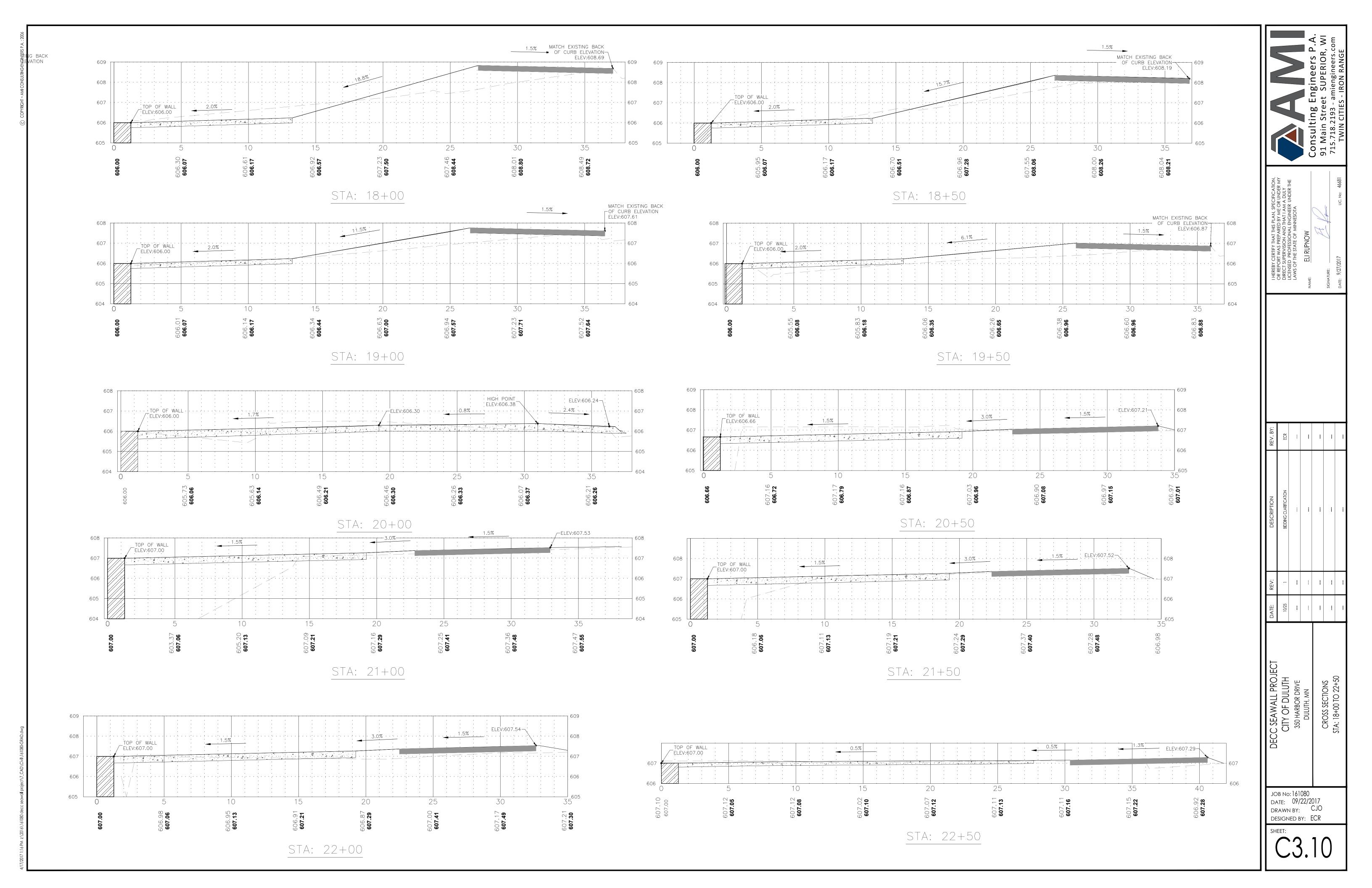
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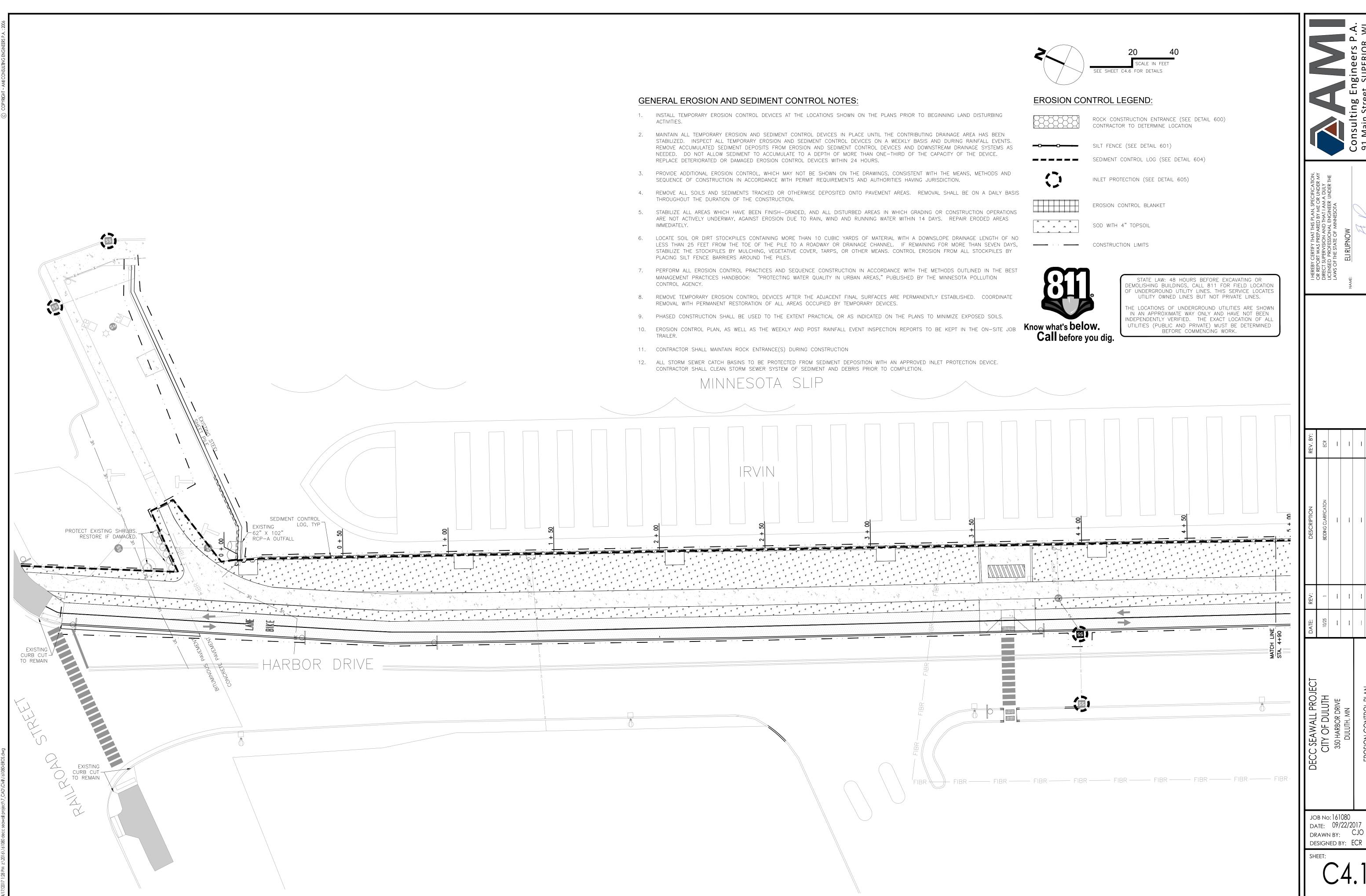




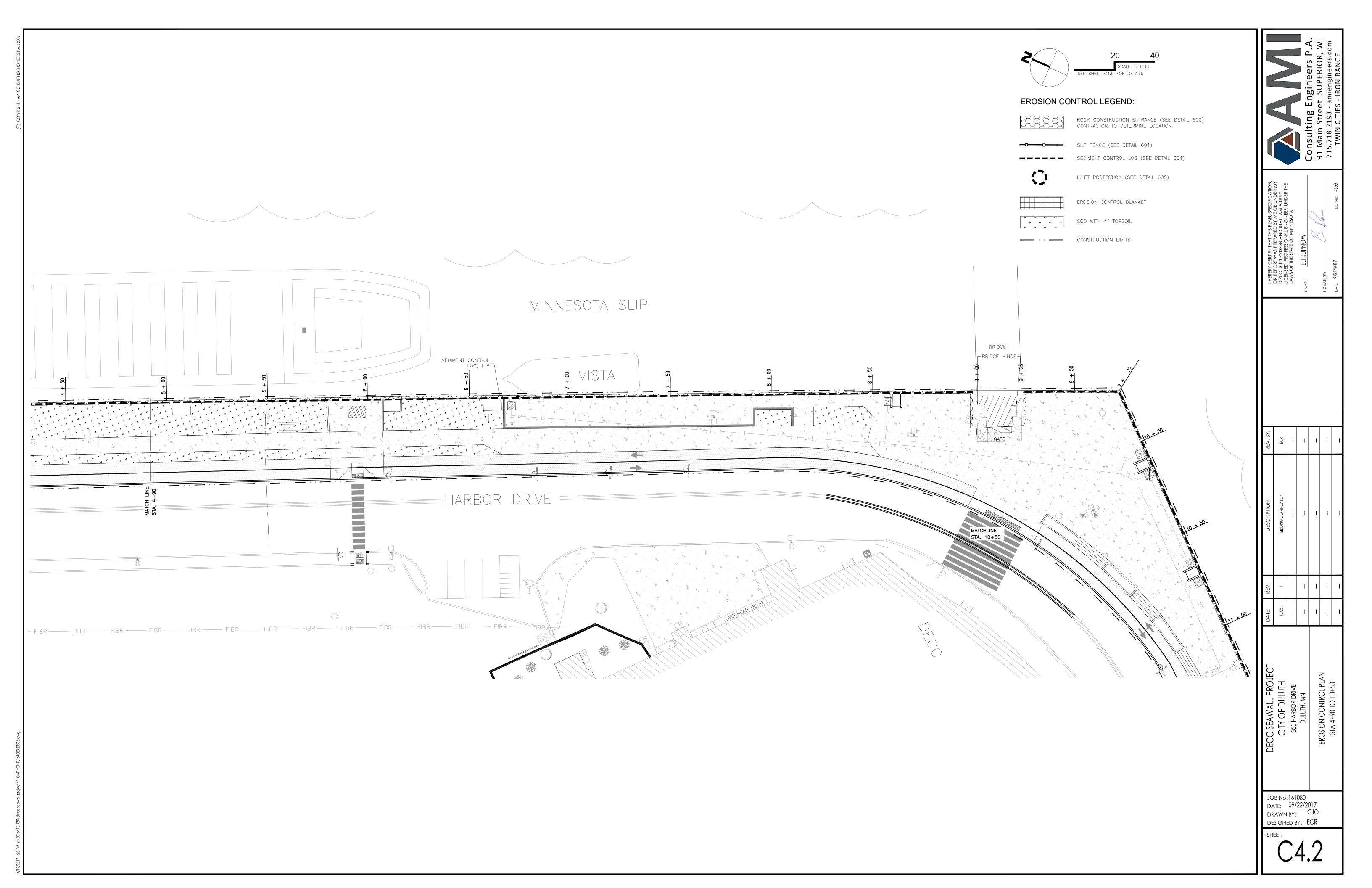


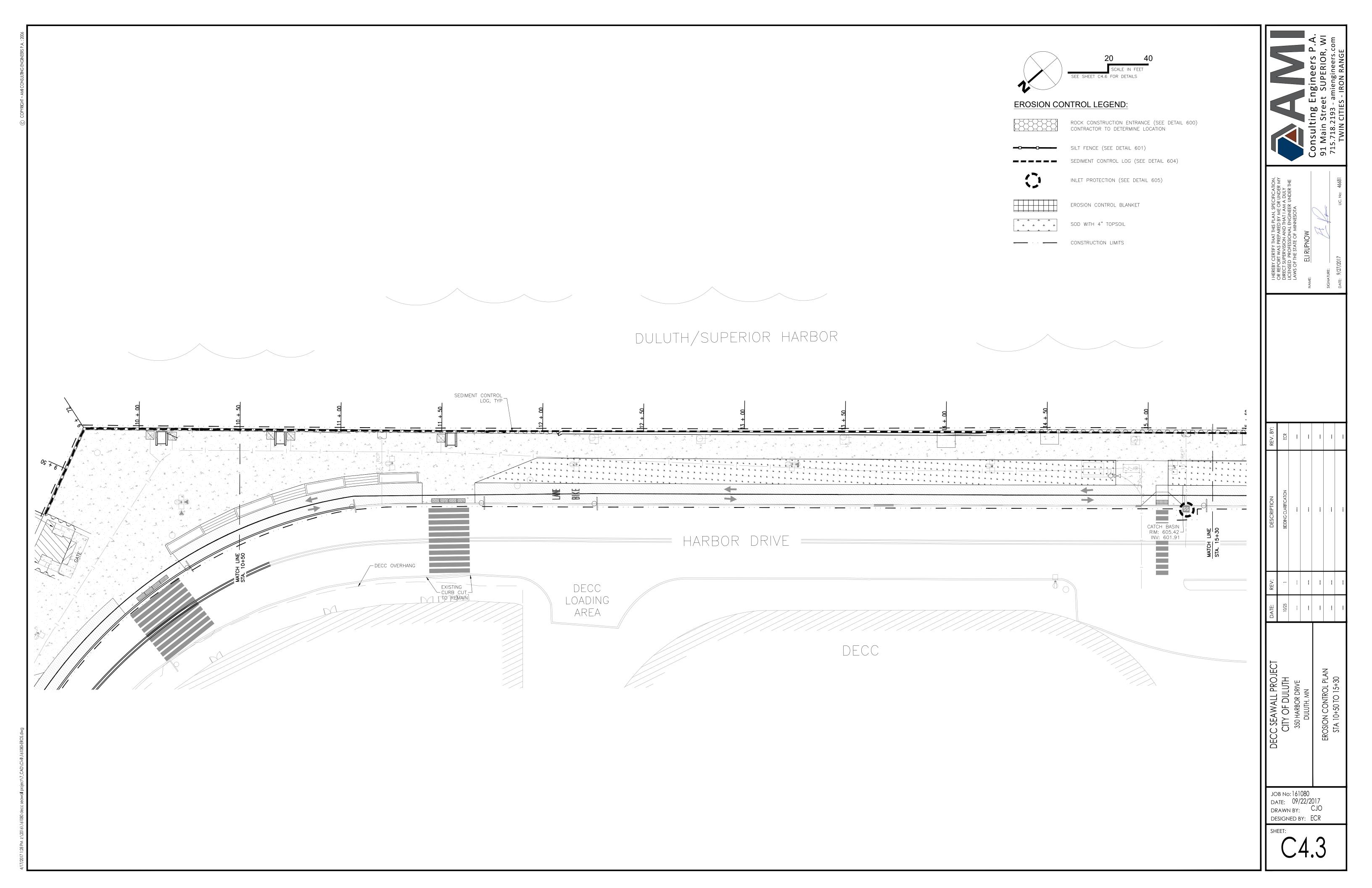


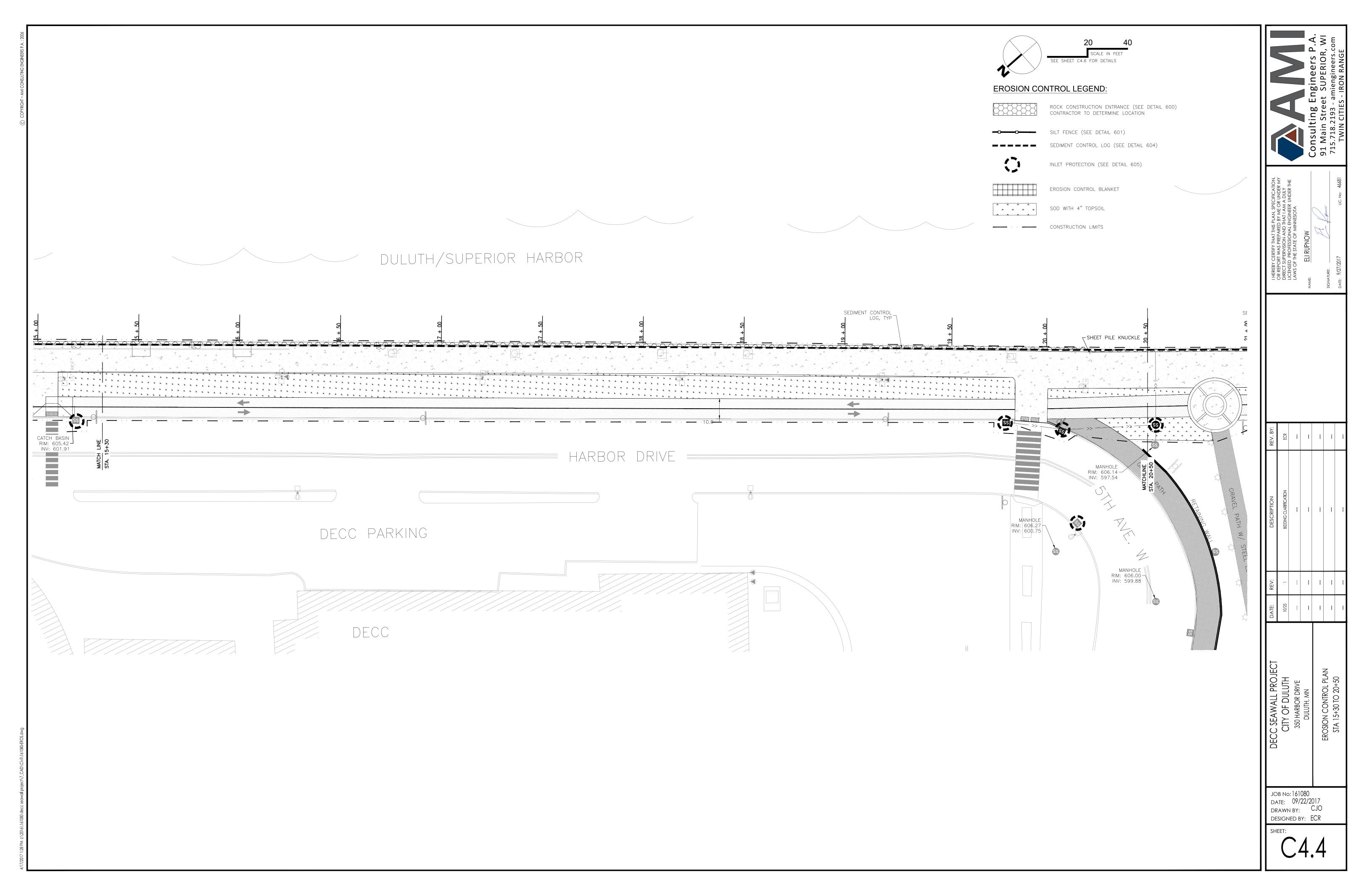




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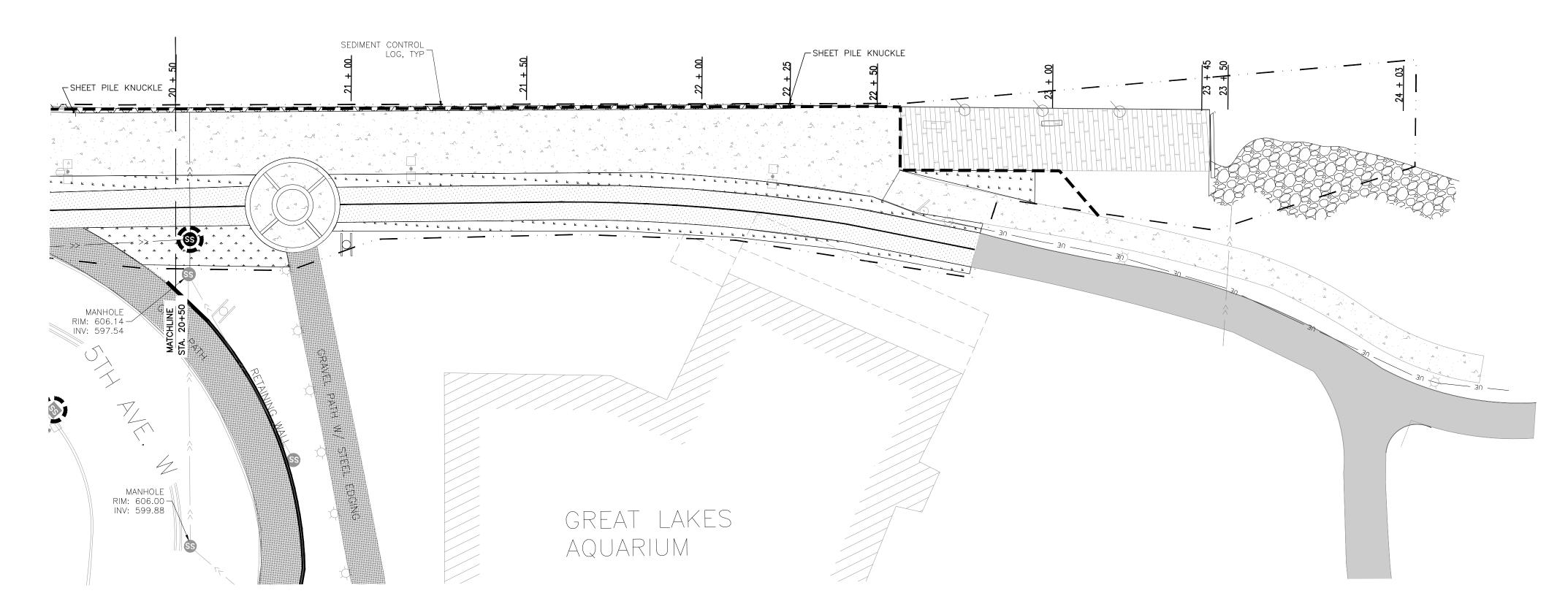


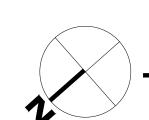






DULUTH/SUPERIOR HARBOR





EROSION CONTROL LEGEND:

ROCK CONSTRUCTION ENTRANCE (SEE DETAIL 600)
CONTRACTOR TO DETERMINE LOCATION

SILT FENCE (SEE DETAIL 601) SEDIMENT CONTROL LOG (SEE DETAIL 604)



INLET PROTECTION (SEE DETAIL 605)



EROSION CONTROL BLANKET

----- CONSTRUCTION LIMITS

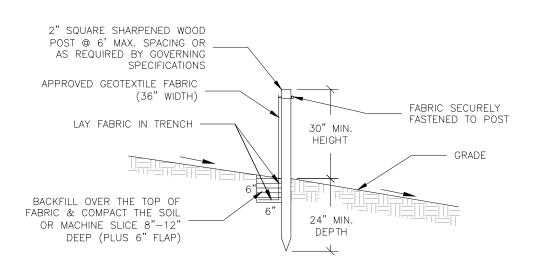
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NOTES:

- GEOTEXTILE SHALL BE PLACED UNDER ROCK TO STOP MUD MIGRATION THROUGH ROCK
- 2. OTHER METHODS BESIDES ROCK (SLASH MULCH, RUMBLE PAD, ETC.) MAY BE USED FOR EXIT STABILIZATION WITH ENGINEER
- 3. SEE EROSION CONTROL PLAN OR SWPPP FOR MAINTENANCE.
- 4. SEE MNDOT SPECS: 2573 & 3882

600| ROCK CONSTRUCTION ENTRANCE

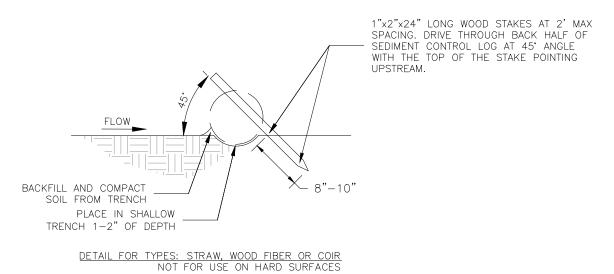
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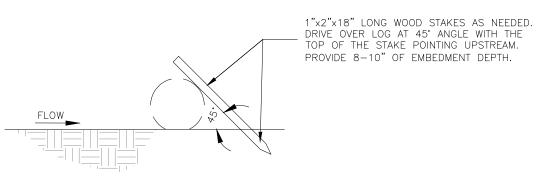


NOTES

- 1. POST SPACING SHALL NOT EXCEED 6 FEET.
- 2. POST TO BE ON LOWER SIDE OF SLOPE (TOWARD PROPERTY BOUNDARY OR POND SIDE) AND FENCE UPSTREAM.
- 3. GEOTEXTILE FABRIC SHALL BE "MIRAFI" TYPE OR APPROVED EQUAL.
- 4. TRENCH SHALL BE A MIN. OF 6 INCHES DEEP BY 6 INCHES WIDE.
- 5. MACHINE SLICED METHOD IS ACCEPTABLE
- 6. SEE EROSION CONTROL PLAN OR SWPPP FOR REPAIR AND MAINTENANCE.
- 7. SEE MNDOT SPECS 3886.

601 SILT FENCE NOT TO SCALE





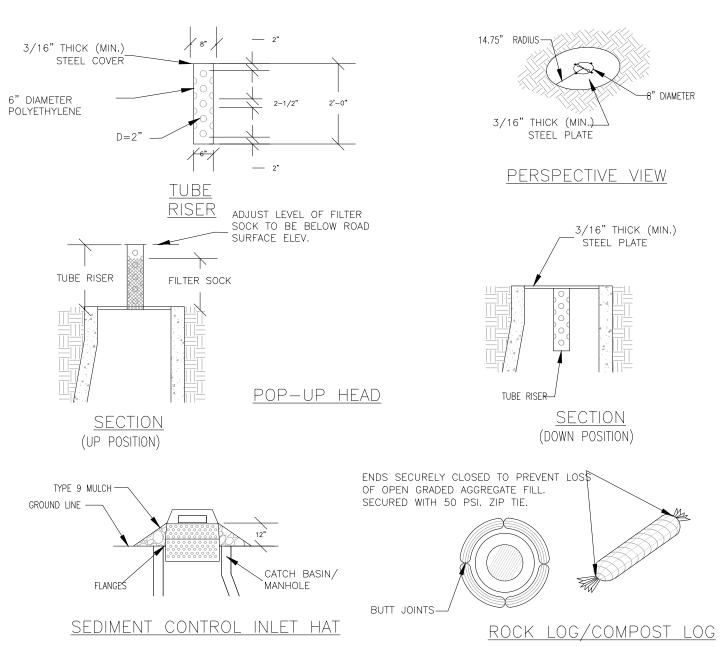
<u>DETAIL FOR TYPES: WOOD CHIP, COMPOST OR ROCK</u>
MAY BE USED ON HARD SURFACES. NO STAKES IF SHOWN ON HARD SURFACES

SEE MNDOT SPECS: 2573, 3601, 3885, 3886 AND 2573

NOTES:

- 1. SPACE BETWEEN STAKES SHALL BE A MAXIMUM OF 1 FOOT FOR DITCH CHECKS OR 2 FEET FOR OTHER APPLICATIONS.
- 2. PLACE STAKES AS NEEDED TO PREVENT MOVEMENT OF SEDIMENT CONTROL LOGS PLACED ON SLOPES OR AS NEEDED DUE TO OTHER FACTORS. STAKES SHALL BE INCIDENTAL.
- 3. FOR PERMANENT DITCH CHECK REDUCE THE HEIGHT TO 16 IN. AND MODIFY THE 1:2 (V:h) SIDE SLOPE TO 1:6 (V:H).
 4. TO BE USED FOR CRITICAL PERIMETER CONTROL AREAS WHERE STANDING WATER OCCURS (6 INCH MAX. DEPTH). BALES SHALL CONSIST OF TYPE 1 MULCH OF APPROXIMATELY 14 IN. X 18 IN. X 36 IN. LONG. BALES SHALL BE PLACED ON EDGE AND BUTTED TIGHT TO ADJACENT BALES.
- 5. INSTEAD OF TRENCHING, PLACE BALE ON THE BLANKET AND WRAP BLANKET AROUND THE BALE. PLACE STAKE THROUGH BALE AND BLANKET.

604 | SEDIMENT CONTROL LOG



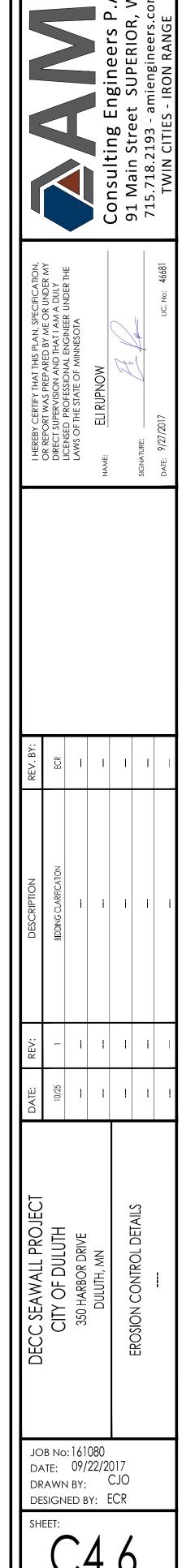
NOTES:

1. THE SEDIMENT CONTROL BARRIER SHALL BE A METAL OR PLASTIC/POLYETHYLENE RISER SIZED TO FIT INSIDE THE CATCH BASIN/MANHOLE: HAVE PERFORATIONS TO ALLOW FOR WATER INFILTRATION: HAVE AN OVERFLOW OPENING, FLANGES AND A LID/COVER.

- 2. MANUFACTURED ALTERNATIVES LISTED ON MNDOT'S APPROVED PRODUCTS LIST MAY BE SUBSTITUTED.
- 3. SOCK HEIGHT MUST NOT BE SO HIGH AS TO SLOW DOWN WATER FILTRATION TO CAUSE FLOODING
- 4. GEOTEXTILE SOCK BETWEEN 4-10 FEET LONG AND 4-6 INCH DIAMETER. SEAM TO BE JOINED BY TWO ROWS OF STITCHING WITH A PLASTIC MESH BACKING OR PROVIDE A HEAT BONDED SEAM (OR APPROVED EQUIVALENT). FILL ROCK LOG WITH OPEN GRADED CONFORMING TO SPEC. 3137 TABLE 3137-1; CA-3 GRADATION.
- 5. SEE MNDOT SPECS: 2573, 3137, 3886, & 2573.

605 | INLET PROTECTION

NOT TO SCALE



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- ELEVATIONS SHOWN, EXCEPT AS NOTED, ARE BASED ON LAKE SUPERIOR IGLD 1985 LOW WATER DATUM OF 601.1. ON GENERAL ARRANGEMENT DRAWINGS, ELEVATIONS ARE ALSO SHOWN RELATIVE TO IGLD
- ALL PLAN DIMENSIONS ON THE DRAWINGS ARE MEASURED IN A TRUE HORIZONTAL PLANE UNLESS NOTED
- EXISTING WATER ELEVATIONS MAY VARY ABOVE AND BELOW IGLD ELEVATION THROUGHOUT THE PROJECT DUE TO ENVIRONMENTAL CHANGES, I.E. RAINFALL, WIND, RUNOFF, AND CYCLICAL CHANGES IN WATER
- ALL MATERIALS AND INSTALLATION MUST MEET THE STANDARD SPECIFICATIONS LISTED IN THE DESIGN CRITERIA SECTION OF THE STRUCTURAL NOTES AND THE PROJECT SPECIFICATIONS.
- THE MARINE DRAWINGS ARE TO BE WORKED TOGETHER WITH MECHANICAL, ELECTRICAL, AND CIVIL DRAWINGS AND SPECIFICATIONS FOR ALL INTER-DISCIPLINE INTERFACE WORK WHICH MAY NOT BE INCLUSIVE ON THE MARINE DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATION AND SHALL REPORT ANY DISCREPANCY TO THE ENGINEER PRIOR TO COMMENCING THE WORK.
- OPENINGS AND PENETRATIONS NOT SHOWN IN THE CONTRACT DOCUMENTS THROUGH ANY STRUCTURAL ELEMENTS OR ITEMS EMBEDDED IN THE STRUCTURAL ELEMENTS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO IMPLEMENTING THE WORK.
- PLANS, SECTIONS, AND DETAILS SHALL NOT BE SCALED FOR DETERMINATION OF SIZE, QUANTITIES,
- ALL MEMBERS ARE DESIGNED TO RESIST THE DESIGN LOADS WITHIN THE COMPLETED MARINE SYSTEM. CONTRACTOR IS RESPONSIBLE FOR ADEQUATE SHORING, BRACING, ETC DURING CONSTRUCTION. - CONTRACTOR IS RESPONSIBLE FOR PROTECTION OF ANY AND ALL STREETS, UTILITIES, EXISTING
- STRUCTURES, EQUIPMENT, ETC. - CONTRACTOR IS RESPONSIBLE TO FOLLOW ALL LOCAL, STATE, & FEDERAL PERMIT REQUIREMENTS AT ALL
- EXISTING CONDITIONS, RELATED DIMENSIONS, ELEVATIONS INDICATED IN THE CONTRACT DOCUMENTS SHALL BE FIELD VERIFIED AS SITE CONDITIONS MAY HAVE CHANGED SINCE LAST INSPECTION BY ENGINEER. ANY VERIFIED CONDITIONS THAT DIFFER FROM THAT INDICATED IN THE CONTRACT DOCUMENTS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO PRODUCTION OF SHOP DRAWINGS & FARRICATION
- WHERE A SPECIFIC MODEL, MANUFACTURER, OR GEOMETRIC SIZE/SHAPE OF AN ITEM ARE IDENTIFIED ON THE DRAWINGS OR IN THE SPECIFICATIONS, THE MODEL, MANUFACTURER, OR GEOMETRIC SIZE/SHAPE IDENTIFIED ARE THE BASIS OF THE DESIGN. ITEMS OF OTHER MODEL, MANUFACTURER, OR GEOMETRIC SIZE/SHAPE OF EQUAL DESIGN WHICH ARE ACCEPTED BY THE ENGINEER THAT REQUIRE ANY ADDITIONAL DRAWINGS, ENGINEERING DEVIATIONS, OR CONSTRUCTION/QUANTITY CHANGES ARE THE RESPONSIBILITY OF THE CONTRACTOR INCLUDING ALL ASSOCIATED COSTS.
- THE ACCURACY OF EXISTING UNDERGROUND UTILITIES AND STRUCTURES ARE NOT GUARANTEED AND NOT
- INCLUSIVE. FIELD CONDITIONS SHALL BE VERIFIED PRIOR TO ANY EXCAVATION — THE GENERAL STRUCTURAL AND MARINE NOTES GIVEN IN THE CONSTRUCTION DOCUMENTS MAY NOT BE
- INCLUSIVE TO THE ENTIRE PROJECT. SEE FULL PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION. - CONTRACTOR IS RESPONSIBLE FOR SUBMITTING A SITE SAFETY AND SITE CONTINGENCY PLAN, AND ADHERE TO THE RESPONSE ACTION PLAN (RAP) ADDRESSING THE REQUIREMENTS SET FORTH IN 29 CFR
- ALL CONSTRUCTION MATERIALS, STRUCTURAL STEEL, STEEL SHEET PILE, AND COATINGS SHALL BE AMERICAN MINED AND SOURCED.

DESIGN CRITERIA

- CODES AND SPECIFICATIONS
- ALL DESIGN, UNLESS OTHERWISE NOTED, ARE IN ACCORDANCE WITH THE FOLLOWING:
 - I. LOCAL & STATE CODES FOR WHICH THE PROJECT IS ERECTED II. INTERNATIONAL BUILDING CODE (IBC)
 - III. ASCE STANDARD 7 IV. STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES (AASHTO)

DESIGN LOADS

A. LIVE LOADS WIND SPEED FOR DOCK STRUCTURE: 105 MPH 3 SEC GUST PRIMARY DOCK SURCHARGE: 300 PSF

- B. MOORING ARRANGEMENT BASED ON BOLLARD CAPACITY OF 65 TONS C. FOR MOORING DESIGN, WIND PRESSURE ON VESSEL BASED ON WIND PRESSURES SHOWN ABOVE
- D. A DESIGN VESSEL IMPACT VELOCITY OF 0.5 FEET PER SECOND AND A 10° APPROACH ANGLE WERE
- USED TO DETERMINE THE TOTAL BERTHING IMPACT ENERGY FOR EACH BARGE VESSEL E. THE STRUCTURES HAVE BEEN DESIGNED TO RESIST DESIGN LOADS ONLY AS COMPLETED STRUCTURES, UNLESS OTHERWISE NOTED ON THE DRAWINGS. ANY PROPOSED APPLICATION OF CONSTRUCTION LOADS WHICH EXCEED THE DESIGN LOADS MUST BE APPROVED BY THE ENGINEER.

GENERAL VESSEL PARAMETERS

A. S.S. WILLIAM A. IRVIN VESSEL PROPERTIES: LENGTH OVERALL: 610 FT BREADTH MOLDED: DRAFT, WITHOUT BALLAST 15 FT 33 FT DRAFT, WITH BALLAST B. VISTA STAR VESSEL PROPERTIES: 92 FT LENGTH OVERALL: BREADTH MOLDED: 26 FT 7.5 FT DRAFT, DESIGN, MOLDED:

TESTING NOTES

- CONTRACTOR SHALL PERFORM ALL QUALITY CONTROL TESTING PER THE SPECIFICATIONS AND SUBMIT REPORTS TO OWNER/ENGINEER WEEKLY.
- CONTRACTOR SHALL COORDINATE AND COOPERATE WITH THE ENGINEER AND OWNER'S TESTING AGENCY ENGINEER TO ALLOW FOR PERFORMANCE & QUALITY TESTING OF ALL MATERIALS.
- THE OWNER/FNGINFER WILL PERFORM OR ENGAGE A REPRESENTATIVE FIRM(S) FOR THE INDEPENDENT TESTING OF THE FOLLOWING ITEMS:
- A. CONCRETE: ONE SET OF FIVE CONCRETE TEST CYLINDERS WILL BE TAKEN FOR EVERY 100 FEET OF DOCK WALL, EACH INDIVIDUAL CONCRETE STRUCTURE (I.E. DOLPHIN, PIER, BOLLARD PAD, ETC.) OR EVERY 50 CUBIC YARDS OR LESS OF CONCRETE PLACED DAILY. TEST TWO CYLINDER AT SEVEN DAYS AND THREE AT 28 DAYS. CONCRETE TEST REPORTS TO BE SUBMITTED TO THE ENGINEER. AIR CONTENT AND SLUMP TESTS SHALL BE TAKEN FOR EACH SET OF TEST
- B. MARINE COATING: VERIFICATION THAT THE STEEL PILES HAVE BEEN PREPARED FOR COATING IN ACCORDANCE WITH THE SPECIFICATIONS AND COATING MANUFACTURER'S RECOMMENDATIONS AND ALL OTHER REQUIREMENTS. VERIFICATION OF THE DRY FILM THICKNESS OF THE EPOXY COATINGS, SURFACE PREPARATION, AND THAT THE APPLICATION PROCESS IS BEING PERFORMED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. DESTRUCTIVE COATING ADHESION TESTS WILL BE PERFORMED TO DETERMINE ADEQUATE PERFORMANCE FOR COATING. COATING
- SHALL BE REPAIRED IN ACCORDANCE MANUFACTURER'S RECOMMENDATIONS C. PILE LOAD TESTS (HP PILES FOR HELICALS/TRANSFER BEAM): PERFORM A MINIMUM OF THREE DYNAMIC PILE TESTS. PDA TEST SHALL CONFORM TO ASTM 4945 AND THE CONTRACTOR MAY USE PRODUCTION PILES FOR THE LOAD TEST AT HIS OPTION. PILING MUST HAVE A MINIMUM 31 TON AXIAL CAPACITY PER PILE (SERVICE LOAD OF 62 TONS WITH FS = 2.0). PERFORM A DRIVE TEST ONCE FOR THE PURPOSE OF CONFIRMING PILE LENGTHS REQUIRED. DRIVE TEST PILES SHALL BE
- A MINIMUM 15' LONGER THAN AS SHOWN. D. HELICAL ANCHORS: PERFORMANCE TESTS SHALL BE PERFORMED ON 5% OF THE ANCHOR INSTALLED. PROOF TESTS SHALL BE PERFORMED ON ALL OF THE REMAINING ANCHORS. ALL OF THE TESTS SHALL CONFORM TO MANUFACTURERS RECOMMENDATIONS AND ASTM A981 & D4435 WHERE APPLICABLE. HELICAL ANCHORS MUST BE DESIGNED FOR A SERVICE LOAD OF 60 OR 80 KIPS AT THE INSTALLATION ANGLE WITH AN APPROPRIATE FACTOR OF SAFETY. ANY ANCHORS THAT FAIL DURING THE TESTING SHALL BE REPORTED TO THE OWNER'S REPRESENTATIVE AND THE ANCHOR SHALL BE REPLACED AT NO ADDITIONAL COST TO THE OWNER.

STRUCTURAL STEEL

- ALL STRUCTURAL STEEL AND MISCELLANEOUS METAL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH ALL APPLICABLE PROVISIONS OF THE LATEST:
- A. AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR
- B. AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"
- C. SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS (RCRBJ) D. AISC QUALITY CERTIFICATION PROGRAM.

- STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING MATERIAL SPECIFICATIONS: WIDE FLANGES AND SHAPES CUT FROM THEM ASTM A992

S-BEAMS, CHANNELS, AND ANGLES ASTM A36 ASTM A36 OR A572 GR. 50 (SEE PLAN) STEEL PLATES HOLLOW STRUCTURAL SECTIONS ASTM A500 GRADE B STEEL PIPE ASTM A53 GRADE B HIGH STRENGTH BOLTS ASTM A325-X OR ASTM F1852-X ASTM A563 NUTS WASHERS ASTM F436 WELDED STUDS ASTM A108 WELD ELECTRODES F70XX GALVANIZED BOLTS ASTM A307 STAINLESS STEEL BOLTS TYPE 304 OR 316 GALVANIZED NUT ASTM A563 GALVANIZED WASHER ASTM F436 WALL ANCHORAGE ASTM A193 GRADE B7

- WELDING SHALL BE PERFORMED IN ACCORDANCE WITH AWS D1.1 BY QUALIFIED WELDERS AS DEFINED BY AWS D1.1 & ALL UNDERWATER WELDING SHALL BE IN ACCORDANCE WITH AWS D3.6. ELEMENTS INVOLVING CONNECTIONS OF SHEET STEEL OR STRIP STEEL SHALL ALSO BE IN ACCORDANCE WITH THE

LATEST EDITION OF AWS D1.3. SPECIFICATION FOR WELDING SHEET STEEL IN STRUCTURES. – WELDING RODS UTILIZED IN UNDERWATER WELDING SHALL MEET THE STANDARDS OF AWS D3.6. - BOLTED CONNECTIONS SHALL BE WITH 1/2" O HIGH STRENGTH BOLTS CONFORMING TO ASTM A325 WITH THREADS EXCLUDED FROM THE SHEAR PLANE UNLESS INDICATED OTHERWISE ON THE DRAWINGS. THE

USE OF HIGH STRENGTH STEEL BOLTS SHALL BE GOVERNED BY "SPECIFICATION FOR STRUCTURAL JOINTS

- USING ASTM A325 OR A490 BOLTS". - FAYING SURFACES OF PRE-TENSIONED BOLTS SHALL BE FREE OF ALL LOOSE RUST, LOOSE MILL SCALE, AND LOOSE PAINT TO A DEGREE SPECIFIED BY HAND CHIPPING, SCRAPING, SANDING AND WIRE BRUSHING
- TO OBTAIN A CLASS A FAYING SURFACE. - BEVELED WASHERS SHALL BE UTILIZED WHEN THE OUTER FACE OF THE JOINT HAS A SLOPE THAT IS
- GREATER THAN 1:20 WITH RESPECT TO A PLANE THAT IS NORMAL TO THE BOLT AXIS. - A TENSION CALIBRATOR SHALL BE USED WHERE BOLTS ARE TO BE PRE-TENSIONED TO CONFIRM THE SUITABILITY OF THE COMPLETED FASTENER ASSEMBLE, INCLUDING LUBRICATION, FOR PRE-TENSIONED
- INSTALLATION; AND, TO CONFIRM THE PROCEDURE AND PROPER USE BY THE BOLTING CREW OF THE PRE-TENSIONING METHOD TO BE USED. - THE ACCURACY OF A HYDRAULIC TENSION CALIBRATOR SHALL BE CONFIRMED THROUGH CALIBRATION AT
- LEAST ANNUALLY.
- THE HARDWARE SHALL BE PRE-TENSIONED ONCE THE SURFACES OF THE JOINING MATERIAL ARE IN FIRM
- PROVIDE FIRE WATCH DURING ALL WELDING, FLAME CUTTING, AND BURNING.
- STEEL MEMBERS AND CONNECTIONS WERE DESIGNED ASSUMING A TYPE PR (PARTIALLY RESTRAINED) OR "SIMPLE FRAMING" CONSTRUCTION TYPE.
- ALL BOLTED CONNECTIONS SHALL BE SNUG TIGHTENED PER RCSC 2009. - PRE-TENSIONED BOLTED CONNECTIONS SHALL BE PRE-TENSIONED PER THE PLANS.
- ALL HARDWARE AND STRUCTURAL STEEL SHAPES TO BE PAINTED SHALL BE DESIGNATED IN THE CONSTRUCTION DOCUMENTS OR PROJECT SPECIFICATIONS.
- ALL HARDWARE AND STRUCTURAL STEEL SHAPES TO BE PAINTED SHALL BE COMPLETED IN ACCORDANCE WITH THE GENERAL SPECIFICATIONS AND PLANS OR GALVANIZED BY THE HOT DIPPED PROCESS IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM A123 AND/OR A153, AS APPLICABLE, AFTER
- FABRICATION, UNLESS OTHERWISE NOTED. - FIELD TREAT DAMAGED GALVANIZED FINISH WITH TWO COATS OF HIGH ZINC DUST OXIDE PAINT, COLD GALVANIZING COMPOUNDS OR APPROVED EQUAL CONFORMING TO THE REQUIREMENTS OF ASTM A780. - FIELD TREAT ALL COATING DAMAGED WITH MANUFACTURERS RECOMMENDED REPAIR PROCESS. ALL PROCESSES FOR REPAIR MUST BE APPROVED BY THE ENGINEER PRIOR TO THE REPAIR.
- ALL TOPSIDE CONCRETE AND CONCRETE MATERIALS TO BE DESIGNED, MIXED AND PLACED IN ACCORDANCE WITH THE STANDARDS AND RECOMMENDATIONS OF THE LATEST ACI-318 CODE FOR REINFORCED
- ALL NEW CONCRETE PLACED UNDERWATER SHALL FOLLOW CHAPTER 8 OF ACI 304R "GUIDE FOR MEASURING, MIXING, TRANSPORTING, AND PLACING CONCRETE" AND ALL EXISTING CONCRETE BELOW THE WATER SHALL BE REPAIRED IN ACCORDANCE TO ACI 546.2R "GUIDE TO UNDERWATER REPAIR OF CONCRETE".
- CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 4000 PSI UNLESS NOTED OTHERWISE.
- CONCRETE FOR BOLLARD FOUNDATIONS SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF
- CONCRETE SLUMP SHALL BE 4" ± 1" WITH ALLOWANCE TO USE SUPERPLASTICIZING ADMIXTURE TO INCREASE UP TO 8" IF DESIRED.
- ENTRAINED AIR SHALL BE $6\% \pm 1.5\%$ (MEASURED AT TRUCK DISCHARGE).
- MAXIMUM COARSE-AGGREGATE SIZE OF SHALL BE 34". - CONCRETE SHALL BE DESIGNED TO A MINIMUM WATER-CEMENTITIOUS MATERIAL RATIO TO LIMIT SHRINKAGE AND PRODUCE MAXIMUM DURABILITY. MAXIMUM WATER-CEMENTITIOUS RATIO SHALL BE 0.40.
- CEMENT USED SHALL BE PORTLAND CEMENT MEETING THE REQUIREMENTS OF ASTM C150, TYPE II. ALL AGGREGATES SHALL CONFORM TO ASTM C33.
- ALL FLY ASH INCLUDED IN THE CONCRETE SHALL BE TYPE F MEETING THE REQUIREMENTS OF ASTM
- ALL SLAG CEMENT INCLUDED IN THE CONCRETE SHALL MEET THE REQUIREMENTS OF ASTM C989. - ALL SILICA FUME INCLUDED IN THE CONCRETE SHALL MEET THE REQUIREMENTS OF ASTM C1240.
- ALL HORIZONTAL REINFORCING BARS SHALL BE CONTINUOUS AROUND CORNERS. - WATER USED TO BE POTABLE AND FREE OF DEBRIS, OIL, AND OTHER DELETERIOUS SUBSTANCES AND MEET THE REQUIREMENTS OF ASTM C1602. - CONCRETE PLACED SHALL BE THOROUGHLY COMPACTED DURING PLACEMENT BY VIBRATING.

- FOLLOW ACI 305 AND ACI 306 FOR HOT AND COLD WEATHER CONSTRUCTION WHEN APPLICABLE.

- ALL FORMING SHALL BE TRUE AND STRAIGHT IN ACCORDANCE WITH ACI STANDARDS. - ALL HORIZONTAL CONCRETE SURFACES SHALL HAVE A BROOM FINISH. ALL OTHER SURFACES SHALL HAVE SMOOTH FORM FINISH
- SUFFICIENT CURING SHALL BE PROVIDED FOR CONCRETE. WET CURE WITH BURLAP AND WATER IMMEDIATELY AFTER FINISHING AND FOR A MINIMUM OF 7 DAYS. ALTERNATELY USE CURING COMPOUND
- ON FORMED SIDES, ONLY AFTER REMOVAL OF FORMS. CURE AS PER ACI STANDARDS. - ALL EMBEDDED ITEMS AND OPENINGS REQUIRED FOR MOORING, UTILITY, ELECTRICAL, AND MECHANICAL SERVICES SHALL BE INCORPORATED INTO THE STRUCTURES WHETHER OR NOT THEY ARE DETAILED OR INDICATED ON THE STRUCTURAL / MARINE DRAWINGS. ALL OPENING DIMENSIONS SHOWN ON THE DRAWINGS FOR MECHANICAL EQUIPMENT SHALL BE COORDINATED WITH THE ACTUAL EQUIPMENT TO BE USED, AND THE DIMENSIONS ADJUSTED IN ACCORDANCE WITH THE MANUFACTURER'S REQUIREMENTS. REFER TO THE OTHER DISCIPLINE DRAWINGS FOR FLOOR FINISHES, SLOPES, DRAINS, DEPRESSIONS,
- CELLS, EQUIPMENT PADS, ETC. - ALL ITEMS EMBEDDED INTO CONCRETE SHALL BE CLEANED TO REMOVE ALL LOOSE SCALE, RUST, AND MARINE GROWTH. REFER TO THE RESPECTIVE DRAWINGS FOR THE DETAILS OF EMBEDDED ITEMS AND
- ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED ¾" BY ¾" UNLESS SHOWN OTHERWISE. - THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO OWNERS REPRESENTATIVE FOR REINFORCING STEEL
- BEFORE PROCEEDING WITH FABRICATION. - ALL OPENING DIMENSIONS SHOWN ON THE DRAWINGS FOR MECHANICAL EQUIPMENT SHALL BE COORDINATED WITH THE ACTUAL EQUIPMENT TO BE USED, AND THE DIMENSIONS ADJUSTED IN ACCORDANCE WITH THE MANUFACTURER'S REQUIREMENTS. REFER TO THE OTHER DISCIPLINE DRAWINGS FOR FLOOR FINISHES, SLOPES, DRAINS, DEPRESSIONS, CELLS, EQUIPMENT PADS, ETC.

VIBRO COMPACTION

- VIBRO COMPACTION WILL BE REQUIRED FOR ALL MATERIAL PLACED IN WATER DEPTHS GREATER THAN ONE
- CONTRACTOR SHALL SUBMIT VIBRO COMPACTION EQUIPMENT AND PROCEDURE FOR APPROVAL PRIOR TO STARTING WORK.

CAMEL SYSTEM

- A CAMEL SYSTEM WILL BE REQUIRED TO SEPARATE THE S.S. WILLIAM A. IRVIN FROM THE EXISTING DOCKWALL
- CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR PROPER DESIGN OF CAMEL SYSTEMS.

DEPENDING ON LOCAL REQUIREMENTS OR PRACTICES.

 CONTRACTOR SHALL SUBMIT CAMEL DESIGN INFORMATION TO ENGINEER FOR APPROVAL - DESIGN OF CAMEL SYSTEM SHALL BE PERFORMED BY AN ENTITY AS REQUIRED IN ACCORDANCE WITH EXISTING LOCAL CODE REQUIREMENTS OR ESTABLISHED LOCAL PRACTICES. THIS DESIGN WORK MAY BE PERFORMED BY A LICENSED PROFESSIONAL ENGINEER, A CERTIFIED CAMEL SYSTEM DEALER, OR DESIGNER

REINFORCING STEEL

- ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60, WITH A MINIMUM YIELD POINT OF
- ALL REINFORCING BAR DIMENSIONS SHOWN ON THE DRAWINGS ARE TO CENTERLINE OF BARS UNLESS
- NOTED OTHERWISE. - DETAIL REINFORCING STEEL IN ACCORDANCE WITH "ACI DETAILING MANUAL" ACI SP66 AND "CRSI: MANUAL OF STANDARD PRACTICE AND RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS", EXCEPT WHERE
- DEVELOPMENT LENGTH AND LAP SPLICE LENGTH OF REINFORCING BARS SHALL BE AS SHOWN ON THE PLANS. ALL REINFORCING BAR SPLICES SHALL BE CLASS "B" TENSION LAP SPLICES, UNLESS NOTED
- OTHERWISE. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
- REINFORCING MEETING ASTM A615 SHALL NOT BE WELDED WITHOUT PRIOR APPROVAL FROM ENGINEER. - WELDED WIRE FABRIC SHALL BE LAPPED TWO FULL MESH PANELS AND TIED SECURELY.
- REINFORCING TO BE WELDED SHALL BE WELDABLE REBAR MEETING ASTM A706 SPECIFICATION OR DEFORM BAR ANCHORS MEETING ASTM A496 STANDARD. ALL WELDING OF REBAR SHALL BE SUBMITTED FOR APPROVAL TO ENGINEER PRIOR TO WELDING.

CEMENTITIOUS GROUT

- CEMENTITIOUS GROUT PLACED: A. UNDERWATER SHALL BE SEASHIELD 510 UNDERWATER GROUT PROVIDED BY DENSO NORTH AMERICA
- OR APPROVED EQUAL. B. TOPSIDE SHALL BE MASTERFLOW 713 PROVIDED BY BASF OR APPROVED EQUAL - CONTRACTOR SHALL FOLLOW ALL OF MANUFACTURERS RECOMMENDATIONS OF POT LIFE AND
- TEMPERATURE REQUIREMENTS DURING MIXING AND PLACEMENT OF PRODUCT.
- WATER USED IN MIXTURE SHALL BE POTABLE AND FREE OF OIL, GREASE, & DEBRIS. - FOLLOW THE MANUFACTURERS RECOMMENDATION FOR THE USED OF ADMIXTURES.
- FINE AGGREGATE SHALL MEET THE REQUIREMENTS OF ASTM C33. - ITEMS EMBEDDED INTO THE GROUT SHALL BE FREE OF LOOSE SCALE, RUST, AND MARINE GROWTH
- SURFACES MUST BE CLEANED OF ALL OILS, GREASES, DIRT, WAX SOLUTIONS, AND OLD COATINGS. - SURFACE SHOULD BE CLEANED WITH A HIGH PRESSURE WATERBLAST, SANDBLAST, OR OTHER APPROVED
- METHODS TO REMOVE ALL CONTAMINANTS WHICH MIGHT INTERFERE WITH PROPER ADHESION. - METAL SURFACES SHALL BE CLEANED TO A BARE METAL SURFACE AND CONCRETE SURFACES SHALL BE FREE OF WEAK & LOOSE CONCRETE BY CHIPPING DOWN TO SOUND CONCRETE.
- ALL GROUT SHALL BE PLACED BY MEANS OF TREMIE OR PUMP.

ENERGY CONTROLS

 CONTRACTOR IS RESPONSIBLE FOR COORDINATING WITH THE PROJECT OWNER TO INSURE A SAFE WORK AREA, INCLUDING THE LOCKOUT OF MACHINERY, VESSEL PROPS, AND SHIP EQUIPMENT WHEN REQUIRED.

- THE FOLLOWING PERMITS/APPROVALS HAVE BEEN OBTAINED FOR THIS PROJECT: A. DNR PERMIT FOR DOCK REHABILITATION — PENDING
- B. ARMY CORPS OF ENGINEERS PERMIT FOR DOCK REHABILITATION PENDING - CONTRACTOR IS RESPONSIBLE FOR ANY ADDITIONAL DISPOSAL, OR LOCAL PERMITTING NECESSARY BEFORE THE START OF THE PROJECT.

WORK SCHEDULE & DELAYS

- ALL DOCK CONSTRUCTION MUST BE COMPLETED DURING SCHEDULED DOWN TIME PERIODS TO PREVENT INTERFERENCE WITH PIER/DOCK OPERATIONS. THE CONTRACTOR SHALL COORDINATE WITH DOCK OWNER TO SCHEDULE WORK.
- THE FOLLOWING SCHEDULE MILESTONES MUST BE MET:
- APRIL 27, 2018: COMPLETION AND ACCEPTANCE BY ENGINEER/OWNER OF DOCK WALL AND ANCHORAGE SYSTEM, BOLLARD FOUNDATION AND ANCHORAGE SYSTEM, AND FENDERING SYSTEM. JUNE 1, 2018: COMPLETION AND ACCEPTANCE BY ENGINEER/OWNER OF PROJECT.
- ALL WORK THAT MAY BE CONSIDERED A PUBLIC NOISE NUISANCE AS DETERMINED BY ENGINEER/OWNER (E.G. PILE DRIVING) SHALL BE PERFORMED BETWEEN 7:00 AM AND 7:00 PM. NO PUBLIC NOISE
- NUISANCE WORK MAY BE PERFORMED OUTSIDE OF THESE HOURS UNLESS APPROVED BY - LIQUIDATED DAMAGES WILL BE \$1,000 PER CALENDAR DAY FOR EVERY CALENDAR DAY THAT SCHEDULE
- MILESTONES ARE NOT MET. CONSEQUENTIAL DAMAGES WILL BE CHARGE TO CONTRACTOR IF WORK IS NOT COMPLETED AND ACCEPTED BY JUNE 15, 2018.

SHEET PILE SEAWALL

- HOT ROLLED STEEL SHEET PILING SHALL CONFORM TO ASTM A6 "STANDARD SPECIFICATION FOR GENERAL REQUIREMENTS FOR ROLLED STRUCTURAL STEEL BARS, PLATES, SHAPES, AND SHEET PILING" WHILE COLD ROLLED STEEL SHEET PILING SHALL CONFORM TO ASTM A857 "STANDARD SPECIFICATION FOR STEEL SHEET PILING, COLD FORMED, LIGHT GAGE,"
- ALL STEEL SHEET PILE SHALL BE ASTM A572 GR 50. SHEET PILE SHALL HAVE A MINIMUM SECTION MODULUS OF Sx=33.5 IN³/FT OF WALL AND A MINIMUM THICKNESS OF 0.375" ON FLANGES AND WEB. - IF THE STEEL SHEET PILE SIZE DESIGNATED ON THE PLANS ARE NOT UTILIZED, THE CONTRACTOR IS
- RESPONSIBLE FOR ANY ADDITIONAL DRAWINGS, ENGINEERING DEVIATIONS, OR CONSTRUCTION/QUANTITY CHANGES INCLUDING ALL ASSOCIATED COSTS - ALL STEEL SHEET PILE SIZES TO BE UTILIZED DURING CONSTRUCTION SHALL BE SUBMITTED TO THE
- ENGINEER FOR APPROVAL PRIOR TO PURCHASE. - FOR PILE TIP ELEVATIONS, SEE RELEVANT DRAWINGS. - PILES SHALL BE DRIVEN WITHIN 2 INCHES FROM BASELINE INDICATED ON PLANS AFTER DRIVING IS
- PILES SHALL MAINTAIN 1/4 INCH PER FOOT OUT OF PLUMB TOLERANCE IN THE PARALLEL AND PERPENDICULAR PLANE OF THE WALL.
- TEMPORARY WALES, TEMPLATES, OR GUIDE STRUCTURES SHALL BE USED TO INSURE THAT THE PILING IS PLACED AND DRIVEN WITHIN THE ALIGNMENT TOLERANCES IN ALL PLANES. - PILING DAMAGED OR DRIVEN OUTSIDE THE ABOVE TOLERANCES SHALL BE REMOVED & REPLACED WITH NO
- ADDITIONAL EXPENSE TO THE OWNER. - THE CONTRACTOR SHALL PREPARE DRIVING RECORDS FOR ALL PILES INCLUDING PROJECT NAME AND NUMBER, NAME OF CONTRACTOR, PILE LOCATION AND IDENTIFICATION MARK, SEQUENCE OF DRIVING, PILE DIMENSIONS, GROUND ELEVATION, SPLICE LOCATIONS AND TYPE, TIP AND CUTOFF ELEVATIONS AFTER DRIVING PILE GROUP, PLUMBNESS CHECKS BEFORE AND AFTER DRIVING, VERTICAL AND HORIZONTAL ALIGNMENT VERIFICATIONS, DATE, START & FINISH TIMES OF DRIVING, TOTAL DRIVING TIME, AND ANY UNUSUAL OCCURRENCES. DAILY RECORDS SHALL BE TURNED OVER TO THE ENGINEER FOR REVIEW THE
- SAME DAY THE WORK IS PERFORMED. - THE CONTRACTOR IS RESPONSIBLE FOR CLEARING THE DRIVING LINE OF ANY DEBRIS UP TO 5 FT BELOW THE EXISTING MUD LINE WHICH MAY DAMAGE OR PREVENT THE INSTALLATION OF THE SHEET PILE.
- DESIGN OF THE SHEET PILE WALL WAS BASED OFF OF SOILS INFORMATION PROVIDED BY LAKEHEAD

TESTING REPORT #910-87-271 DATED MAY 28, 1987.

STEEL PILE FOUNDATIONS

- ALL STEEL PILES SHALL BE HP10x42 OR HP14x73 PILES (SEE PLAN FOR LOCATIONS) CONFORMING TO
- ASTM A572 GRADE 50 UNLESS NOTED OTHERWISE. - NO SECONDS, REJECT, OR USED PILING WILL BE CONSIDERED UNLESS APPROVED BY ENGINEER.
- SEE PLAN FOR ESTIMATED PILE LENGTHS AND PILE TIP ELEVATIONS.
- ALL PILES SHALL BE DRIVEN OPEN ENDED.
- SEE PLANS FOR PILE ALLOWABLE COMPRESSION DESIGN CAPACITIES AND ADDITIONAL TESTING NOTES. - PILES SHALL BE DRIVEN HORIZONTALLY WITHIN 3 INCHES OF THE PLAN LOCATION AND PILES SHALL NOT BE OUT OF PLUMB VERTICALLY BY MORE THAN 2% FOR PROPER ALIGNMENTS. - PILES MAY BE INSTALLED WITH VIBRATORY HAMMERS FOR THE FIRST 20 FT. THE FINAL 10 FT MUST BE
- DRIVEN WITH AN IMPACT HAMMER. - THE CONTRACTOR SHALL PREPARE DRIVING RECORDS FOR ALL PILES INCLUDING PILE IDENTIFICATION MARK. TYPE, SIZE AND LENGTH OF PILE, PILE HAMMER USED, HAMMER SPEED AND PRESSURE, SPLICE LOCATIONS AND TYPE. TIP AND CUTOFF ELEVATIONS. PLUMBNESS CHECKS BEFORE AND AFTER DRIVING. DRIVING RESISTANCE PER FOOT, DATE AND TIME OF DRIVING, AND ANY UNUSUAL OCCURRENCES. DAILY
- RECORDS SHALL BE TURNED OVER TO THE ENGINEER FOR REVIEW. - PILING DAMAGED OR DRIVEN OUTSIDE THE ABOVE TOLERANCES SHALL BE REMOVED AND REPLACED WITH NO ADDITIONAL EXPENSE TO THE OWNER.
- TEST PILES SHALL BE DRIVEN TO DETERMINE FINAL CAPACITIES AND LENGTHS ACCORDING TO TESTING NOTES, CONSTRUCTION DOCUMENTS, AND PROJECT SPECIFICATIONS.
- SOILS INFORMATION FOR THE DESIGN OF THE STEEL PILING WERE BASED OFF OF SOILS INFORMATION PROVIDED BY LAKEHEAD TESTING REPORT #910-87-271 DATED MAY 28, 1987.
- UPON COMPLETION OF PILE DRIVING, AN OWNER REPRESENTATIVE SHALL FURNISH A SURVEY OF AS-DRIVEN PILE LOCATIONS. THE SURVEY SHALL INDICATE THE MISALIGNMENT OF EACH PILE IN TOW PERPENDICULAR DIRECTIONS, GIVEN IN INCHES, AND ACTUAL CUTOFF ELEVATIONS OF EACH PILE.

HELICAL SCREW & ANCHOR PILES

- HELICAL PIERS SHALL BE MANUFACTURED BY CHANCE™ OR APPROVED EQUAL. - PILES SHALL BE INSTALLED BY AN AUTHORIZED HELICAL SCREW PILE INSTALLING CONTRACTOR WHO HAS SATISFIED THE CERTIFICATION REQUIREMENTS RELATING TO THE TECHNICAL ASPECTS OF THE PRODUCT AND THE ASCRIBED INSTALLATION TECHNIQUES. PROOF OF CURRENT CERTIFICATION MUST BE PROVIDED.
- ALL PILES SHALL BE CORROSION PROTECTED BY HOT DIP GALVANIZATION. INSTALLATION UNITS SHALL BE CAPABLE OF DEVELOPING THE MAXIMUM TORQUE AS REQ'D.
- INSTALLATION UNIT SHALL BE CAPABLE OF POSITIONING THE HELICAL PIER AT THE PROPER INSTALLATION ANGLE. THIS ANGLE MAY VARY BETWEEN VERTICAL AND 5 DEGREES DEPENDING UPON APPLICATION AND TYPE OF LOAD TRANSFER DEVICE SPECIFIED OR REQUIRED.
- PILES SHALL BE DRIVEN WITHIN 3 INCHES OF PILE LOCATIONS. - CONTRACTOR SHALL PROTECT AND SHORE ROADWAY AS REQUIRED DURING INSTALLATION OF HELICAL
- HELICAL PIERS SHALL BE INSTALLED TO THE MAXIMUM TORQUE VALUE REQUIRED TO PROVIDE THE LOAD CAPACITIES SHOWN ON THE PLANS
- PIERS SHALL BE INSTALLED WITH A ROTARY TYPE MOTOR WITH EQUAL FORWARD AND REVERSE TORQUE CAPABILITIES. THIS EQUIPMENT SHALL BE CAPABLE OF CONTINUAL ADJUSTMENT OF THE TORQUE DRIVE UNIT'S REVOLUTIONS PER MINUTE (RPM'S) DURING INSTALLATION. PERCUSSION DRILLING EQUIPMENT SHALL NOT BE ALLOWED.
- INSTALLATION EQUIPMENT SHALL BE CAPABLE OF APPLYING AXIAL COMPRESSION (CROWD) PRESSURE AND TORQUE SIMULTANEOUSLY.
- THE TORQUE BEING APPLIED BY THE INSTALLING UNITS SHALL BE MONITORED THROUGHOUT THE INSTALLATION BY THE INSTALLER. THE TORQUE MONITORING DEVICE SHALL EITHER BE A PART OF THE INSTALLING UNIT OR AN INDEPENDENT DEVICE IN-LINE WITH THE INSTALLING UNIT. CALIBRATION FOR EITHER UNIT SHALL BE AVAILABLE FOR REVIEW BY THE OWNER.
- THE APPROPRIATE STEEL NEW CONSTRUCTION LOAD TRANSFER DEVICE SHALL BE USED. APPROPRIATE HELICAL PIER SELECTION WILL CONSIDER DESIGN LOAD PLUS SAFETY FACTOR, SOIL PARAMETERS, AND THE INSTALLATION TORQUE VS. CAPACITY EQUATION AS PER THE MANUFACTURERS RECOMMENDATIONS.
- ACCORDANCE WITH EXISTING LOCAL CODE REQUIREMENTS OR ESTABLISHED LOCAL PRACTICES. THIS DESIGN WORK MAY BE PERFORMED BY A LICENSED PROFESSIONAL ENGINEER, A CERTIFIED HELICAL PIER DEALER, OR DESIGNER DEPENDING ON LOCAL REQUIREMENTS OR PRACTICES. - SOILS INFORMATION FOR THE PROJECT PROVIDED BY LAKEHEAD TESTING REPORT #910-87-271 DATED

- DESIGN OF HELICAL SCREW PILES AND ANCHORS SHALL BE PERFORMED BY AN ENTITY AS REQUIRED IN

- UPON COMPLETION OF PILE DRIVING, AN OWNER REPRESENTATIVE SHALL FURNISH A SURVEY OF AS-DRIVEN PILE LOCATIONS. THE SURVEY SHALL INDICATE THE MISALIGNMENT OF EACH PILE IN TOW

PERPENDICULAR DIRECTIONS, GIVEN IN INCHES, AND ACTUAL CUTOFF ELEVATIONS OF EACH PILE.

- **BOLLARDS** UNLESS OTHERWISE NOTED, NEW CAST STEEL BOLLARDS SHALL BE T-HEAD TYPE, MODEL #1352 PROVIDED BY SCHOELLHORN-ALBRECHT OR APPROVED EQUAL AND INSTALLED PER MANUFACTURER'S
- RECOMMENDATIONS. - NEW CAST STEEL BOLLARDS BETWEEN STATION 0+00 TO 6+70 SHALL BE STAGHORN TYPE, MODEL #S1547 PROVIDED BY SCHOELLHORN-ALBRECHT OR APPROVED EQUAL AND INSTALLED PER
- MANUFACTURER'S RECOMMENDATIONS. - EQUIVALENT BOLLARDS PROVIDED BY OTHER MANUFACTURERS MUST BE APPROVED BY ENGINEER PRIOR - IF AN EQUIVALENT BOLLARD IS APPROVED BY THE ENGINEER, THE CONTRACTOR IS RESPONSIBLE FOR ANY
- ADDITIONAL DRAWINGS, ENGINEERING DEVIATIONS, OR CONSTRUCTION/QUANTITY CHANGES INCLUDING ALL ASSOCIATED COSTS.
- ANCHOR BOLTS FOR BOLLARDS SHALL HAVE A MINIMUM EMBEDMENT OF 3'-0", AND SHALL CONFORM TO DIAMETERS AND GRADES RECOMMENDED BY MANUFACTURER. - NUTS AND WASHERS FOR BOLLARDS SHALL CONFORM TO MANUFACTURER'S RECOMMENDATIONS. - ALL ANCHORAGE HARDWARE SHALL BE HOT DIPPED GALVANIZED TO ASTM A123 OR A153.
- BOLLARD WITH A MINIMUM COMPRESSIVE STRENGTH OF 5,000 PSI WITH A MAXIMUM AGGREGATE SIZE OF - BOLLARD SHALL BE FINISHED WITH (2) - 2 MIL DFT COATS OF DURA-PLATE 235 PW EPOXY PRIMER

- NON-SHRINK GROUT SHALL BE USED TO FILL THE INTERNAL CAVITY AND AROUND THE BASE OF THE

- AND (1) 2 MIL DFT COAT OF ACROLON 218 HS ACRYLIC POLYURETHANE BY THE SHERWIN WILLIAMS CORP OR EQUAL. - BOLLARD SHALL BE CLEANED WITH A POWER TOOL TO A MINIMUM SURFACE PREPARATION AS DEFINED BY
- SSPC-SP10 PRIOR TO TO THE APPLICATION OF ANY COATING. - BOLLARD FINISH SHALL BE OSHA COLOR YELLOW (SHERWIN WILLIAMS SAFETY YELLOW SW 4084 OR
- EXISTING BOLLARDS, CLEATS, AND MOORING HARDWARE SHALL BE REMOVED AND NOT REUSED. - ANCHOR BOLT HEADS AND CAVITIES FOR THE ANCHOR BOLTS SHALL BE FILLED WITH AN APPROVED EPOXY TO PREVENT STANDING WATER. - 5000 PSI CONCRETE SHALL BE USED FOR ALL BOLLARD FOUNDATIONS.

- UNLESS OTHERWISE NOTED, NEW CAST STEEL CLEATS SHALL BE MODEL #1460-24 PROVIDED BY SCHOELLHORN-ALBRECHT OR APPROVED EQUAL AND INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.

- CLEAT SHALL BE MADE OF CAST STEEL MEETING ASTM A27 GRADE 70-36.

- BOLLARD CAPACITY SHALL BE STAMPED INTO WET CONCRETE BOLLARD FOUNDATION IMMEDIATELY IN FRONT

OF BOLLARD LOCATION.

- STRUCTURAL AND MISC STEEL PAINT - PAINT SHALL BE HI-SOLIDS POLYURETHANE BY THE SHERWIN WILLIAMS CORP OR EQUAL AND APPLIED
- PER MANUFACTURERS RECOMMENDATIONS. - PAINT SHALL BE APPLIED WITH ZINC RICH PRIMER AND IN TWO TOP COATS EACH 4 MIL DFT. COLOR SHALL BE VERIFIED WITH OWNER PRIOR TO APPLICATION. - ALL SURFACES TO BE COATED SHALL BE SAND-BLASTED IN PREPARATION FOR APPLICATION OF THE

COATING. SAND-BLASTING SHALL BE TO NEAR WHITE METAL, AT LEAST EQUIVALENT TO A COMMERCIAL

BLAST AS DEFINED BY SSPC-SP10. ALL SURFACES TO BE COATED MUST BE COMPLETELY DRY, FREE

OF MOISTURE, SOIL, DUST, AND GRIT AT THE TIME THE COATING IS APPLIED. - COATING SHALL BE APPLIED PER MANUFACTURERS SPECIFICATIONS. - ANY COATINGS DAMAGED FROM FASTENERS, WELDING, OR DURING INSTALLATION SHALL BE FIELD REPAIRED

- ALL COATINGS PAINT SHALL BE A HIGH SOLIDS TWO COMPONENT POLYURETHANE RESIN COATING.

WITH THE APPROPRIATE COATING PER THE MANUFACTURERS RECOMMENDATIONS.



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SHEET:

TIMBER RUB RAIL

- ALL TIMBER ELEMENTS SHALL BE MANUFACTURED AND DESIGNED IN ACCORDANCE WITH ANSI/AWC
- NATIONAL DESIGN SPECIFICATIONS (NDS). - SINGLE ROUGH SAWN TIMBER FENDER RUB RAIL SHALL BE INSTALLED ALONG THE LENGTH OF THE DOCK.
- SEE DRAWINGS FOR MORE DETAILS AND CHAMFER REQUIREMENTS. - WOOD SPECIES SHALL BE OAK MATERIAL (WHITE OR RED) GRADED #1 OR BETTER, AS DETERMINED BY
- NELMA, NLGA, SPIB, WCLIB OR WWPA. - TIMBERS SHALL BE GRADE STAMPED OR VISUALLY CERTIFIED BY AN INDEPENDENT TESTING AGENCY.
- LETTER OF CERTIFICATION MUST BE SUBMITTED TO ENGINEERS PRIOR TO INSTALLATION.
- GENERAL: COMPLY WITH DOC PS 20 AND GRADING RULES OF LUMBER GRADING AGENCIES CERTIFIED BY AMERICAN LUMBER STANDARDS COMMITTEE BOARD OF REVIEW, AS APPLICABLE.
- A. FACTORY MARK EACH ITEM OF TIMBER WITH GRADE STAMP OF GRADING AGENCY. B. FOR EXPOSED TIMBER INDICATED TO RECEIVE A STAINED OR NATURAL FINISH, APPLY GRADE STAMPS TO SURFACES THAT WILL NOT BE EXPOSED TO VIEW OR OMIT GRADE STAMPS AND PROVIDE
- CERTIFICATES OF GRADE COMPLIANCE ISSUED BY GRADING AGENCY. - MOISTURE CONTENT: NO WARRANTY REQUIRED IF TIMBER IS PROVIDED WITH 22 PERCENT MAXIMUM MOISTURE CONTENT AT TIME OF DRESSING. ONE YEAR WARRANTY OF TIMBER RUB RAIL MAINTAINING A GRADE #1 AS DETERMINED BY NELMA, NLGA, SPIB, WCLIB, OR WWPA WILL BE REQUIRED IF TIMBER IS PROVIDED WITH MOISTURE CONTENT AT TIME OF DRESSING BETWEEN 22 PERCENT AND 30 PERCENT.
- DRESSING: PROVIDE TIMBER THAT IS ROUGH SAWN (RGH). - END SEALER: MANUFACTURER'S STANDARD, TRANSPARENT, COLORLESS WOOD SEALER THAT IS EFFECTIVE IN RETARDING THE TRANSMISSION OF MOISTURE AT CROSS CUTS.

MOISTURE CONTENTS GREATER THAN 30 PERCENT AT TIME OF DRESSING WILL NOT BE ACCEPTED.

EARTH WORK

- ALL GEOTEXTILE FABRIC SHALL BE PROPEX GEOTEX 315ST OR APPROVED EQUIVALENT.
- CONTRACTOR TO PROVIDE ALL NECESSARY MEANS AND EQUIPMENT TO CONTAIN EXISTING. DEBRIS DURING DEMO INCLUDING; ABSORBENT BOOM, SCREEN FENCE, H.D. SILT FENCE, ETC.
- NON-FROST SUSCEPTIBLE (NFS) SOIL IS DEFINED AS NO MORE THAN 6% OF THE MASS OF UNDISTURBED SOILS OR FILL MATERIAL MUST PASS THROUGH A #200 MESH SIEVE IN ACCORDANCE WITH ASTM D422.
- ALL FILL SHALL BE COMPACTED TO 95% MODIFIED PROCTOR.
- THE AREA OF FILL ON THE BACKSIDE OF THE DOCK WALL WILL BE COVERED UNDER THE
- ACOE PERMIT FOR THE DOCK WALL. - UNCLASSIFIED EXCAVATION MATERIAL IS EXPECTED TO CONTAIN WOOD, CONCRETE, AND METAL DEBRIS.
- CONTAMINATED SOILS ARE EXPECTED TO BE ENCOUNTERED DURING EXCAVATION.

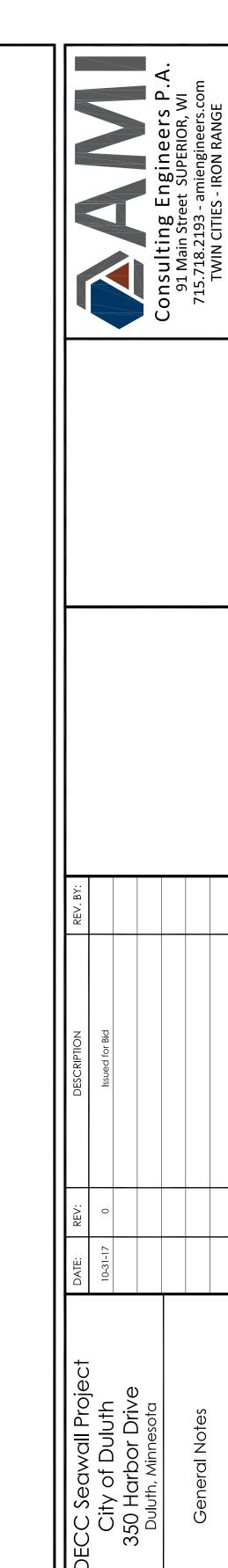
MARINE PILE COATINGS

- COAT (PAINT) STEEL SHEET PILES AND MISCELLANEOUS STRUCTURAL STEEL IN CORROSIVE ZONE AS NOTED ON DRAWINGS PER SPECIFICATIONS AND MANUFACTURER'S RECOMMENDATIONS. SEE DRAWING AND
- SPECIFICATIONS FOR SPECIFIC PAINTING DETAILS AND COVERAGE LIMITS. - THE PROCEDURE FOR DETERMINING DRY FILM THICKNESS OF THE COATING SHALL MEET OR EXCEED THE STANDARDS IN SSPC-PA 2: PROCEDURE FOR DETERMINING CONFORMANCE TO DRY COATING THICKNESS REQUIREMENTS.
- ALL SURFACES TO BE COATED SHALL BE SAND-BLASTED IN PREPARATION FOR APPLICATION OF THE COATING. SAND-BLASTING SHALL BE TO NEAR WHITE METAL, AT LEAST EQUIVALENT TO A NEAR WHITE BLAST AS DEFINED BY SSPC-SP10. ALL SURFACES TO BE COATED MUST BE COMPLETELY DRY, FREE OF MOISTURE, SOIL, DUST, AND GRIT AT THE TIME THE COATING IS APPLIED.
- SURFACE PROFILE SHALL BE MEET OR EXCEED THE SPECIFICATIONS AND MANUFACTURER'S RECOMMENDATIONS.
- WHERE WELDING OR FASTENINGS ARE TO BE ACCOMPLISHED AFTER INSTALLATION OF PILES, EQUIVALENT REPAIR COATING SHALL BE FIELD APPLIED. ALL DAMAGED AREAS MUST BE CLEANED AND PREPARED TO SAME STANDARDS PRIOR TO APPLICATION OF REPAIR COATING.
- THE COATING SHALL BE ALLOWED TO CURE A MINIMUM OF EIGHT DAYS PRIOR TO THE INSTALLATION OF
- EPOXY COATING PRODUCTS SHALL CONFORM TO THE SPECIFICATIONS AND RECOMMENDED MANUFACTURER SPECIFICATIONS. COATINGS SHALL BE MARINE GRADE FOR USE IN ENVIRONMENTS WITH HEAVY ICE AND SUBJECTED TO MICROBIOLOGICALLY INFLUENCED CORROSION. APPROVED PRODUCTS INCLUDE THE FOLLOWING:
 - A. HUMIDUR ME EPOXY COATING, ACOTEC UK, 12 FOREST DRIVE, BELLSHILL, ML4 2PT, NORTH LANARKSHIRE, SCOTLAND. CONTACT: PHILIP STARR, TEL. +44 (0) 7432-3204-87, EMAIL PHILIP.STARR@ACOTEC.BE
 - B. FAST CLAD ER EPOXY COATING, SHERWIN-WILLIAMS COMPANY, 4767 MILLER TRUNK HWY, HERMANTOWN, MN 55811. CONTACT: SCOTT SINNER, TEL. 218-722-7413, EMAIL SCOTT.A.SINNFR@SHFRWIN.COM.
 - C. INTERZONE 954 EPOXY COATING, 803 CORPORATE CT., WAUKESHA, WI 53189. CONTACT:
 - LARRY KELLY, TEL. 262-436-0595, EMAIL LARRY.KELLY@KELLYCOATINGS.COM. D. AQUAPURE SP EPOXY COATING, MARINE COATINGS LLC, 716 GARFIELD AVE., DULUTH, MN
- 55802, TEL. 218-349-6502. - A DIFFERENT PRODUCT OF EQUAL OR BETTER QUALITY MAY BE SUBSTITUTED BY THE CONTRACTOR WITH
- THE APPROVAL OF THE ENGINEER. POSITIVE PROOF OF COATING ADEQUACY TO WITHSTAND HEAVY ICE IMPACT AND ABRASION MUST BE PROVIDED FOR EVALUATION. - ALL SHEET PILING MUST BE PREPARED AND COATED IN INDIVIDUAL PIECES AS SHOWN ON DRAWINGS. NO
- PAIRING OF SHEET PILE WILL BE ALLOWED PRIOR TO COATING, EXCEPT FOR EXISTING IN-PLACE APPLICATIONS ON EXISTING SHEETING OR BY SPECIFIC PERMISSION OF ENGINEER. - VERIFY COATING COLOR PRIOR TO PURCHASE.
- SURFACE TO BE COATED AND COATING ADHESION SHALL BE TESTED ACCORDING TO THE TESTING NOTES, CONSTRUCTION DOCUMENTS, AND PROJECT SPECIFICATIONS.

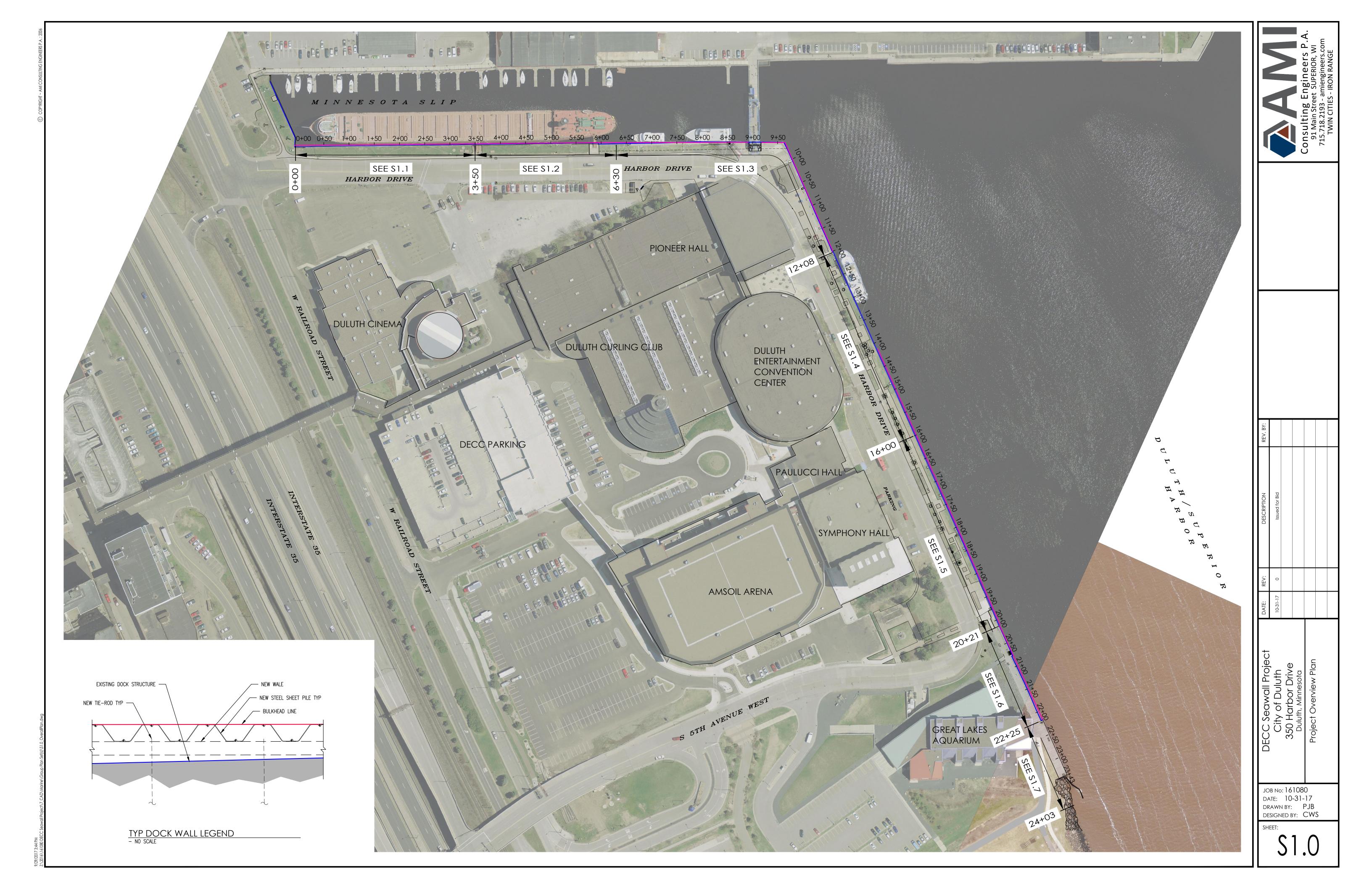
SUBMITTALS

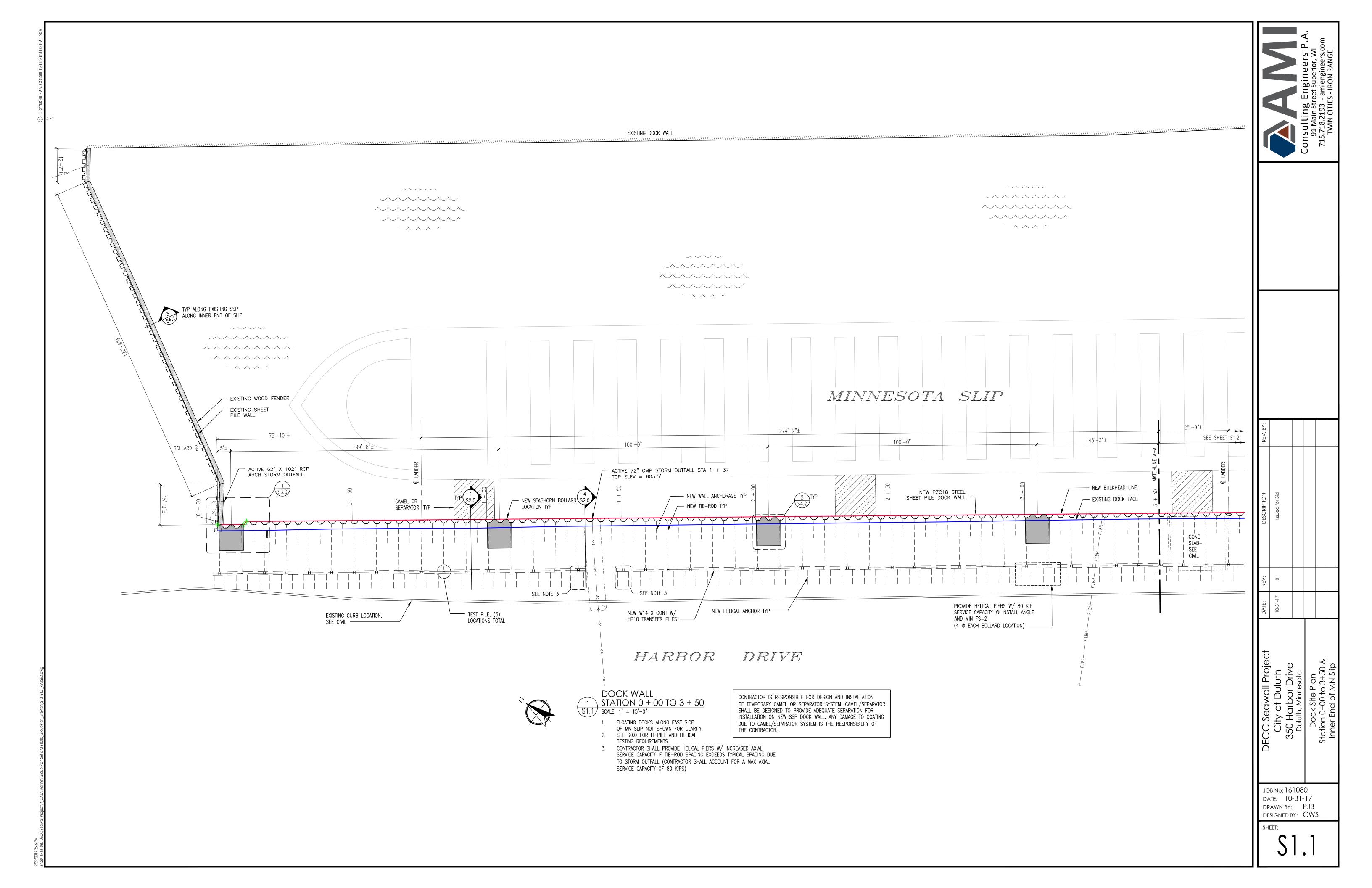
THE FOLLOWING ITEMS SHALL BE SUBMITTED AND APPROVED BY THE ENGINEER OR OWNERS REPRESENTATIVE PRIOR TO THE PURCHASE AND INSTALLATION OF THE ITEM

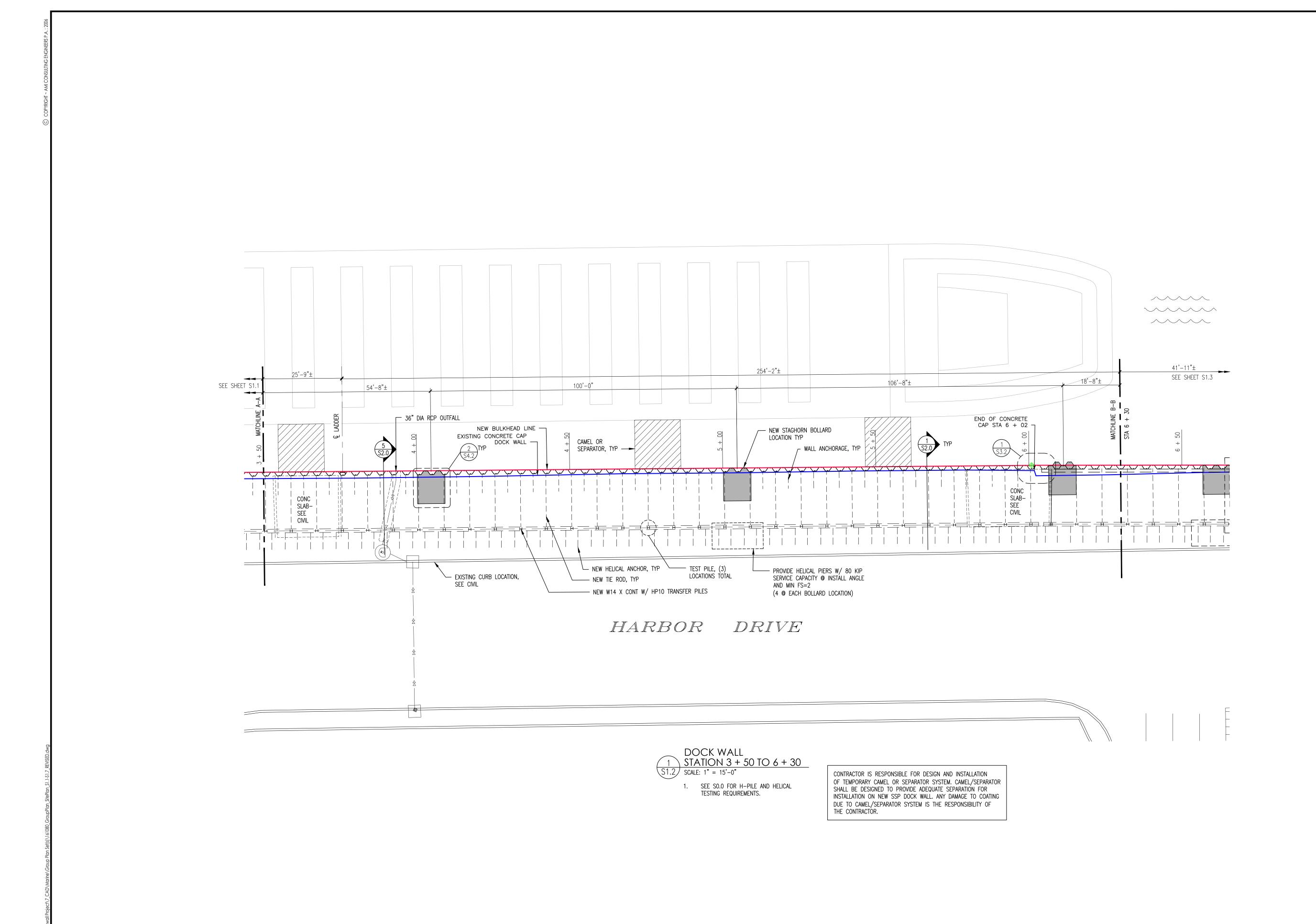
- BASELINE PROJECT SCHEDULE AND FOREMAN CONTACT INFORMATION
- BI-WEEKLY PROJECT SCHEDULE
- PRE-CONSTRUCTION PHOTOGRAPHS, VIDEOS, AND RELEVANT DOCUMENTATION - QUALITY CONTROL TEST RESULTS SHALL BE SUBMITTED WEEKLY FOR ALL QUALITY CONTROL TESTS PERFORMED BY CONTRACTOR
- CONCRETE A. MIX DESIGNS AND MATERIAL SPECIFICATIONS INCLUDING NON-SHRINK GROUT
- B. STEEL REINFORCING MATERIAL SPECIFICATIONS AND PLACEMENT DRAWINGS THAT DETAIL FABRICATION, BENDING, PLACEMENT, BAR SIZES, LENGTHS, AND SPLICE LENGTHS
- C. HOT/COLD WEATHER CONSTRUCTION PROCEDURES
- D. CONCRETE CURING PROCEDURE AND MATERIALS E. FORM-RELEASE AGENT
- F. PATCHING MATERIALS INCLUDING BONDING AGENT G. JOINT LAYOUT
- H. COMPRESSIBLE FILLER BOARD
- I. SILICONE JOINT SEALER
- J. CONCRETE FINISH - STRUCTURAL STEEL & SHEET PILING
- A. SHOP DRAWINGS INCLUDING MEMBER SIZES, LENGTHS, CUT & CONNECTIONS DETAILS, LENGTHS,
- SIZE, & NUMBER OF BOLTS, AND WELD SIZE, TYPE, LENGTH, & LOCATION.
- B. AWS D1.1 WELDING CERTIFICATES FOR PERSONNEL PERFORMING WELDING C. MATERIAL SPECIFICATIONS FOR STRUCTURAL SHAPES AND STEEL PLATES
- D. DAILY FIELD QUALITY CONTROL REPORTS INCLUDING PILE DRIVING LOGS, PILE DRIVING EQUIPMENT, VERTICAL AND HORIZONTAL ALIGNMENT VERIFICATIONS, FINAL TIP AND CUTOFF ELEVATIONS, OBSTRUCTIONS ENCOUNTERED WHILE DRIVING, HOURS WORKED, ETC. THE DAILY FIELD QUALITY
- CONTROL REPORT SHALL BE SUBMITTED THE SAME DAY THE WORK WAS PERFORMED. E. INSTALLER QUALIFICATIONS
- F. CERTIFIED COPIES OF MILL TEST REPORTS
- G. BASELINE DRIVING LANE
- H. PAINT AND PRIMER MATERIAL SPECIFICATIONS
- I. PILE TESTING PROCEDURE J. STEEL SHEET PILE CLOSURE PLATE DESIGN AND DRAWINGS
- K. TESTING AND INSTALLATION PROCEDURE FOR PRE-TENSIONED BOLTS
- L. PILE LOCATION PLAN VERIFYING ACTUAL LOCATIONS OF PILES AND DETAIL RECORDS FOR INDIVIDUAL PILES SHALL BEAR IDENTIFICATION - HELICAL SCREW AND ANCHOR PILES
- A. DESIGN DATA, DRAWINGS, TORQUE VS. CAPACITY EQUATION, AND PRODUCT INFORMATION INCLUDING
- CONNECTIONS. B. PERFORMANCE TESTING PROCEDURE
 - C. INSTALLER CERTIFICATION
 - D. INSTALLATION EQUIPMENT
- E. DAILY FIELD QUALITY CONTROL REPORTS INCLUDING DRIVING LOGS, EQUIPMENT, VERTICAL AND HORIZONTAL ALIGNMENT VERIFICATIONS, FINAL LENGTHS, OBSTRUCTIONS ENCOUNTERED WHILE INSTALLING, HOURS WORKED, ETC. THE DAILY FIELD QUALITY CONTROL REPORT SHALL BE
- SUBMITTED WITHIN 48 HOURS OF WHEN THE WORK WAS PERFORMED. A. MATERIAL SPECIFICATION FOR STEEL BOLLARD, NON-SHRINK GROUT, ANCHOR BOLTS, AND ANCHOR
 - BOLT FINISH
 - B. BOLLARD LOAD RATING, DESIGN CRITERIA, AND CERTIFICATES OF CONFORMANCE C. PAINT AND PRIMER SPECIFICATIONS FOR BOLLARD
- D. CAPACITY STAMPING PROCEDURE AND MATERIALS
- E. SHOP DRAWINGS TIMBER FENDERS
 - A. LETTER OF CERTIFICATION FROM THE SUPPLIER STATING THE SPECIES, GRADE, AND MOISTURE
 - CONTENT OF THE MEMBER. B. MATERIAL SPECIFICATIONS FOR BOLTS, NUTS, WASHERS, ANGLES AND OTHER CONNECTION
- C. END SEALER PRODUCT AND AND SPECIFICATIONS D. SEAL COAT PRODUCT AND SPECIFICATIONS
- E. WARRANTY LETTER DUE TO MOISTURE CONTENT OF FENDERS (IF REQUIRED) - MARINE PILE COATINGS A. COATING REPAIR PROCESS
- B. COATING PRODUCT AND MATERIAL SPECIFICATIONS
- VIBRO COMPACTION
- A. VIBRO COMPACTION EQUIPMENT AND PROCEDURE
- CAMEL SYSTEMS A. GENERAL ARRANGEMENTS OF CAMELS
 - B. DESIGN DATA, CALCULATIONS, AND DRAWINGS FOR CAMEL SYSTEMS
 - . MATERIAL SPECIFICATIONS FOR CAMEL SYSTEMS INCLUDING HARDWARE D. INSTALLATION AND REMOVAL PROCEDURES OF CAMEL SYSTEMS
- CLEATS A. PRODUCT DATA AND TECHNICAL SPECIFICATION
- B. CLEAT LOAD RATING AND CERTIFICATE OF CONFORMANCE C. PAINT AND PRIMER SPECIFICATIONS FOR CLEAT
- SITE SAFETY AND SITE CONTINGENCY PLAN A. CONTRACTOR IS RESPONSIBLE FOR SUBMITTING A SITE SAFETY AND SITE CONTINGENCY PLAN, AND ADHEREING TO THE RESPONSE ACTION PLAN (RAP) ADDRESSING THE REQUIREMENTS SET FORTH IN
- 29 CFR 1910.120 TESTING AGENCY
- A. QUALIFICATIONS
- SELECTIVE SITE DEMOLITION A. WORK SEQUENCE
- SURVEY
- A. EXPERIENCED LAND SURVEYOR QUALIFICATIONS AND SUPERVISION



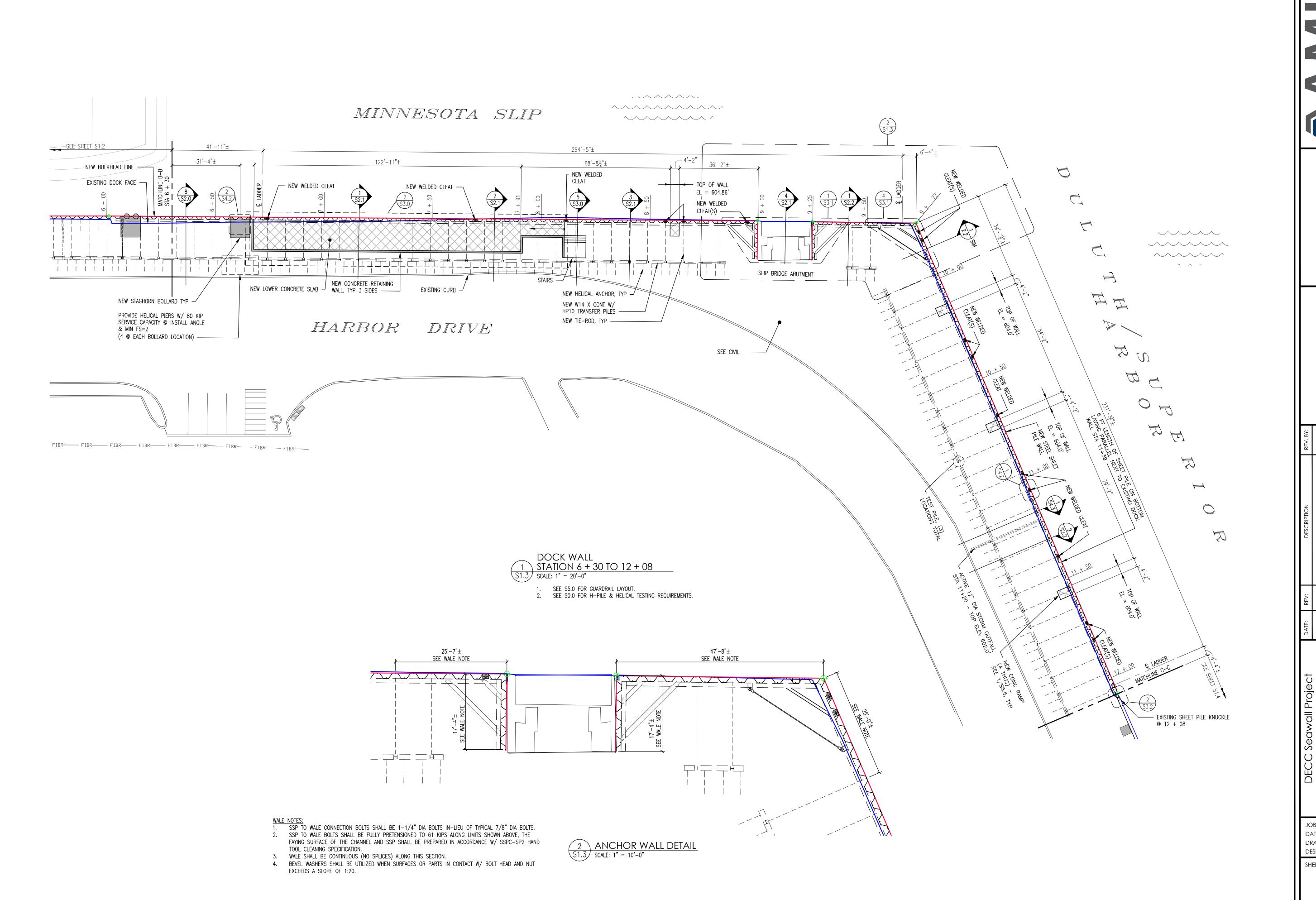
JOB No: 161080 DATE: 10-31-17 drawn by: PJB DESIGNED BY: CWS







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DESCRIPTION	Issued for Bid					
REV:	0					
DATE:	10-31-17					
DECC Seawall Project	City of Duluth	350 Harbor Drive	Duluth, Minnesota	Dock Site Plan	Station 6+30 to 12+08	

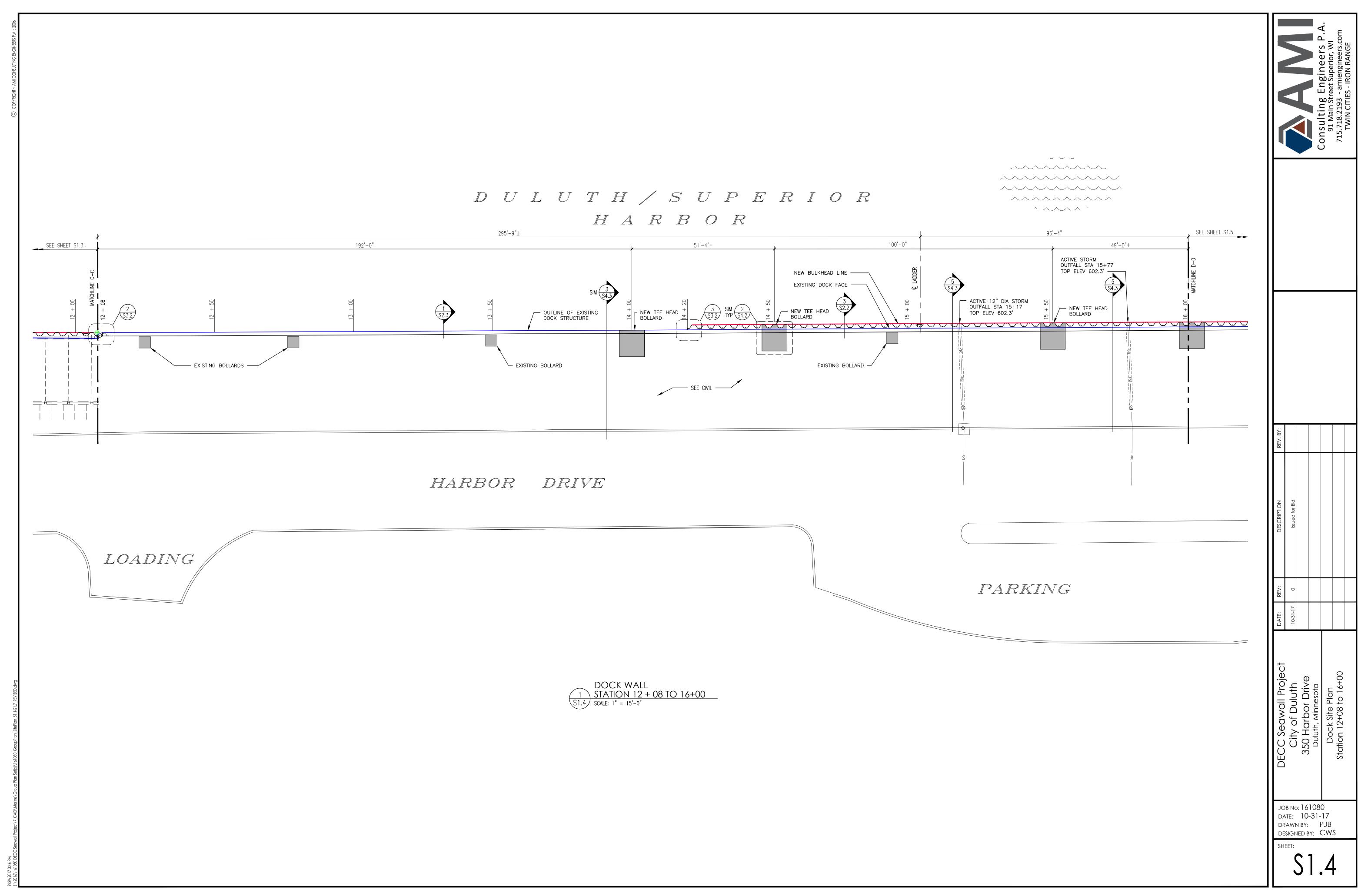
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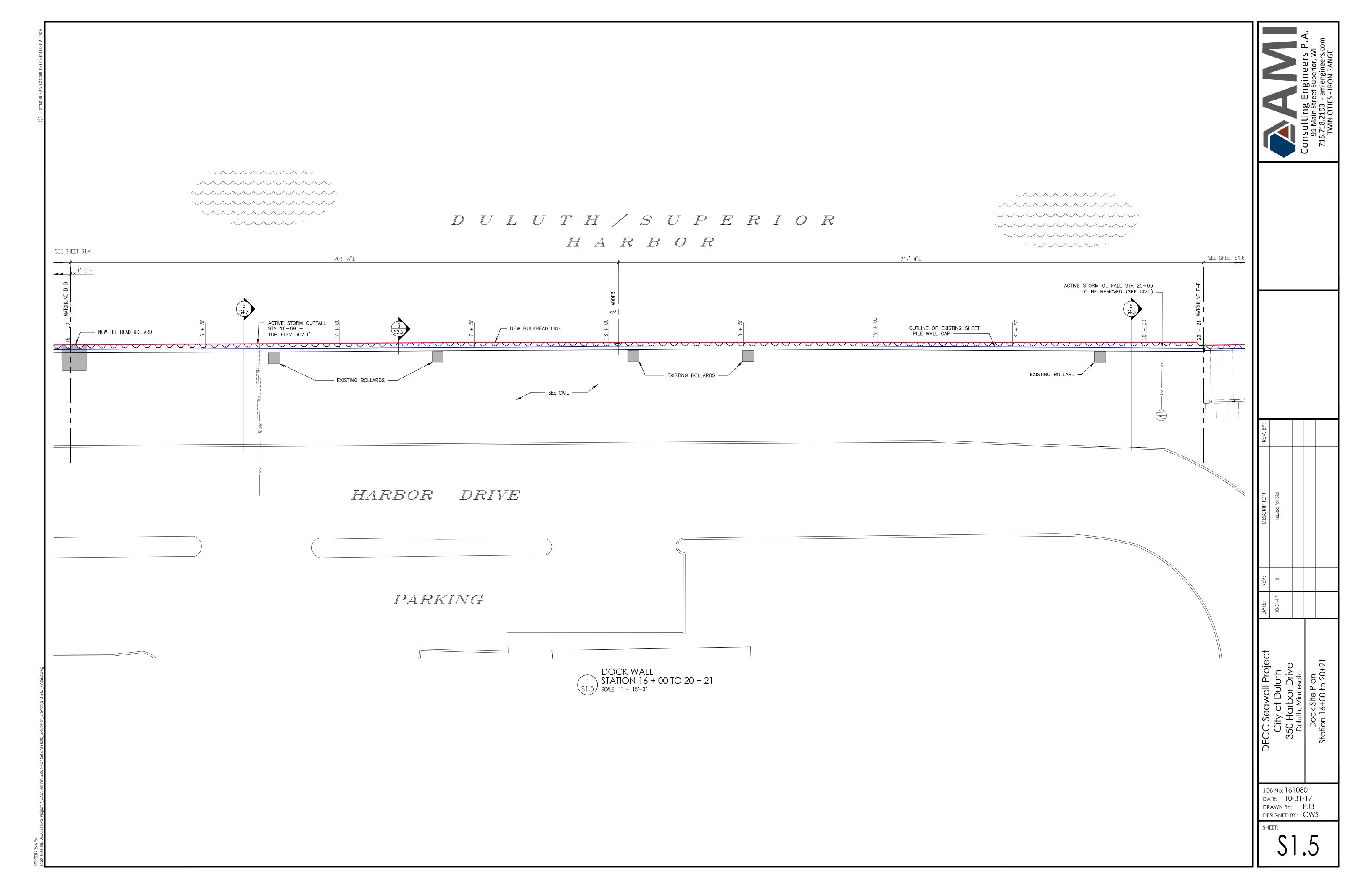
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\$1.3





DULUTH/SUPERIOR ______ HARBOR ______ ^^^^ SEE SHEET S1.5 SEE SHEET S1.7 204'-4"± 81'-5"± - 15" DIA STORM OUTFALL PIPE STA 20+54 TO BE REMOVED (SEE CIVIL) ROW OF ROUND PILES 21½"±
OUT FROM EXISTING DOCK WALL,
PILE HT VARIES FROM 2 TO 12 FT UP FROM MUDLINE __ EXISTING DOCK FACE MEW BULKHEAD LINE — EXISTING ROCK & GRASS MIX — ——— SEE CIVIL FOR NEW OUTLINE OF AQUARIUM BLDG — DOCK WALL 1 STATION 20 + 21 TO 22 + 25 S1.6 SCALE: 1" = 10'-0"

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DAIE:	10-31-17					
DECC Seawall Project	City of Duluth	350 Harbor Drive	Duluth, Minnesota	Dock Site Plan	Station 20+21 to 22+25	

JOB NO: 161080

DATE: 10-31-17

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\$1.6

DULUTH / SUPERIOR ^ ^ HARBOR SEE SHEET S1.6 119'-6"± ROW OF EXISTING SHEET PILE TO BE REMOVED,
 TOTAL LENGTH NOT VERIFIED. EXISTING W BEAM ON BACKSIDE OF SHEET PILE.
 TOTAL LENGTH NOT VERIFIED, TO BE REMOVED — EXISTING BOLLARD TYP EXISTING 12" DIA STEEL PILE TO BE REMOVED EXISTING WOODEN DECK EXISTING CONCRETE PATH - EXISTING GRASS AREA ----- EXISTING OUTFALL PIPE EXISTING ASPHALT PATH OUTLINE OF AQUARIUM DOCK WALL PLAN

STATION 22 + 25 TO 24 + 03

S1.7 SCALE: 1" = 10'-0"

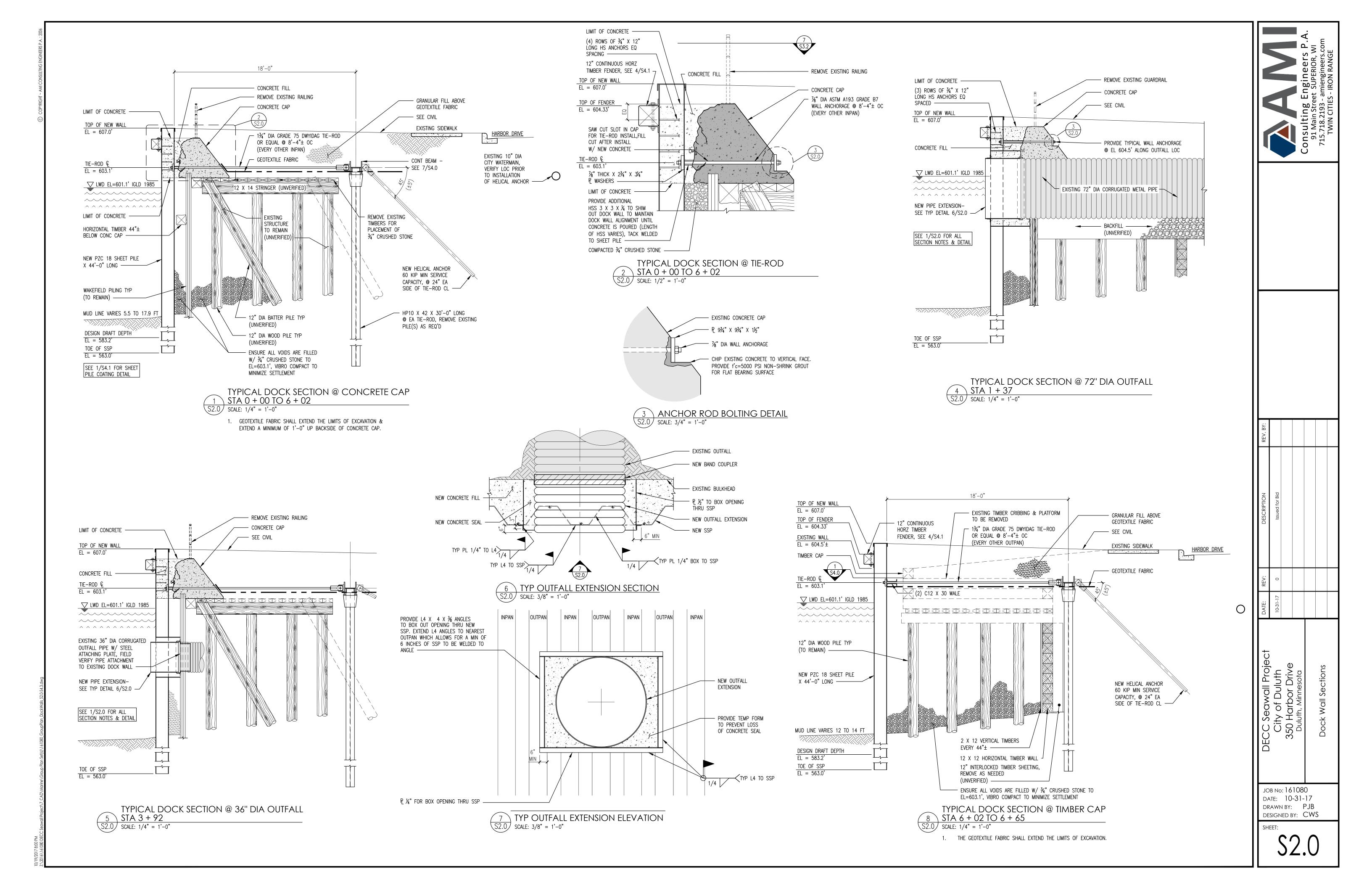
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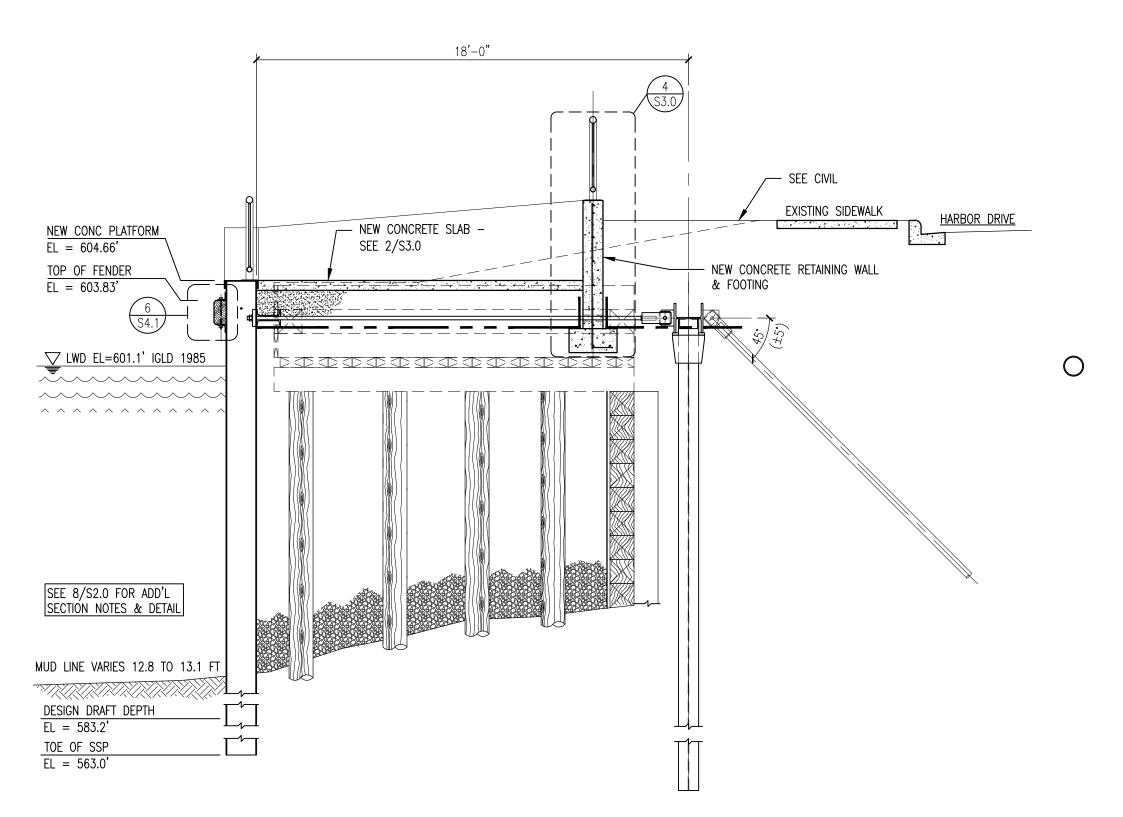
DECC Seawall Project
City of Duluth
350 Harbor Drive
Duluth, Minnesota

Dock Site Plan
Station 22+25 to 24+03

JOB NO: 161080
DATE: 10-31-17
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\$1.7





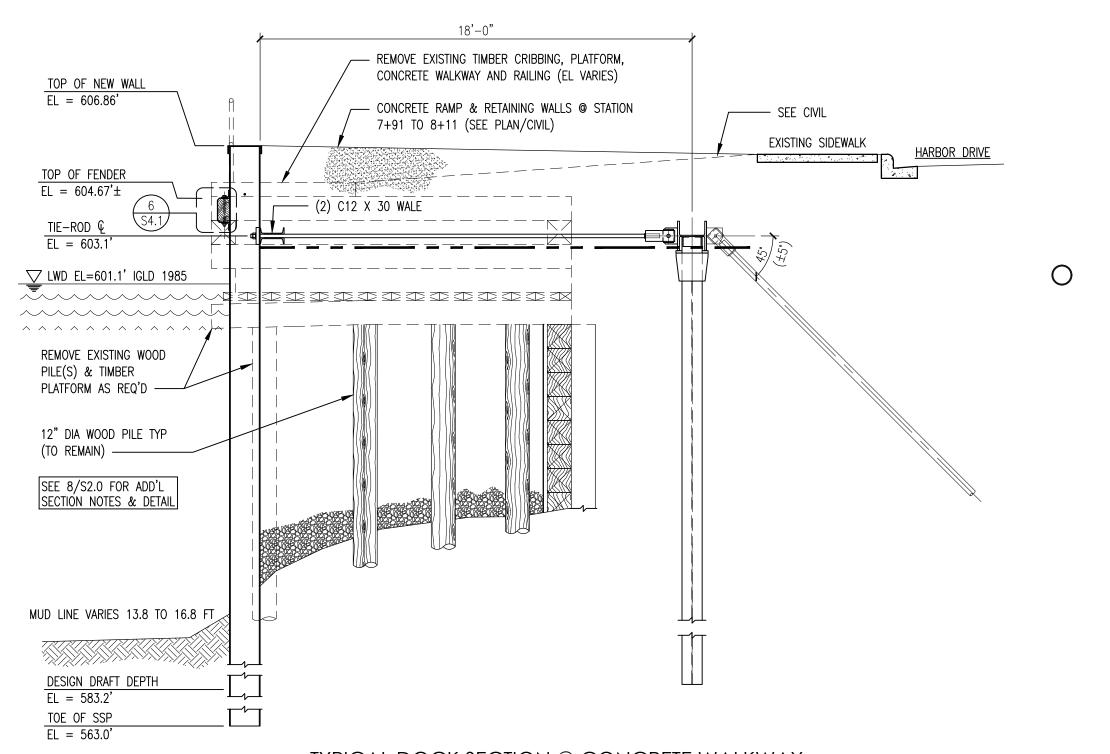
TYPICAL DOCK SECTION @ LOWER PLATFORM

STA 6 + 65 TO 7 + 30±

S2.1) SCALE: 1/4" = 1'-0"

- EXTEND GEOTEXTILE FABRIC MINIMUM 1'-0" UP EACH SIDE OF NEW CONCRETE RETAINING WALL.
- RETAINING WALL.

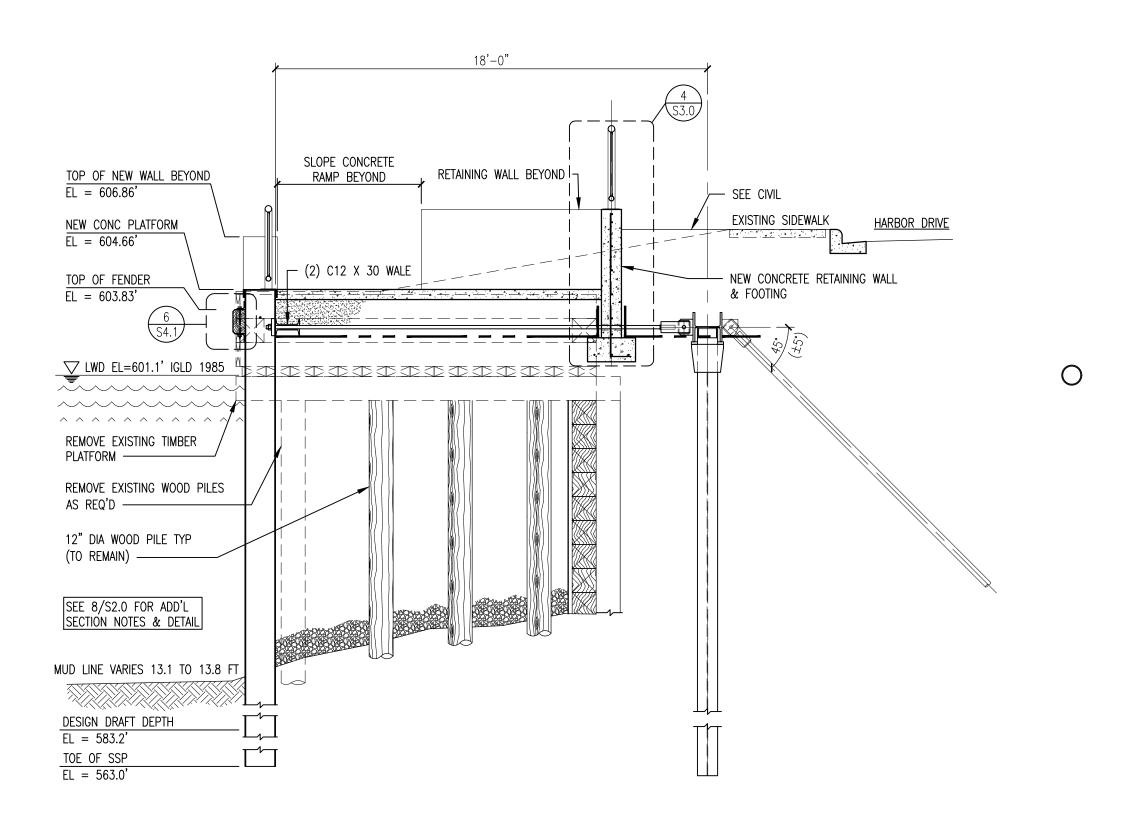
 2. GEOTEXTILE FABRIC SHALL EXTEND LIMITS OF EXCAVATION.



TYPICAL DOCK SECTION @ CONCRETE WALKWAY

@ STA 7 + 91 TO 9 + 00

S2.1 SCALE: 1/4" = 1'-0"

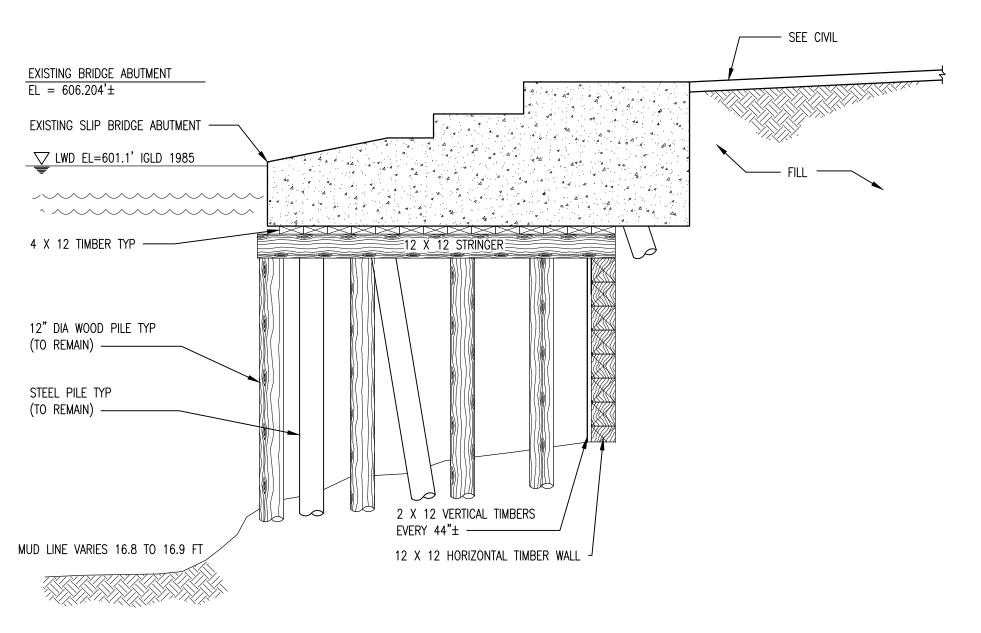


TYPICAL DOCK SECTION @ LOWER PLATFORM

STA 7 + 30± TO 7 + 91

S2.1) SCALE: 1/4" = 1'-0"

- EXTEND GEOTEXTILE FABRIC MINIMUM 1'-0" UP EACH SIDE OF NEW CONCRETE RETAINING WALL.
- 2. GEOTEXTILE FABRIC SHALL EXTEND LIMITS OF EXCAVATION.



SECTION @ SLIP BRIDGE ABUTMENT @ STA 9 + 00 TO 9 + 25 S2.1 SCALE: 1/4" = 1'-0"

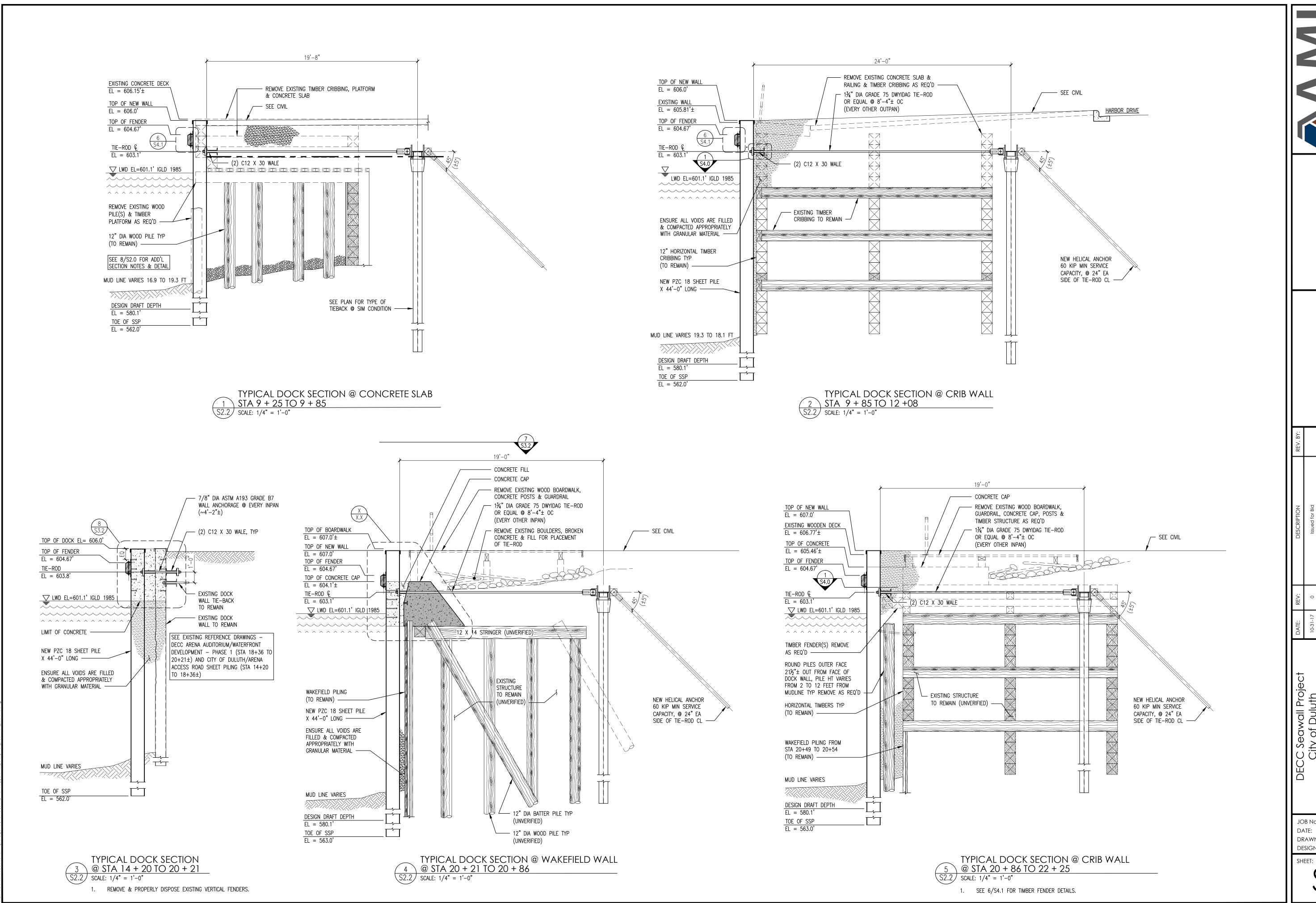
SEE EXISTING REFERENCE DRAWINGS —
CITY OF DULUTH / MINNESOTA SLIP PEDESTRIAN BRIDGE

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\$2.1

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DECC Seawall Project
City of Duluth
350 Harbor Drive
Duluth, Minnesota

Dock Wall Sections

DECC Seawall Project
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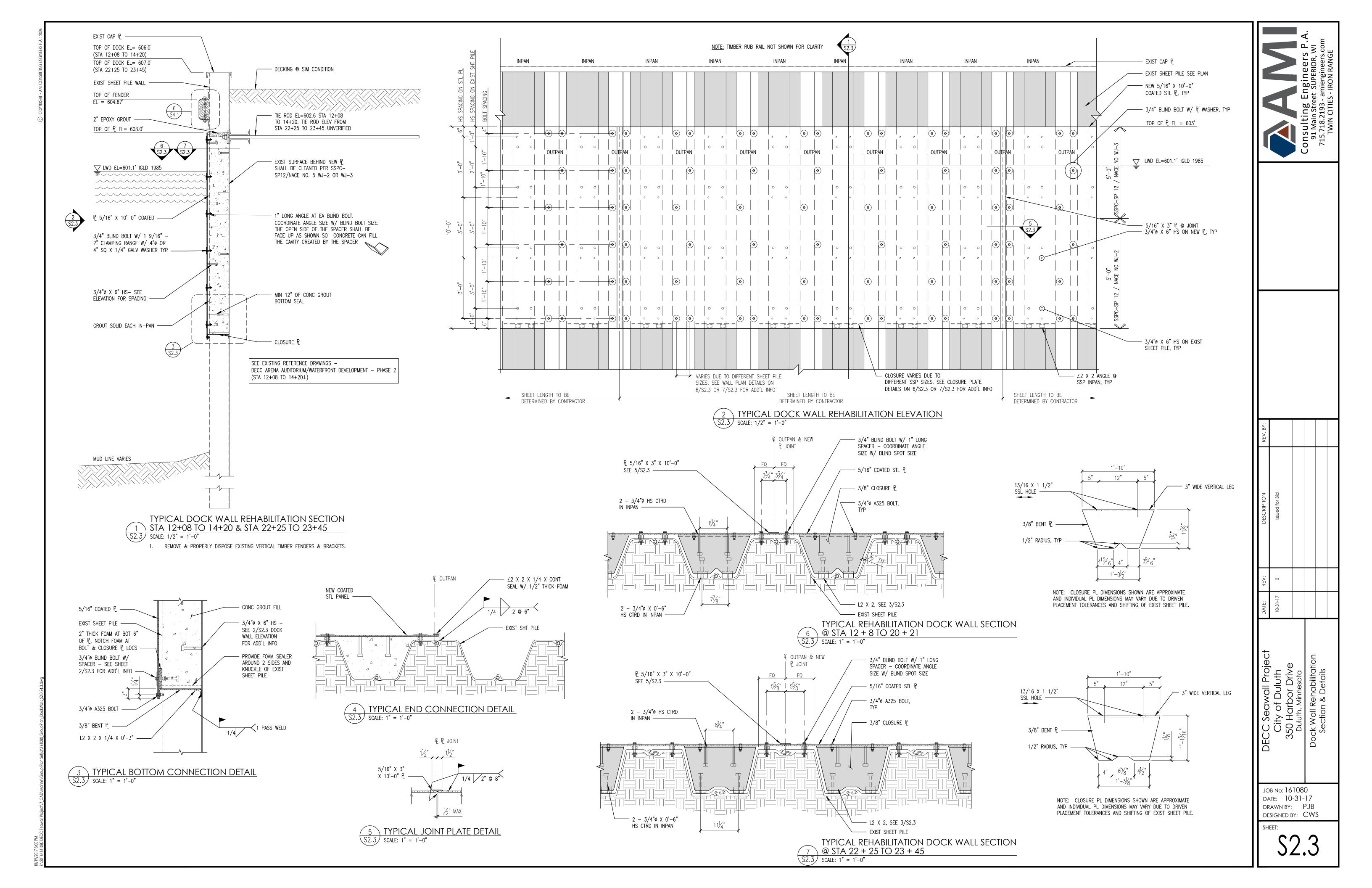
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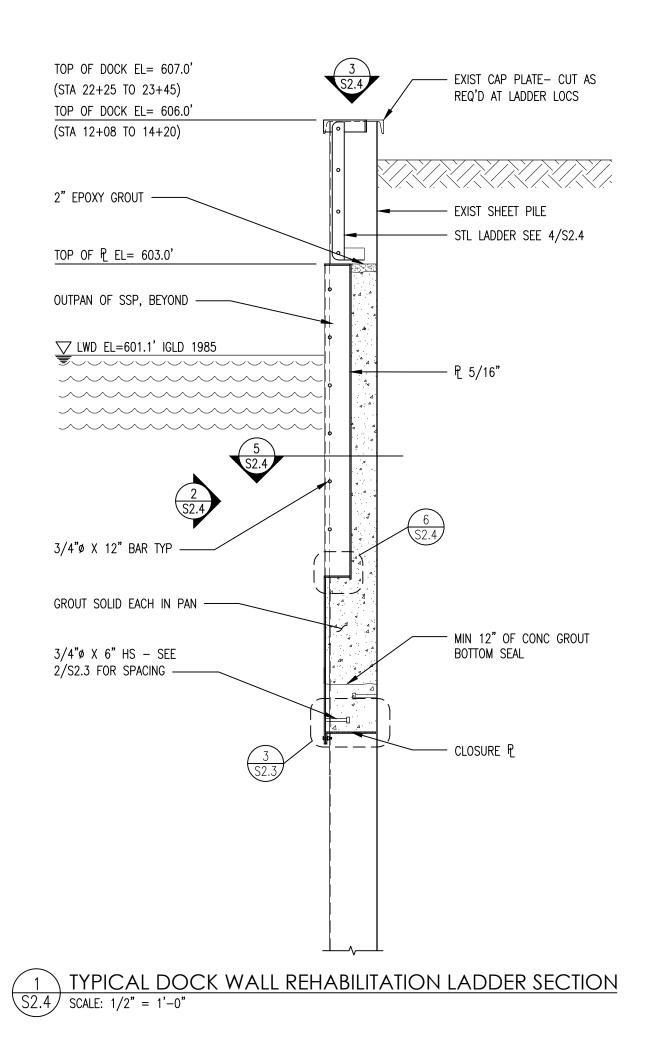
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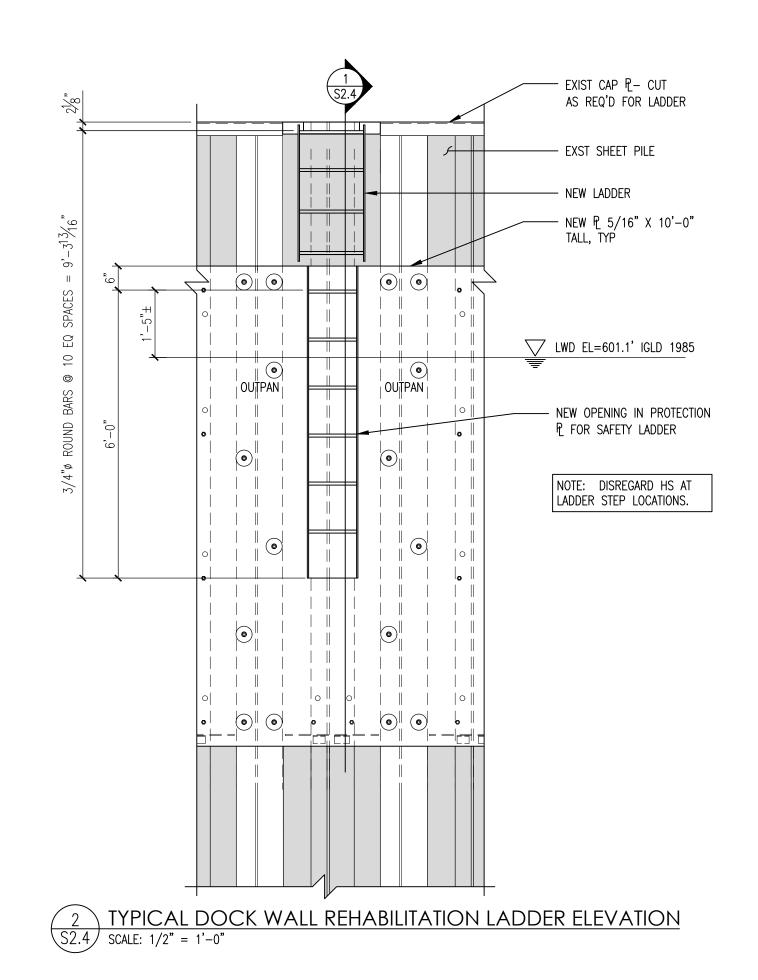
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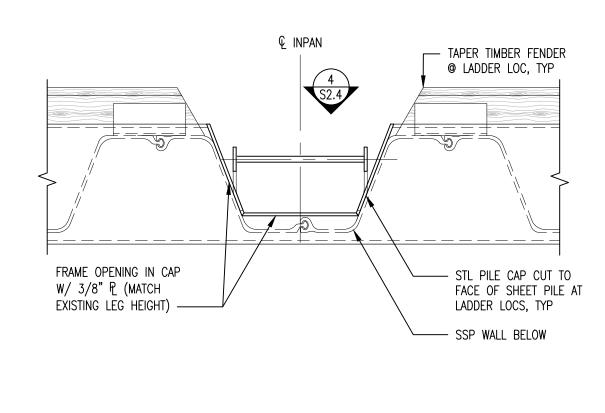
DESIGNED BY: CWS

\$2.2

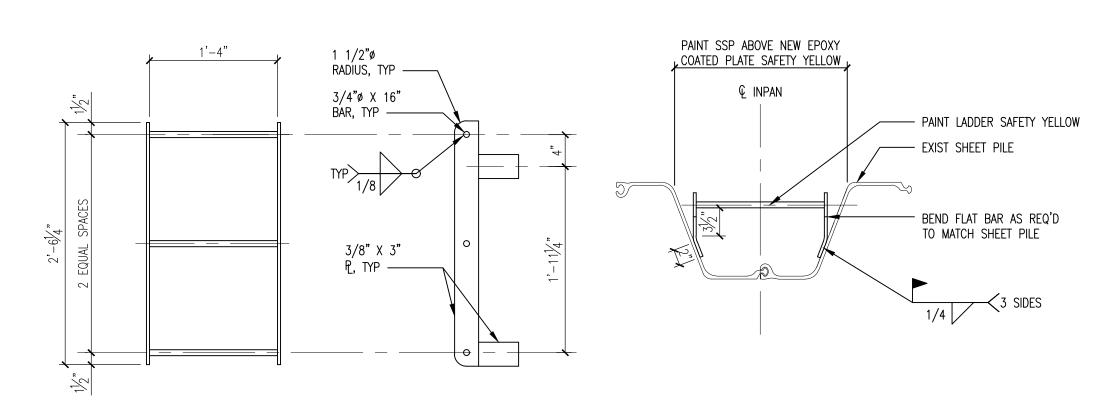




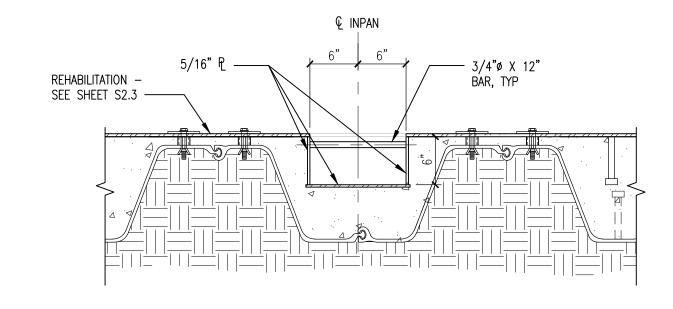




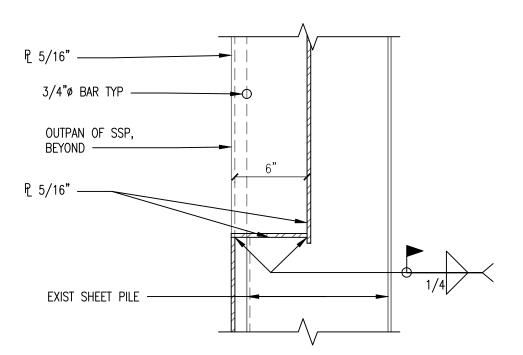
TYPICAL EXISTING DOCK WALL LADDER DETAIL S2.4 SCALE: 1" = 1'-0"



4 TYPICAL EXISTING DOCK WALL LADDER DETAIL SCALE: 1" = 1'-0"



5 TYPICAL REHABILITATION DOCK WALL LADDER DETAIL
S2.4 SCALE: 1" = 1'-0"



6 TYPICAL BOTTOM CONNECTION DETAIL S2.4 SCALE: 1-1/2" = 1'-0"

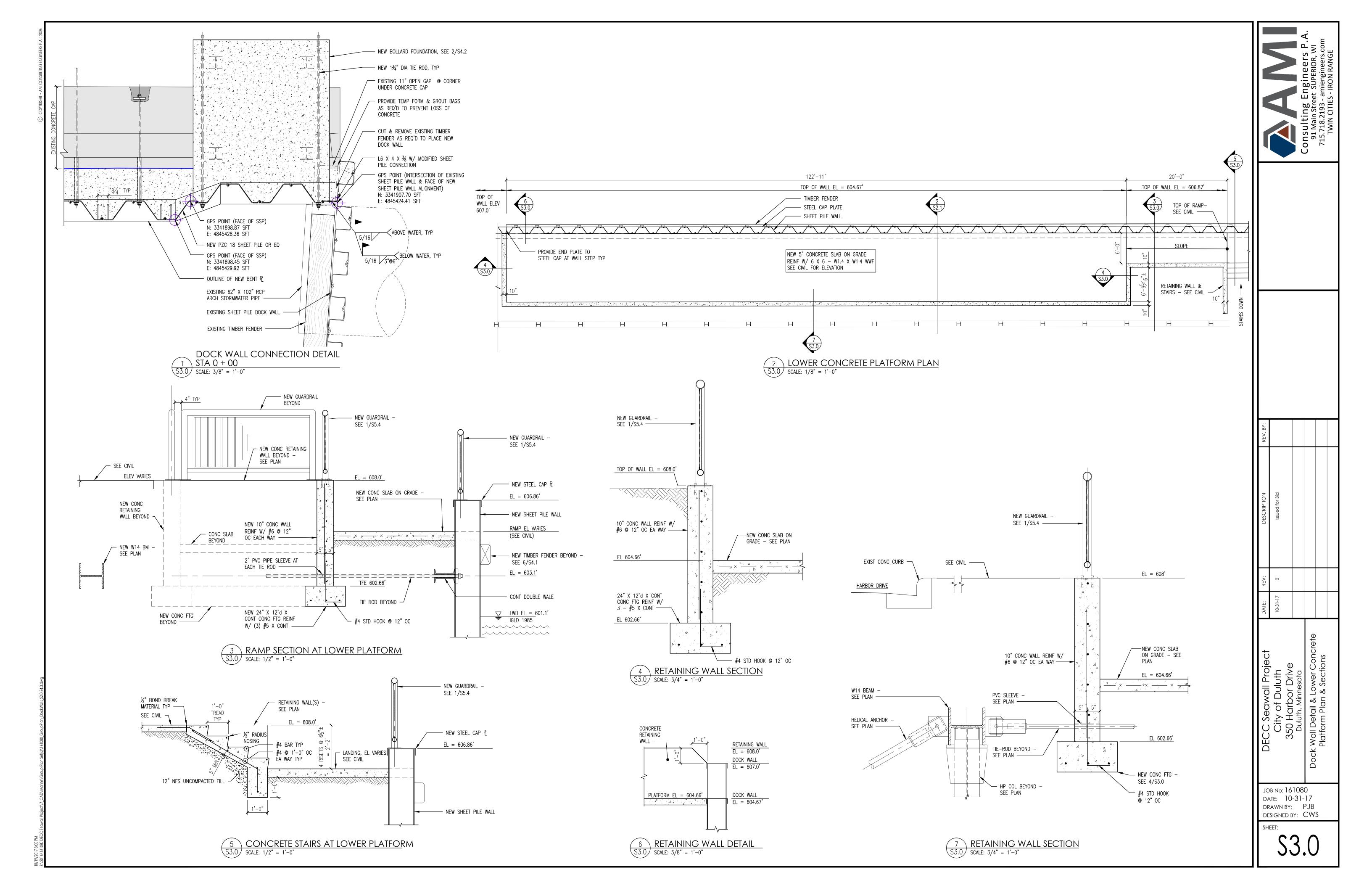
JOB NO: 161080

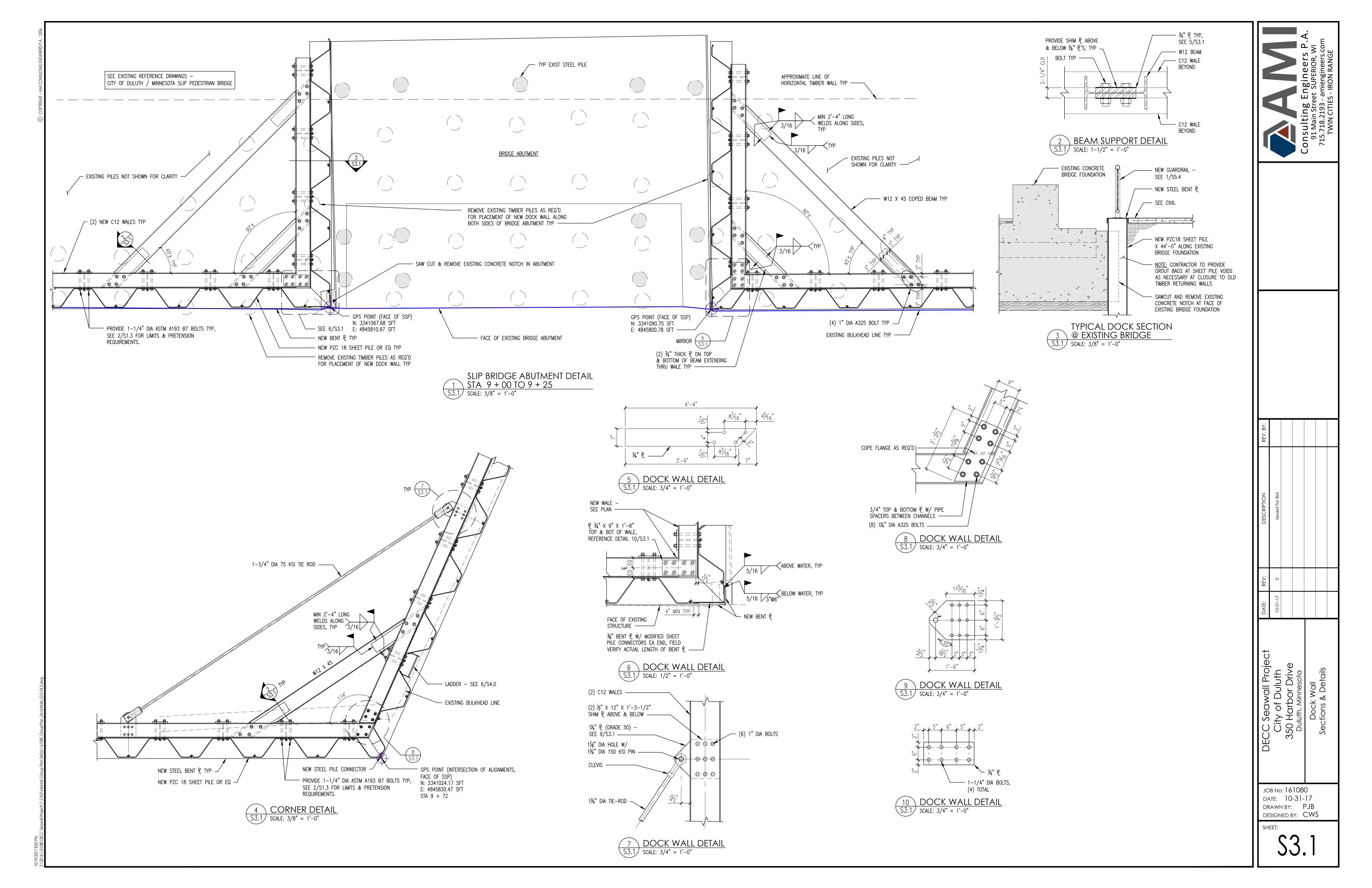
DATE: 10-31-17

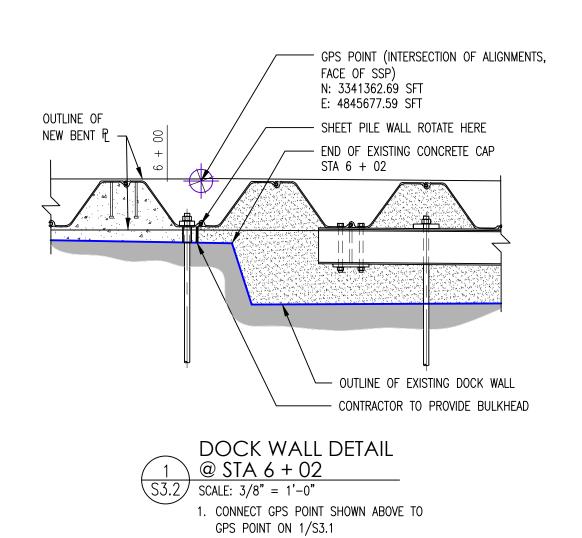
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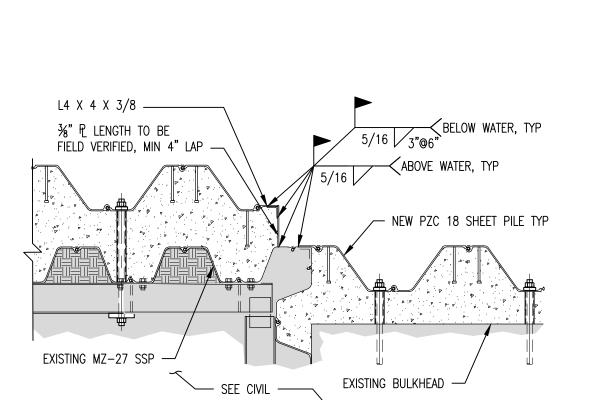
DESIGNED BY: CWS

\$2.4





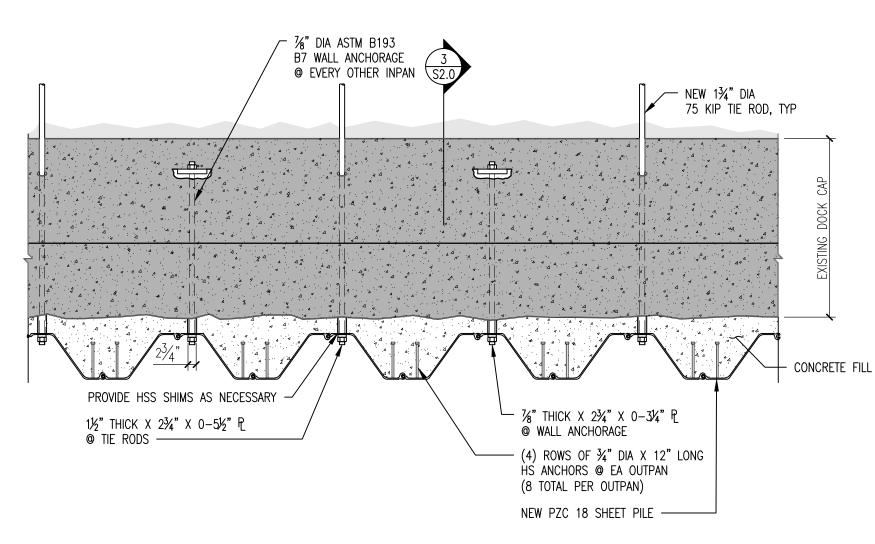




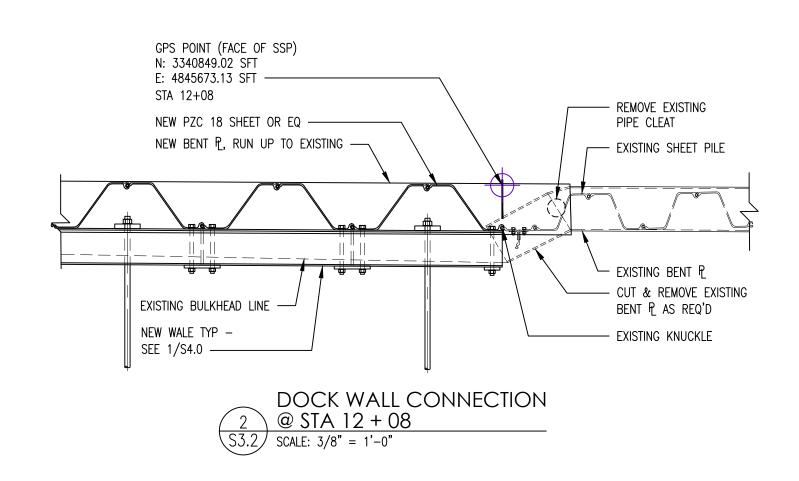
DOCK WALL CONNECTION

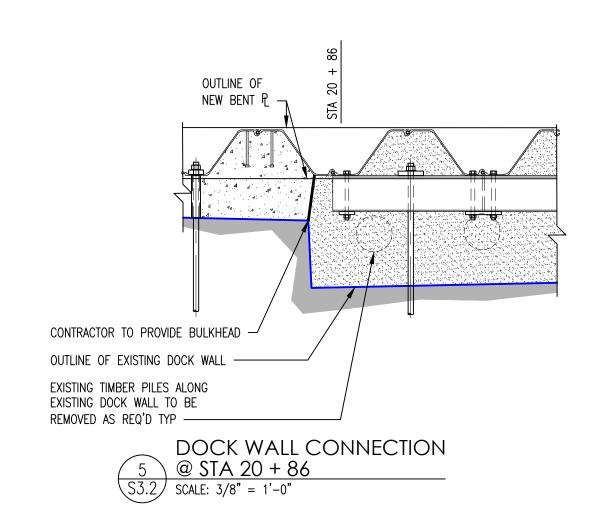
4 @ STA 20 + 21

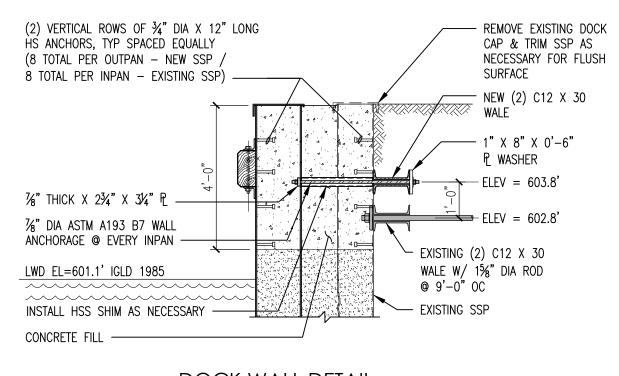
S3.2 SCALE: 3/8" = 1'-0"



TYPICAL DOCK WALL PLAN @ STA 0 + 00 7 TO 6 + 02 & STA 10 + 21 TO 20 + 86 S3.2 SCALE: 3/8" = 1'-0"



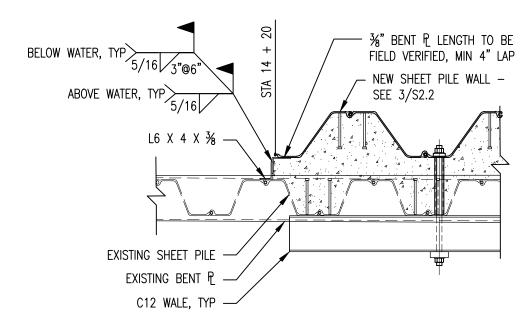




DOCK WALL DETAIL

8 @ STA 14 + 20 TO 20 + 21

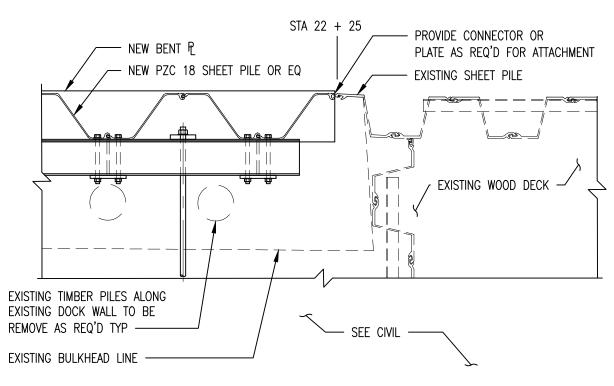
S3.2 SCALE: 3/8" = 1'-0"



DOCK WALL CONNECTION

© STA 14 + 20

S3.2) SCALE: 3/8" = 1'-0"



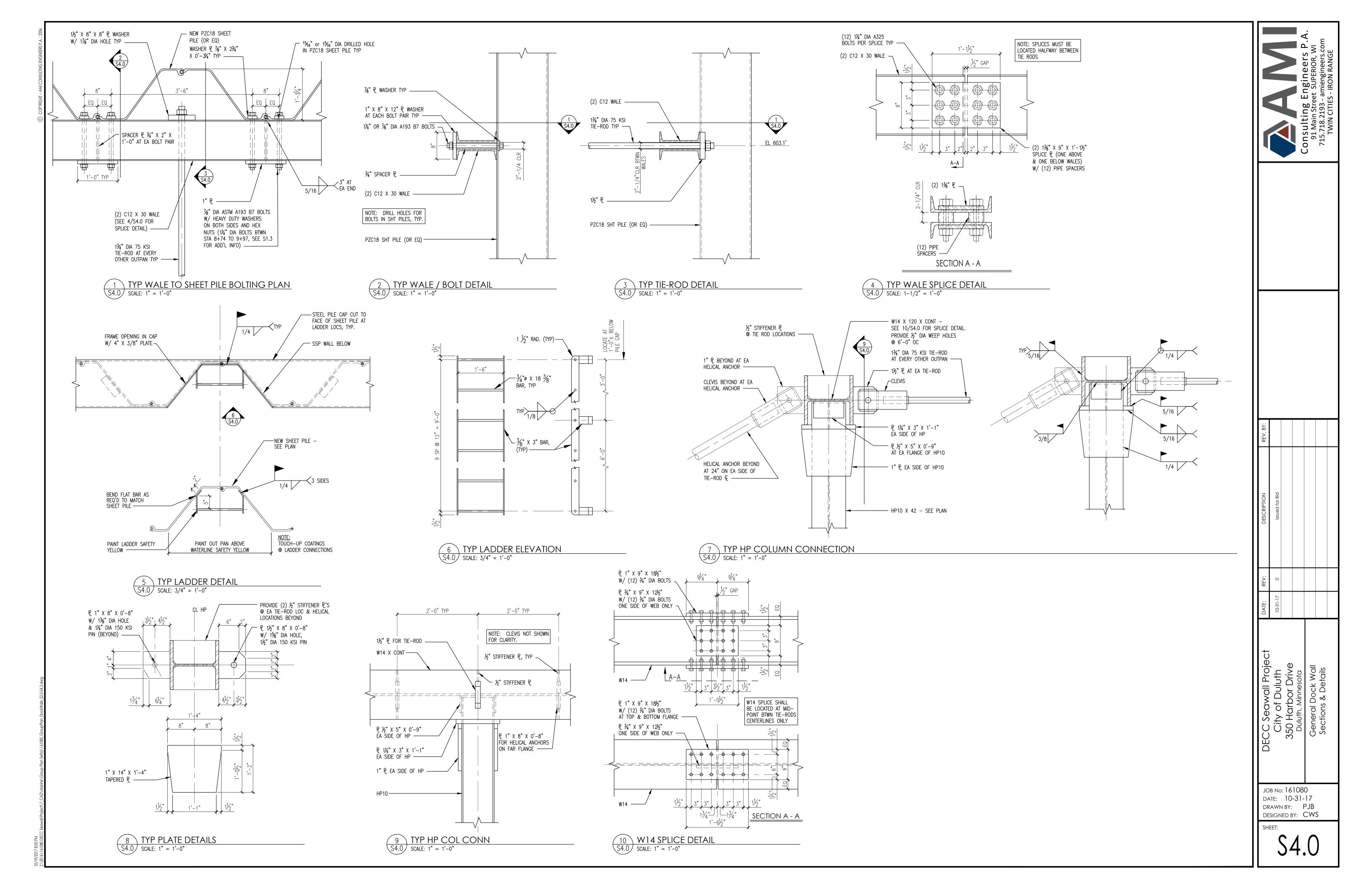
DOCK WALL CONNECTION

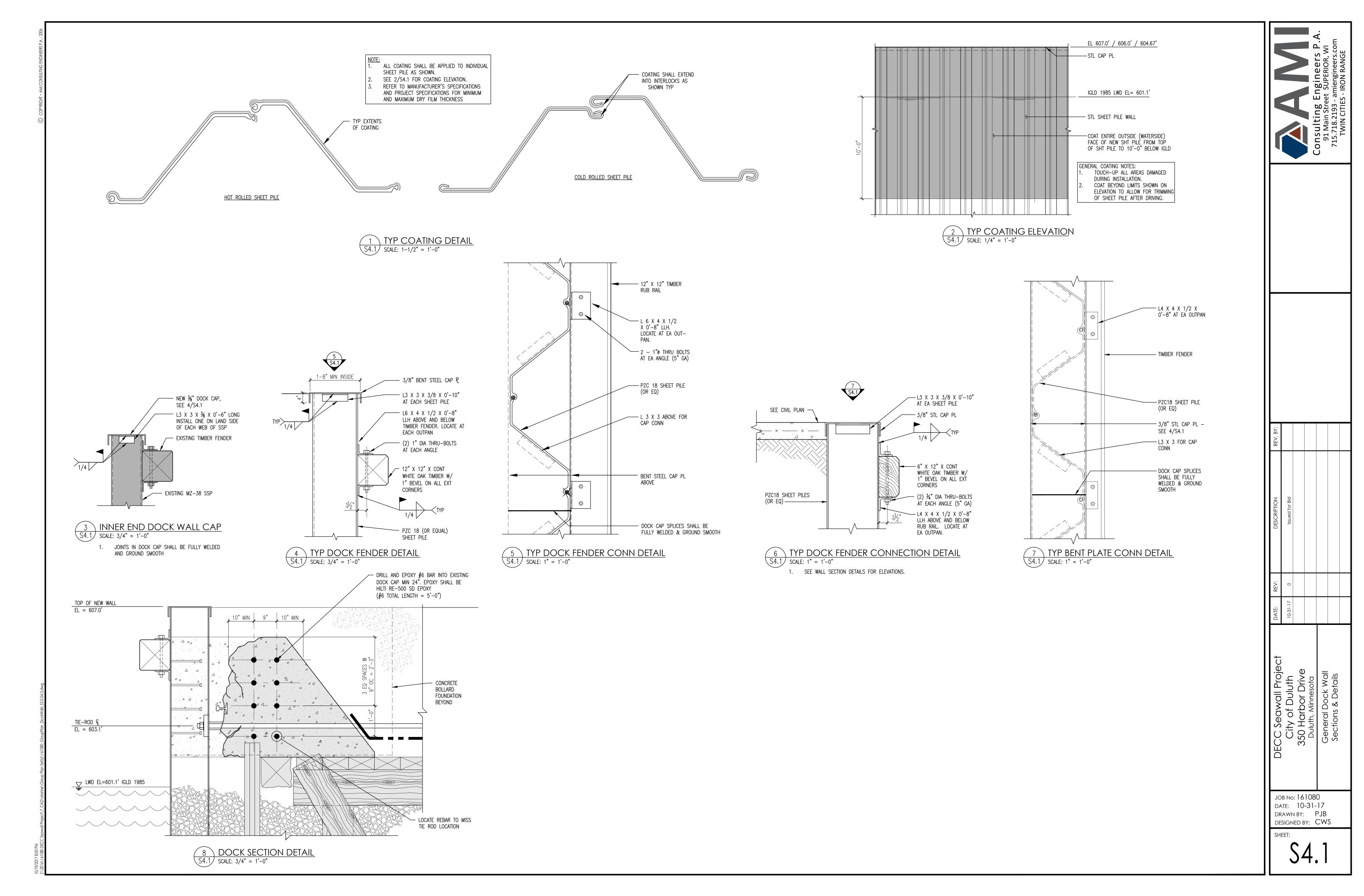
6 @ STA 22 + 25

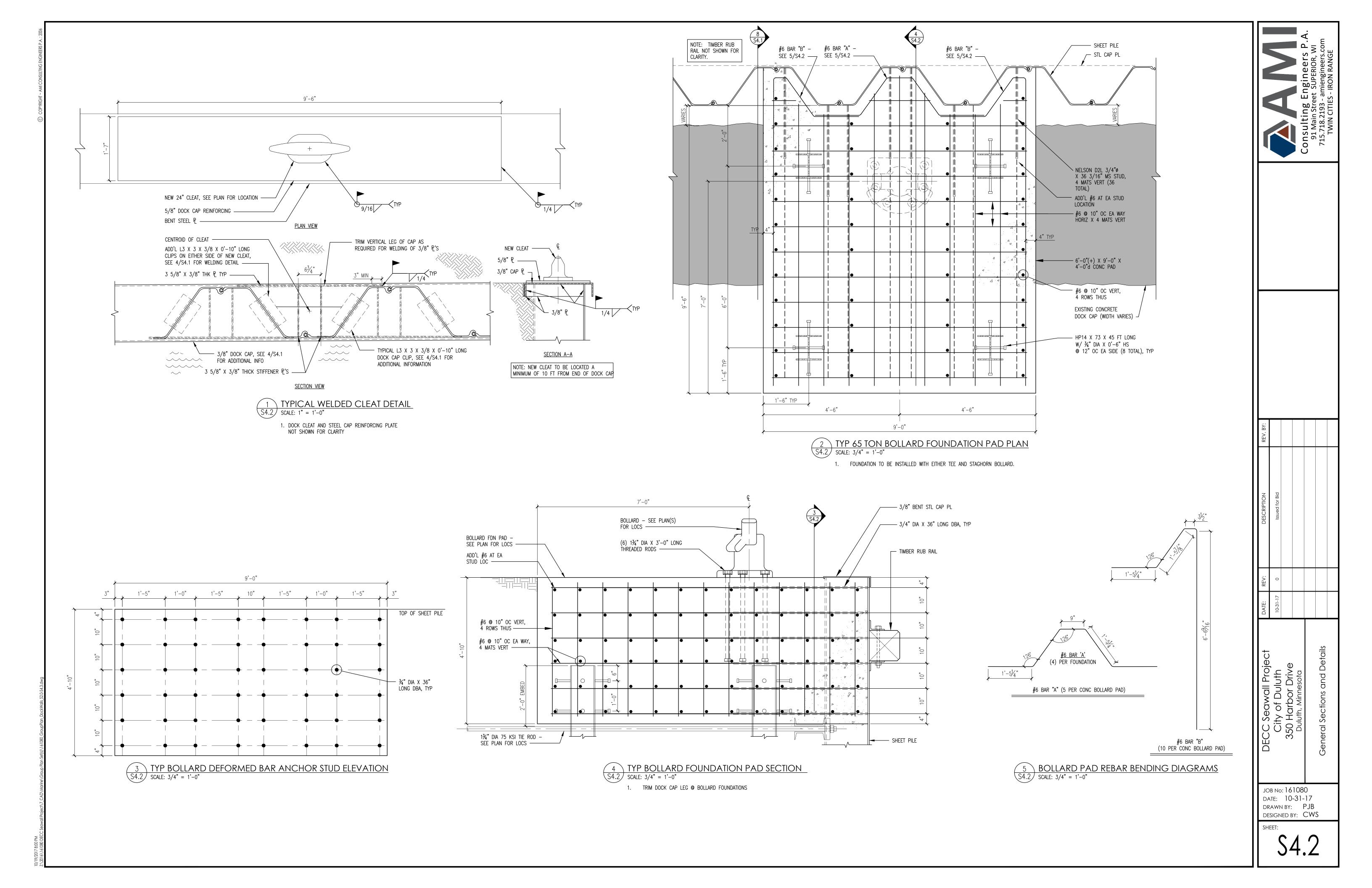
SCALE: 3/8" = 1'-0"

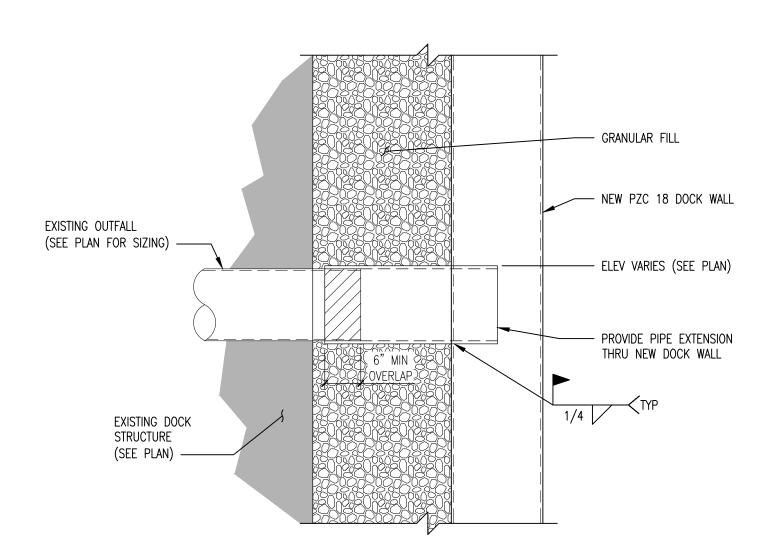
DECC Seawall Project City of Duluth 350 Harbor Drive Duluth, Minnesota JOB No: 161080 DATE: 10-31-17 DRAWN BY: PJB DESIGNED BY: CWS

\$3.2

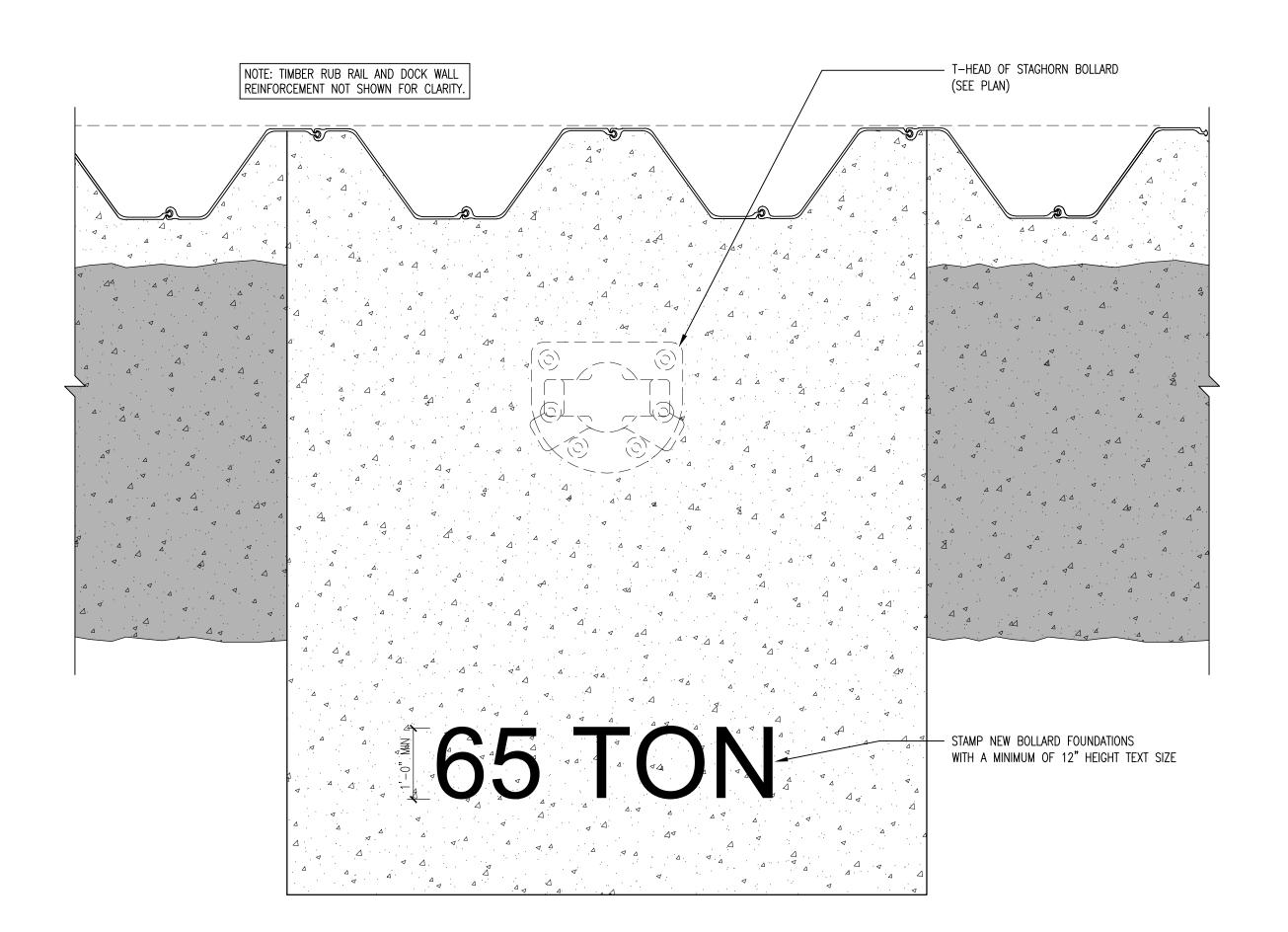








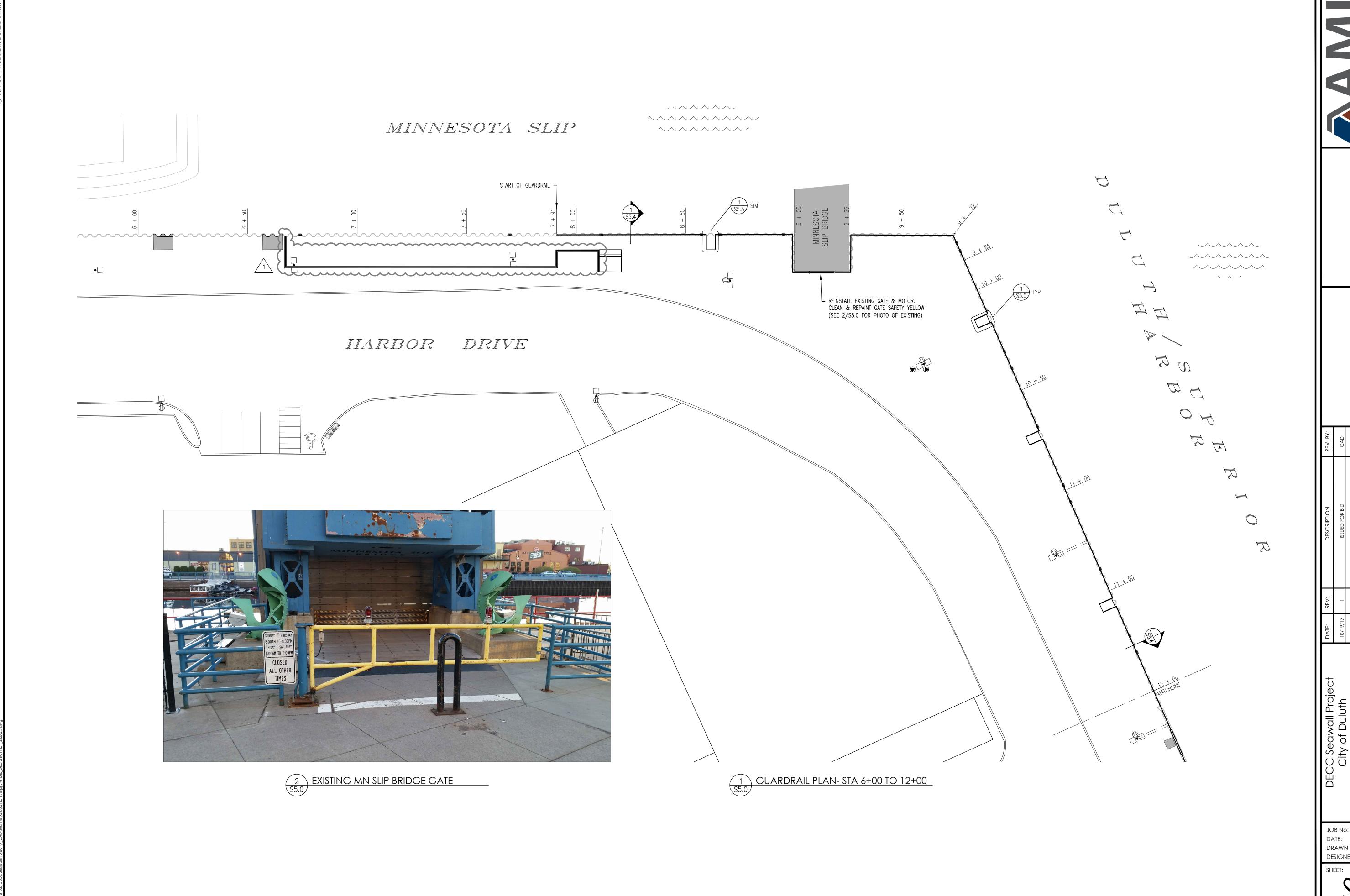




2 TYPICAL BOLLARD FOUNDATION STAMP S4.3 SCALE: 3/4" = 1'-0"

				CONSUITING ENGINEERS 91 Main Street SUPERIOR W	715.718.2193 - amiengineers.co	TWIN CITIES - IRON RANGE
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DESCRIPTION	Issued for Bid					
REV:	0					
DATE:	10-31-17					
DECC Seawall Project	City of Duluth	350 Harbor Drive	Duluth, Minnesota		General Sections and Details	
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\$4.3



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CAD	CAD				
ISSUED FOR BID	General Revisions				
ı	1				
10/19/17	10-31-17				
City of Duluth	350 Harbor Drive	Duluth, Minnesota	Guardrail Plan	Station 6+00 to 12+00	

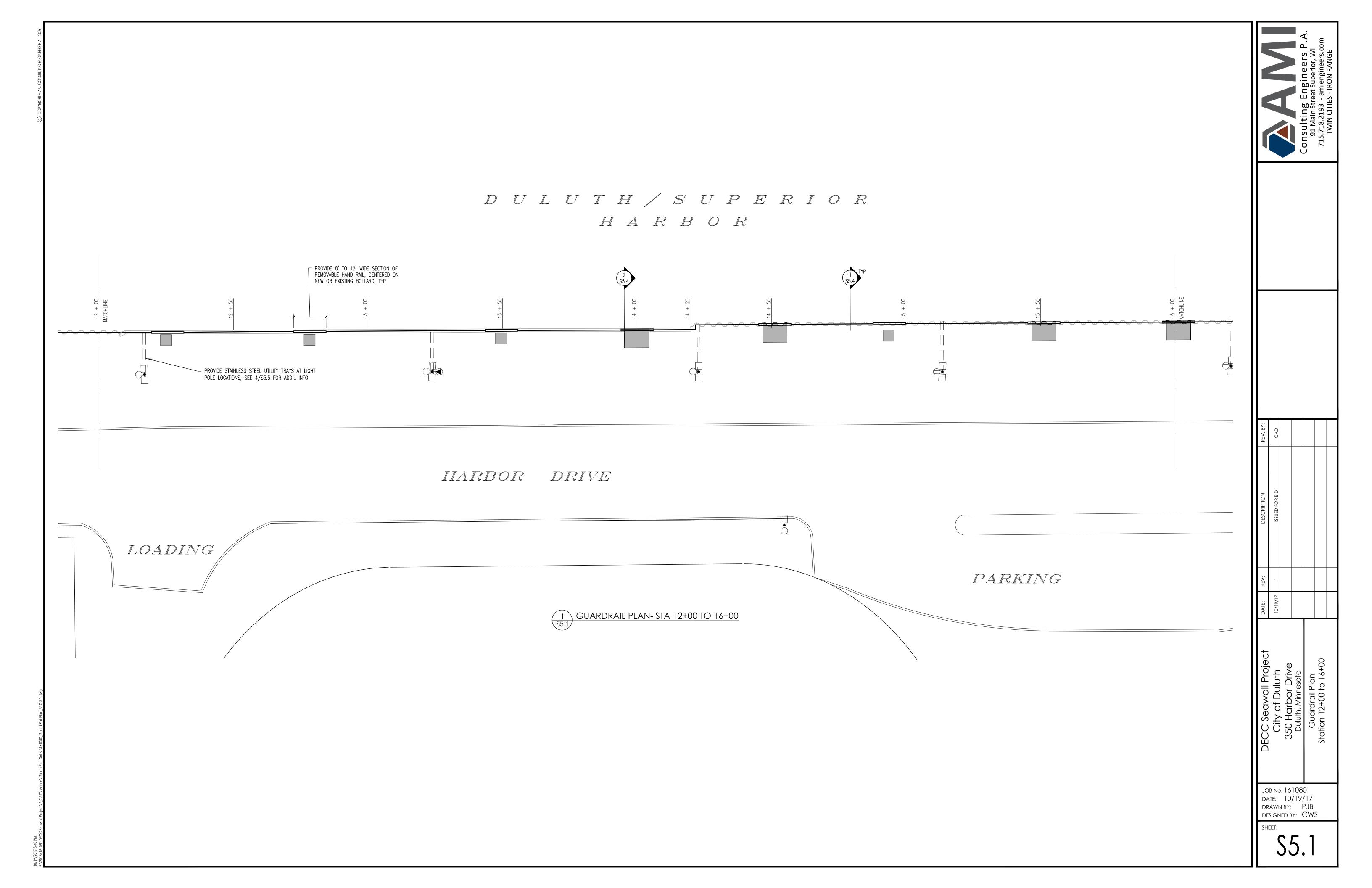
JOB NO: 161080

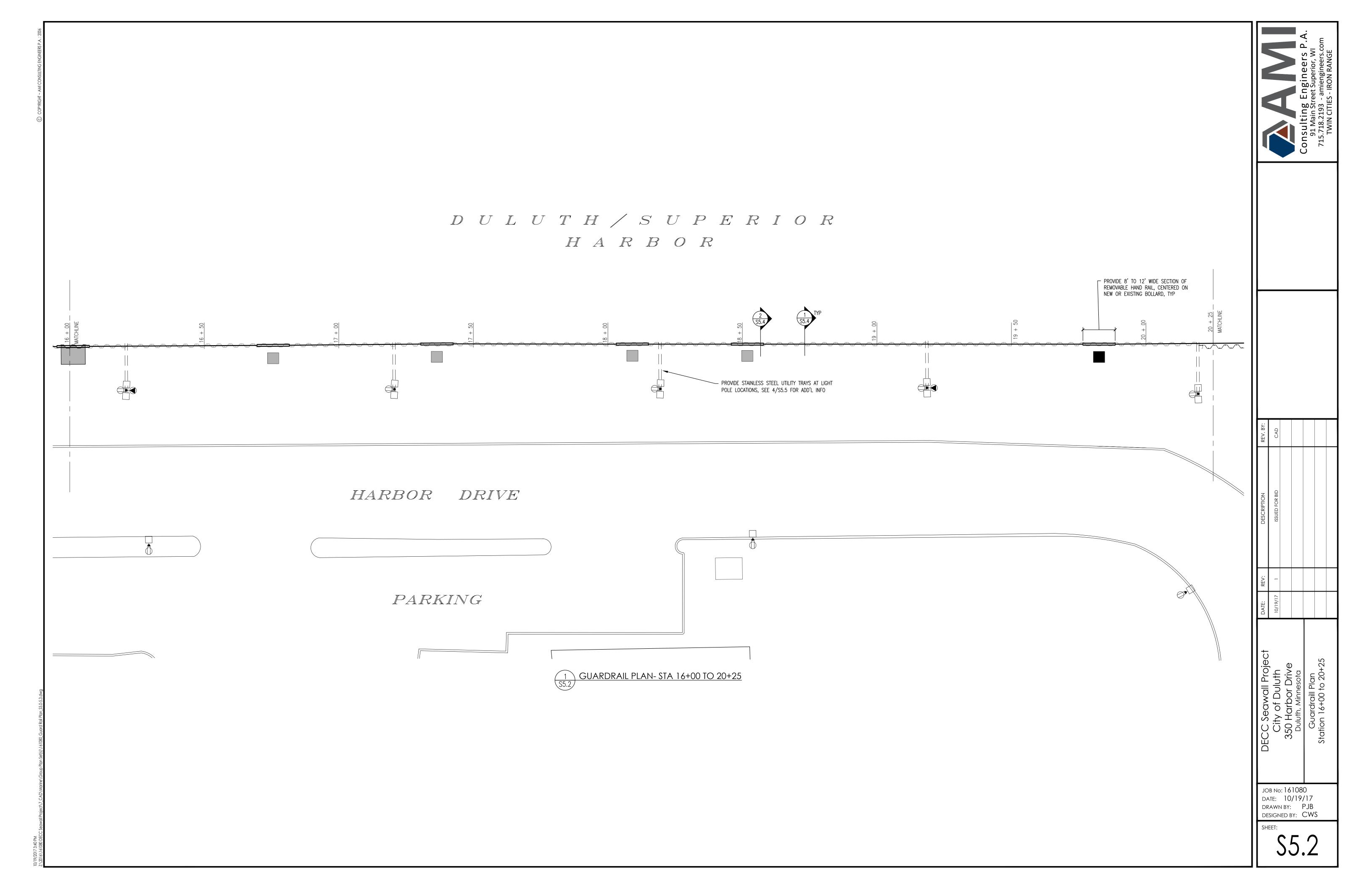
DATE: 10/19/17

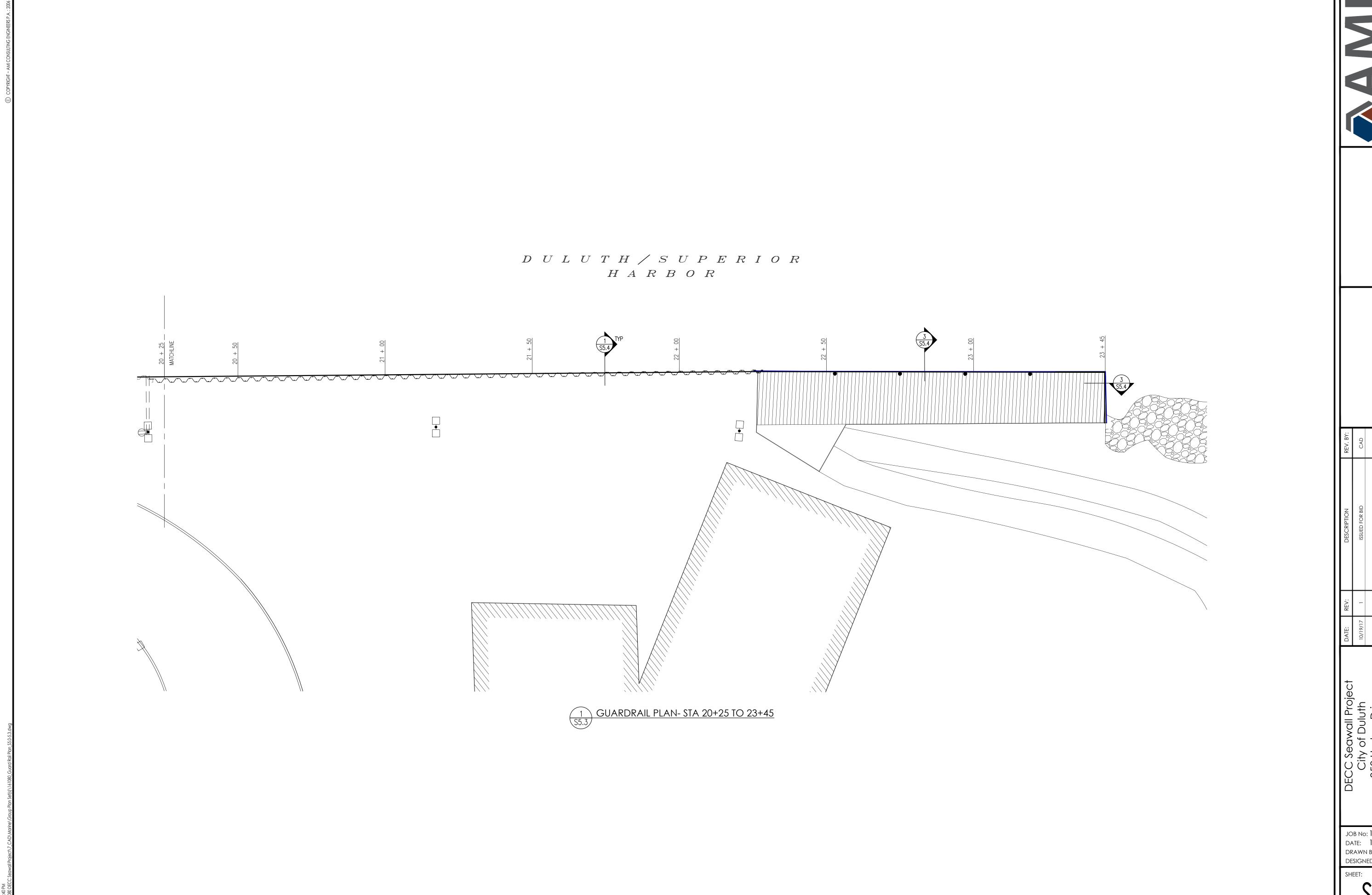
DRAWN BY: PJB

DESIGNED BY: CWS

\$5.0







Consulting Engineers P.A. 91 Main Street Superior, WI 715.718.2193 - amiengineers.com TWIN CITIES - IRON RANGE

 CAD					
ISSUED FOR BID					
 1					
10/19/17					
City of Duluth	350 Harbor Drive	Duluth, Minnesota	Guardrail Plan	Station 20+25 to 23+45	

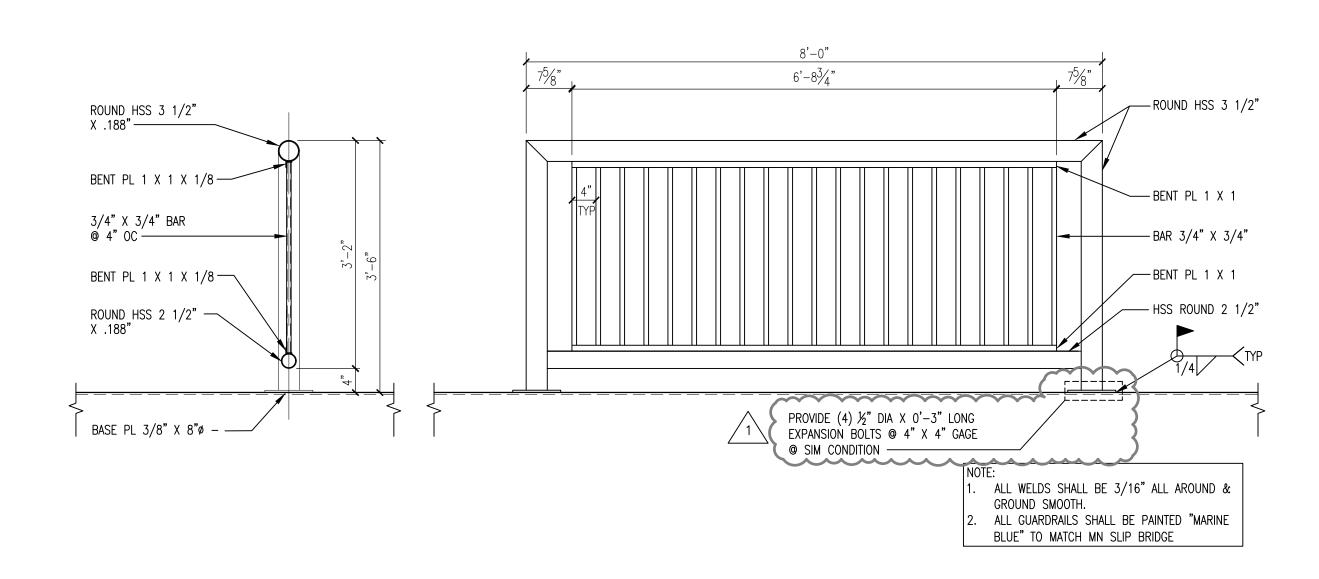
JOB NO: 161080

DATE: 10/19/17

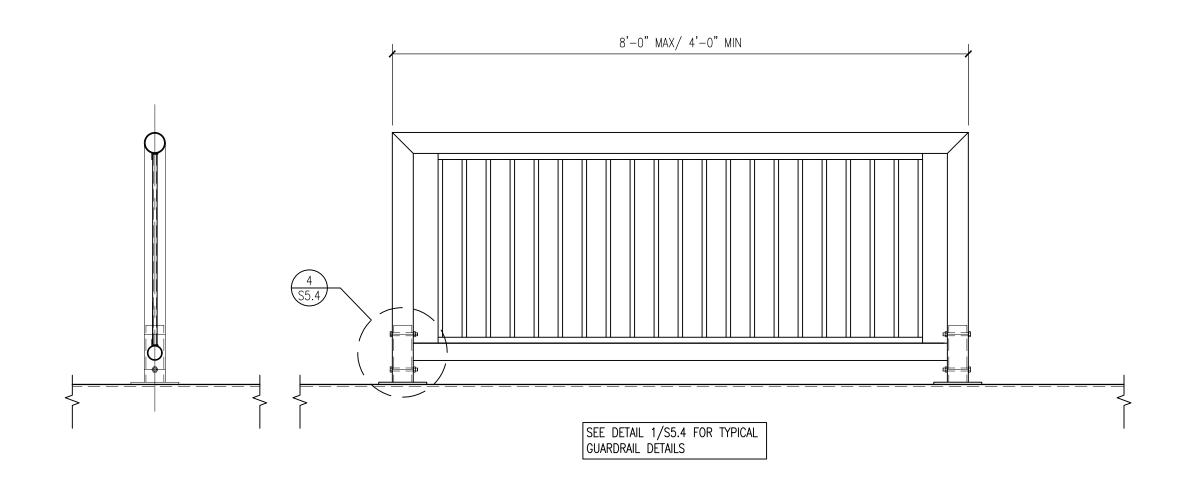
DRAWN BY: PJB

DESIGNED BY: CWS

SHEET: **C.5** '



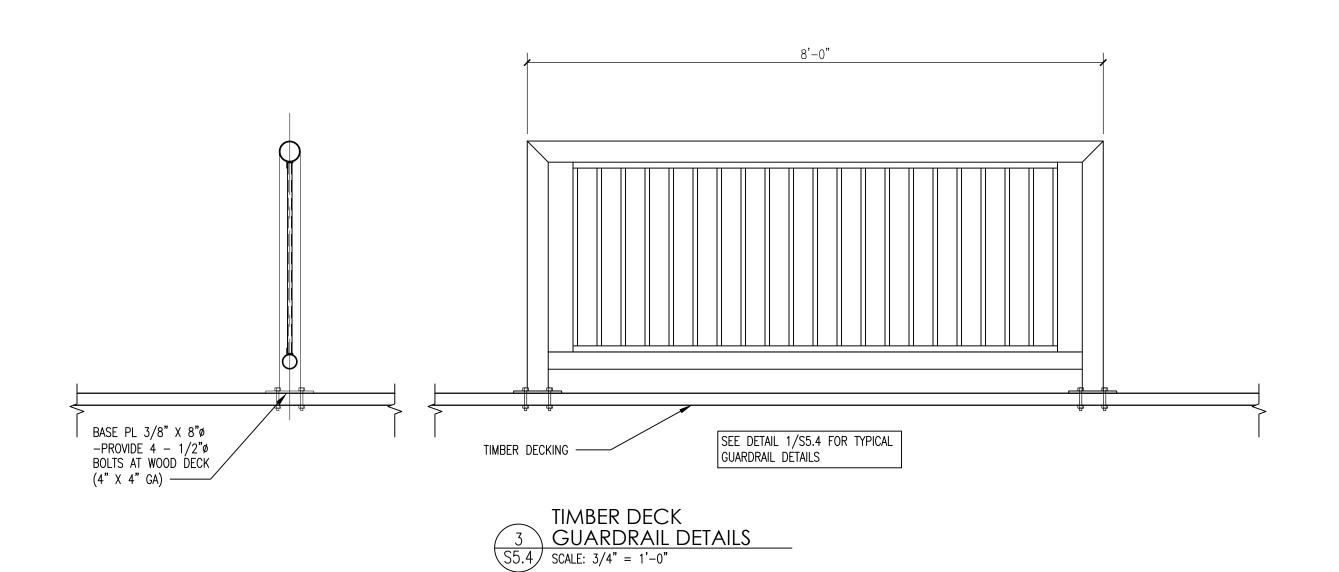
1 TYP GUARDRAIL DETAILS
S5.4 SCALE: 3/4" = 1'-0"

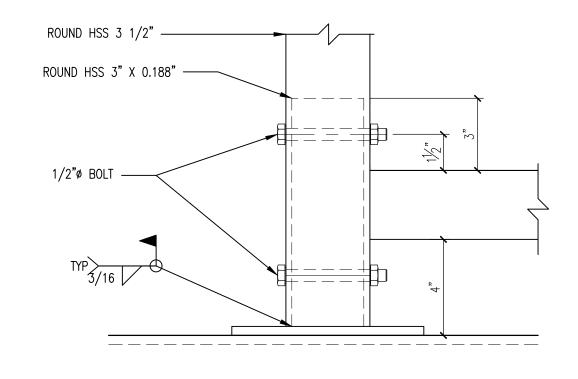


REMOVABLE

2 GUARDRAIL DETAILS

S5.4 SCALE: 3/4" = 1'-0"





REMOVABLE

GUARDRAIL DETAIL

S5.4 SCALE: 3" = 1'-0"

DECC Seawall Project City of Duluth 350 Harbor Drive Duluth, Minnesota General Details General Details PATE: REV: DESCRIPTION REV. BY: PUN19/17 1 ISSUED FOR BID CAD CAD CAD CAD CAD CAD CAD CAD CAD CA						
DATE: REV: 10/19/17 1 10-31-17 1	REV. BY:	CAD	CAD			
DATE: 10/19/17 10-31-17	DESCRIPTION	ISSUED FOR BID	General Revisions			
	REV:	1	1			
DECC Seawall Project City of Duluth 350 Harbor Drive Duluth, Minnesota General Details	DATE:	10/19/17	10-31-17			
1 I	DECC Seawall Project	City of Duluth	350 Harbor Drive	Duluth, Minnesota	General Details	

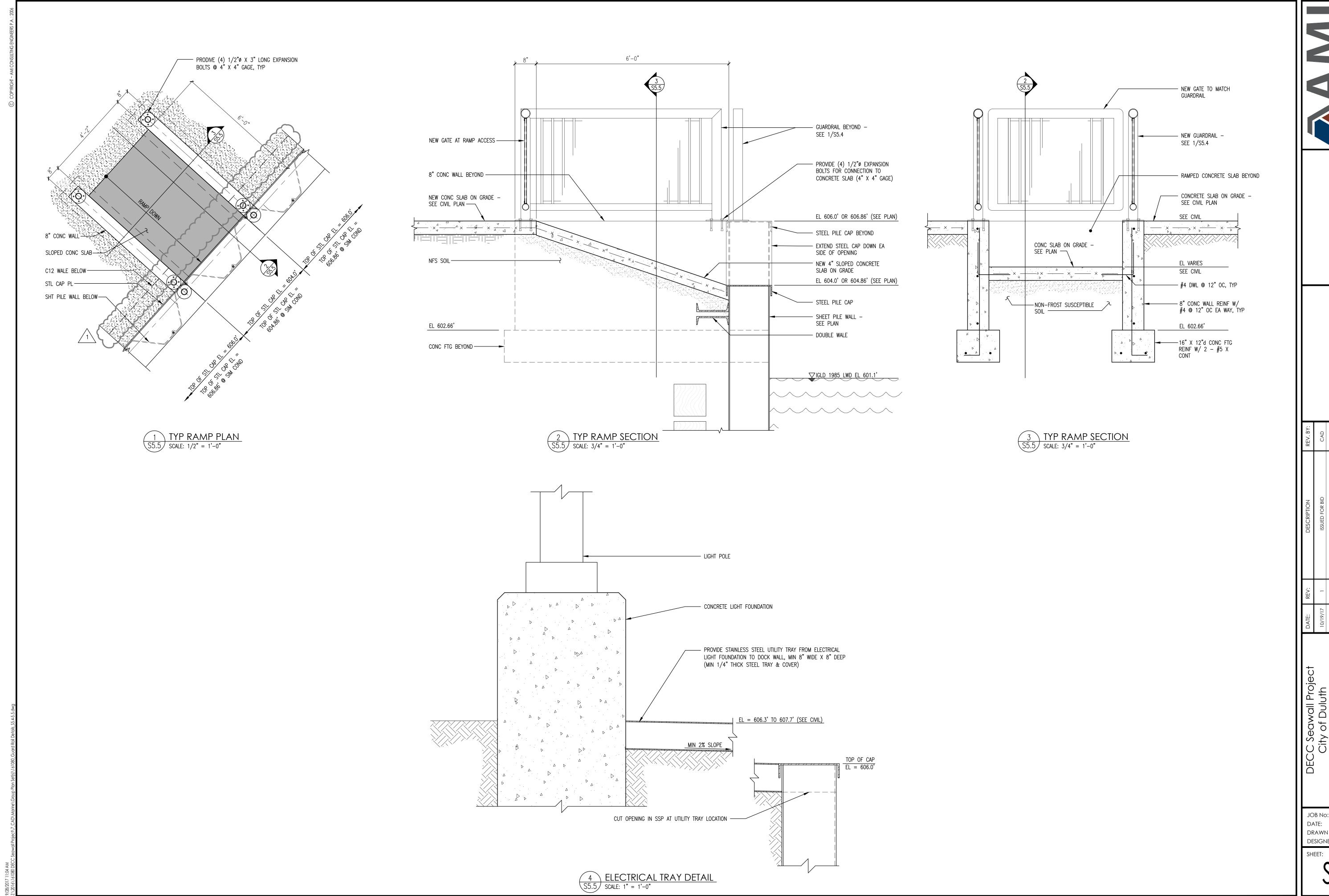
JOB NO: 161080

DATE: 10/19/17

DRAWN BY: SAJ

DESIGNED BY: CAD

C5 /



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DECC Seawall Project

City of Duluth

350 Harbor Drive

Duluth, Minnesota

General Details

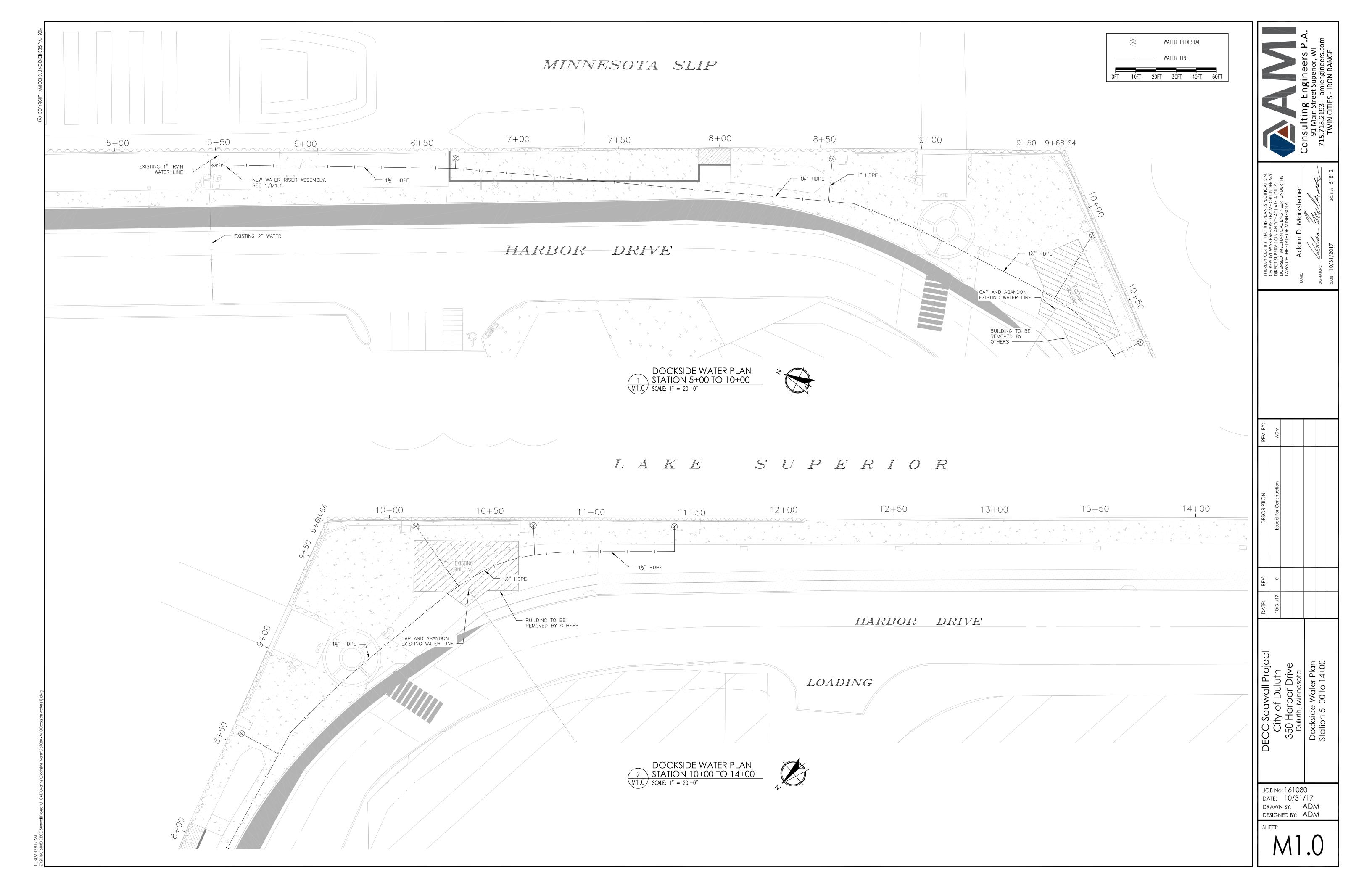
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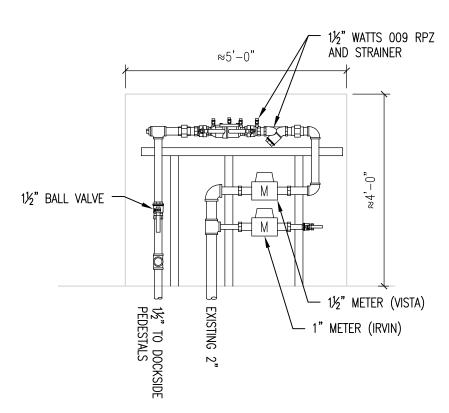
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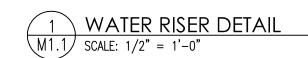
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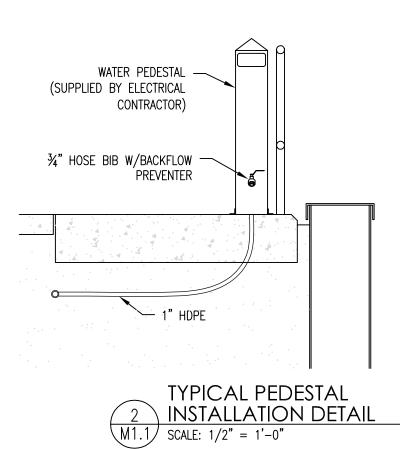
DESIGNED BY: CAD

\$5.5









GENERAL NOTES

1. CODES, RULES, AND REGULATIONS

1.1. ALL WORK SHALL BE IN STRICT ACCORDANCE WITH MN STATE BUILDING CODE, MN STATE PLUMBING CODE, AND APPLICABLE RULES, REGULATIONS OF LOCAL AND STATE GOVERNMENTS, AND UTILITY REQUIREMENTS.

1.2. DRAWINGS AND SPECIFICATIONS INDICATE MINIMUM STANDARDS OF CONSTRUCTION.1.3. CONTRACTOR SHALL COOPERATE WITH ALL LOCAL BUILDING OFFICIALS, AND UTILITIES

2. GENERAL REQUIREMENTS

2.1. CONTRACTOR SHALL COORDINATE WITH OTHER TRADES IN ORDER TO AVOID ALL INTERFERENCE.
2.2. CONTRACTOR SHALL MAKE ALL CONNECTIONS OF HIS TRADE AND SHALL CONFORM WITH, COOPERATE WITH, AND ASSIST OTHER CONTRACTORS

2.3. CONTRACTOR SHALL VERIFY ALL EXISTING SIZES, FUNCTIONS, LOCATIONS, AND CONDITIONS PRIOR TO BEGINNING WORK
2.4. CONTRACTOR SHALL NOT MODIFY STRUCTURAL COMPONENTS WITHOUT PERMISSION FROM THE ENGINEER
2.1. RUN AND ARRANGEMENT OF PIPING SHALL BE APPROXIMATELY AS INDICATED ON DRAWINGS, SUBJECT TO MODIFICATIONS AS REQUIRED. CONTRACTOR SHALL REFER TO AND CAREFULLY CHECK DRAWINGS AND

DETAILS, AND SHALL ARRANGE ITS WORK ACCORDINGLY.

2.2. MATERIALS SHALL BE NEW AND OF BEST QUALITY.

2.2. MATERIALS SHALL BE NEW AND OF BEST QUALITY.

2.3. MATERIALS, EQUIPMENT, FIXTURES, AND FITTINGS SHALL BE PROPERLY AND ADEQUATELY PROTECTED BY THE CONTRACTOR.

3. INSTALLATION

3.1. MATERIALS AND INSTALLATION TO CONFORM TO APPLICABLE STATE AND LOCAL CODES.
3.2. PIPING SHALL BE PROTECTED FROM DAMAGE DUE TO ABRASION, CONTACT WITH SHARP SURFACES, OR KINKING/PINCHING.

A DITIMPING MATERIALS

4. PLUMBING MATERIALS
4.1. ALL HDPE PIPE TO BE PE4710 IPS AND MIN. DR11
4.2. SHALL BE IN ACCORDANCE WITH STATE PLUMBING CODE

4.2. SHALL BE IN ACCORDANCE WITH STATE PLUMBING CODE
4.3. SLEEVES — PROVIDE SLEEVES FOR ALL PIPES PASSING THROUGH MASONRY, CONCRETE, FLOORS OR WALLS OF SUFFICIENT SIZE FOR PIPE AND INSULATION.

4.4. EXPANSION AND CONTRACTION — PROVIDE FOR EXPANSION AND CONTRACTION.

5. MISCELLANEOUS

5.1. CONTRACTOR SHALL TEST ENTIRE WATER SUPPLY SYSTEM PER MN PLUMBING CODE SECTION 609.4. PROVIDE RECORD OF TEST RESULTS TO OWNER AND ENGINEER.
5.2. ALL PIPING TO BE FLUSHED AND SANITIZED PER MN PLUMBING CODE SECTION 609.9. PROVIDE RECORD OF TESTING RESULTS TO OWNER AND ENGINEER.
5.3. THE CONTRACTOR SHALL GUARANTEE THAT DURING THE ONE YEAR PERIOD FOLLOWING THE COMPLETION OF THE INSTALLATION IT SHALL REPAIR ALL DEFECTIVE WORK AND REPLACE ALL DEFECTIVE MATERIALS FURNISHED OR INSTALLED UNDER THIS CONTRACT FREE OF COST TO OWNER

Consulting Engineers P.A. 91 Main Street Superior, WI 715.718.2193 - amiengineers.com TWIN CITIES - IRON RANGE

OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED MECHANICAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA

Adam D. Marksteiner

Adam D. Marksteiner

 Project
 DATE:
 REV:
 DESCRIPTION
 REV. BY:

 Jth
 10/31/17
 0
 Issued For Construction
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JOB No: 161080

DATE: 10/31/17
DRAWN BY: ADM
DESIGNED BY: ADM

M1.

NOT BE USED IN THIS SET OF DRAWINGS.



ECC SEAWALL PROJECT
CITY OF DULUTH
350 HARBOR DRIVE
DULUTH, MN

DESIGNED BY: S.HAEDTKE SHEET:

JOB No: 84877

date: 10/20/2017 drawn by: B.ANDERSON

ABANDON EXISTING IRVIN LIGHTING UNDERGROUND _

RACEWAY SYSTEM. REMOVE WIRE.

EXISTING

POWER

UNDERGROUND -

UNDERGROUND

REMOVE IRVIN LIGHTING, STREET LIGHTING, AND 5K SERVICES.

GENERAL NOTE

A. IT IS THE RESPONSIBILITY OF THIS CONTRACTOR TO PROVIDE COMPLETE DEMOLITION TO FACILITATE NEW CONSTRUCTION IN PHASES REQUIRED.FIELD VERIFY EXISTING CONDITIONS PRIOR TO BID. NO EXTRAS WILL BE ALLOWED DUE TO CONTRACTORS LACK OF KNOWLEDGE OF EXISTING CONDITIONS, SCOPE, AND/OR SCHEDULE.

KEYNOTE:

- 1. DISCONNECT AND REMOVE LIGHT FIXTURES, POWER, SECURITY, AND DATA DEVICES IN THIS AREA. COORDINATE WITH CITY OF DULUTH POLICE DEPARTMENT FOR CCTV EQUIPMENT REMOVAL AND REINSTALLATION.
- 2. EXISTING UNDERGROUND FIBER OPTIC CABLE TO REMAIN. PROTECT AS REQUIRED.
- 3. REMOVE LIGHT FIXTURE, POLE, BASE, AND BRANCH CIRCUITING COMPLETE.
- 4. REFER TO IRVIN PHASING PLAN NOTE FOR SHIP TO SHORE TEMPORARY CONNECTIONS. REMOVE EXISTING COMM. PEDESTAL AND REINSTALL AFTER SLIP WALL CONSTRUCTION IS COMPLETE.
- 5. REMOVE EXISTING LIGHTING SYSTEMS IN THIS AREA COMPLETE. SALVAGE EXISTING HOMERUN FOR CONNECTION TO NEW AS INDICATED ON SHEET E1.2.
- 6. REMOVE EXISTING LANDSCAPE, INCLUDING BUT NOT LIMITED TO TREES, SHRUBS, EXCESS DIRT, LANDSCAPE WALLS AND ELECTRICAL CONNECTIONS IN AREA TO FACILITATE CONSTRUCTION OF NEW MN. POWER SERVICE AREA.
- 7. EXISTING MARINA SLIP SERVICE TRANSFORMER AND EQUIPMENT TO REMAIN. PROTECT AS REQUIRED AND MAINTAIN SERVICE CONTINUITY.
- 8. EXISTING MN POWER SWITCHGEAR TO REMAIN. COORDINATE WITH MN POWER AND MAINTAIN PRIMARY CONTINUITY FOR SERVICES NOT AFFECTED BY DEMOLITION.
- 9. EXISTING SITE POLE WITH CITY OF DULUTH SECURITY CAMERAS ARE SERVED FROM SLIP BRIDGE POWER SYSTEM. MAINTAIN AND PROTECT UNDER BASE BID. UNDER ALTERNATE COORDINATE REMOVAL WITH INSTALLATOIN OF NEW AS INDICATED TO

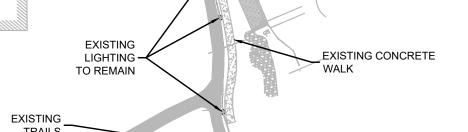
MINIMIZE DOWNTIME. MAINTAIN SHIP TO SHORE POWER, 200A, 480V, AND COMMUNICATION OVERHEAD SERVICES FROM DECC DURING BASE BID WORK. PROVIDE TEMPORARY MAST AND OVERHEAD SERVICE EXTENSION AS REQUIRED FOR TEMPORARY IRVIN MOORING ON THE EAST SIDE OF SLIP TO FACILITATE HARBOR WALL RESTORATION. RESTORE SERVICES TO AS-IS CONDITION AFTER BASE BID WORK IS COMPLETED. COORDINATE PROJECT PHASING WITH GENERAL -CONTRACTOR FOR IRVIN REMOVAL INCLUDING DISCONNECTION OF SHIP TO SHORE POWER COORDINATE WITH VERIZON AND REMOVE AND COMMUNICATION TO FACILITATE SLIP BASIN RESTORATION. RECONNECT EXISTING EXISTING DAMAGED/ABANDONED COMMUNICATION PEDESTAL. UTILITIES AFTER COMPLETION AND RETURN OF IRVIN. UNDER ALTERNATE PROVIDE NEW CONNECTIONS AS INDICATED COORDINATED WITH RETURN OF IRVIN. ABANDONED POWER PEDESTAL EXISTING
TRANSFORMER
TO REMAIN ABANDONED LIGHT POLE BASE

EXISTING
TRANSFORMER
TO REMAIN

REMOVE EXISTING ABANDONED IRVIN FLOOD LIGHTS FROM EXISTING POLE ASSEMBLIES. REMOVE WIRE COMPLETE AND ABANDON RACEWAY. VERIFY FINAL

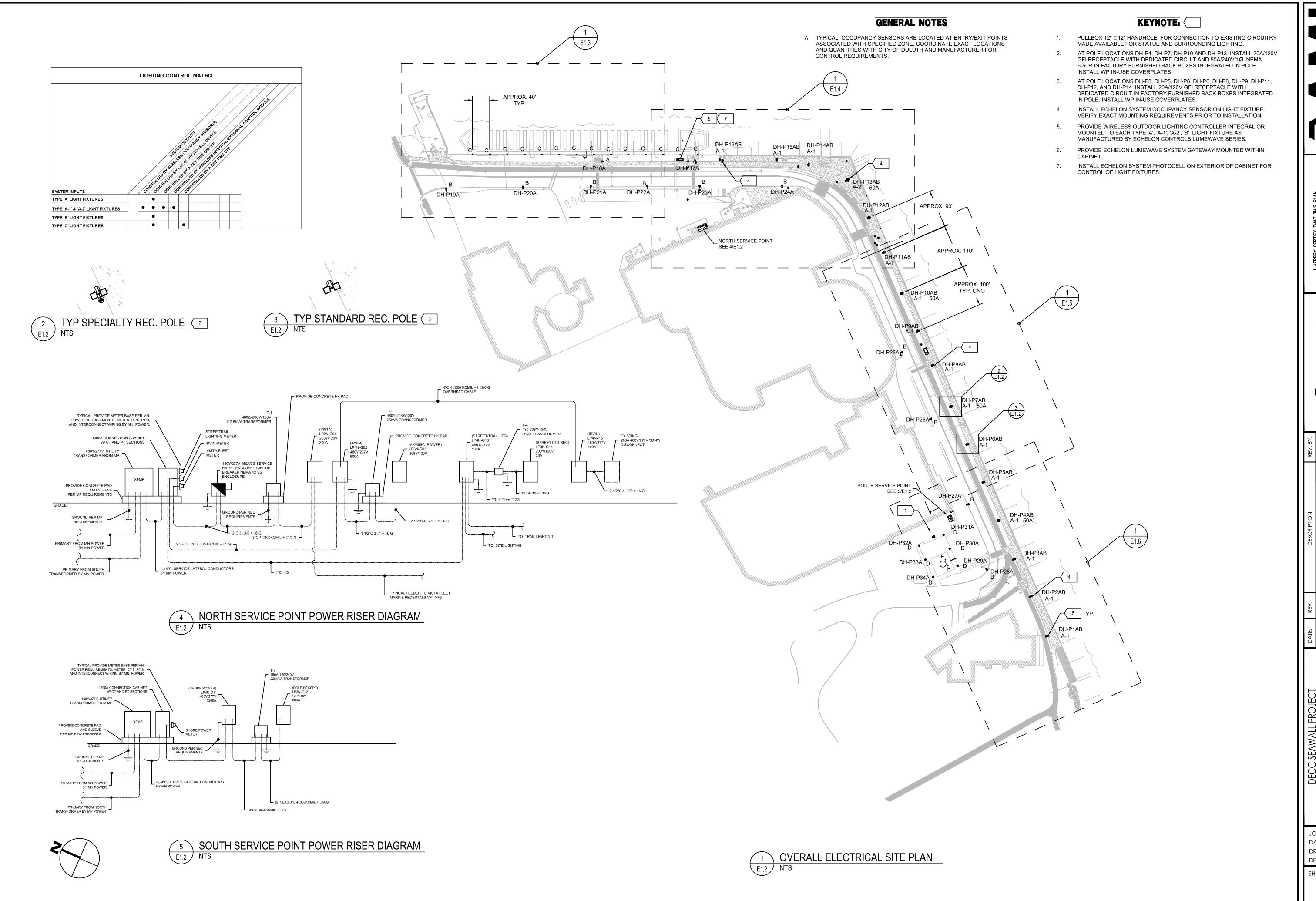
LOCATIONS PRIOR TO BID.

1 ELECTRICAL DEMOLITION PLAN
ED1.1 SCALE: 0' 80'



JOB No: 84877 DATE: 10/20/2017 DRAWN BY: B.ANDERSON DESIGNED BY: S.HAEDTKE





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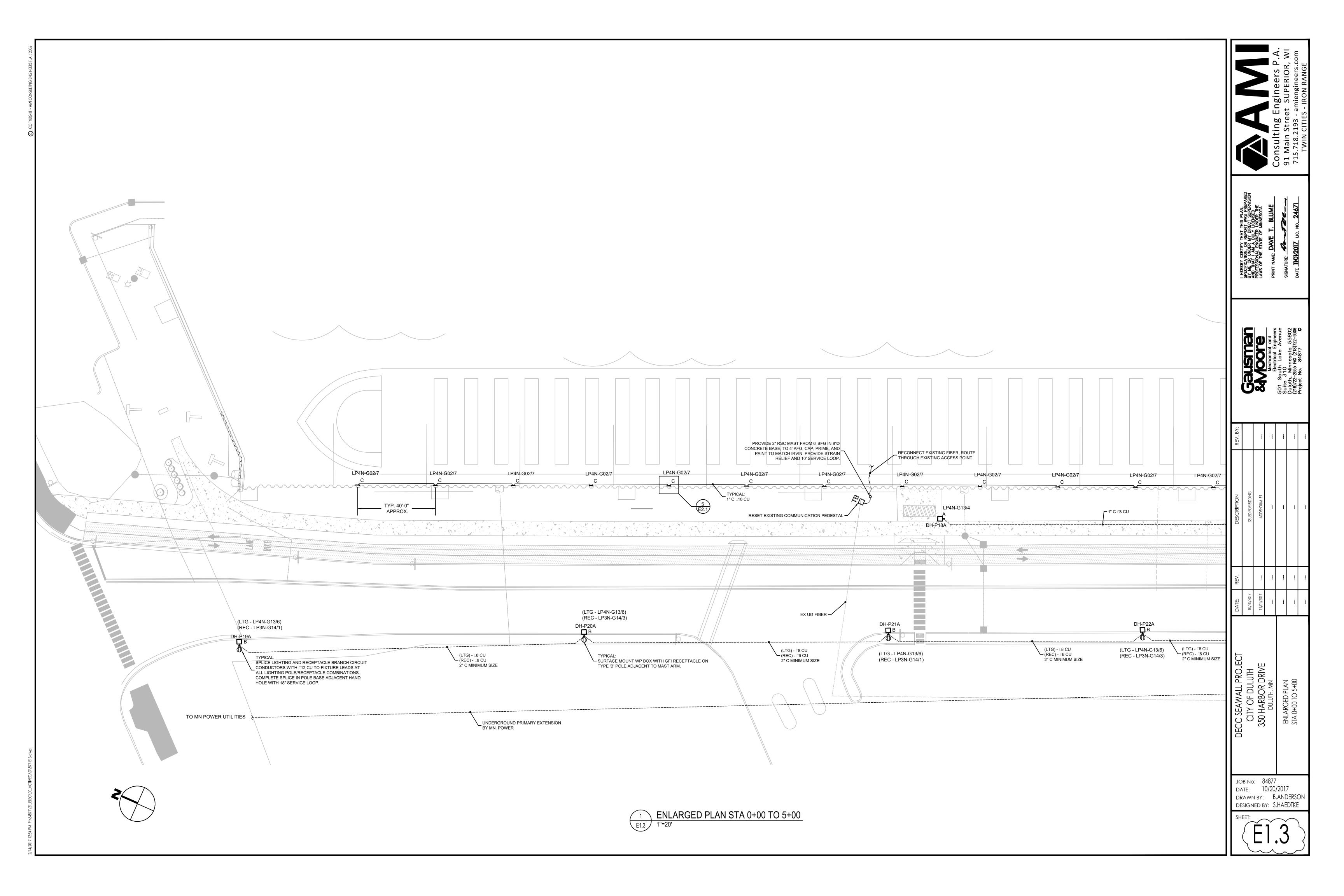
PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA PRINT NAME: DAVE T. BLUME
SIGNATURE: DAVE T. NO. 24671

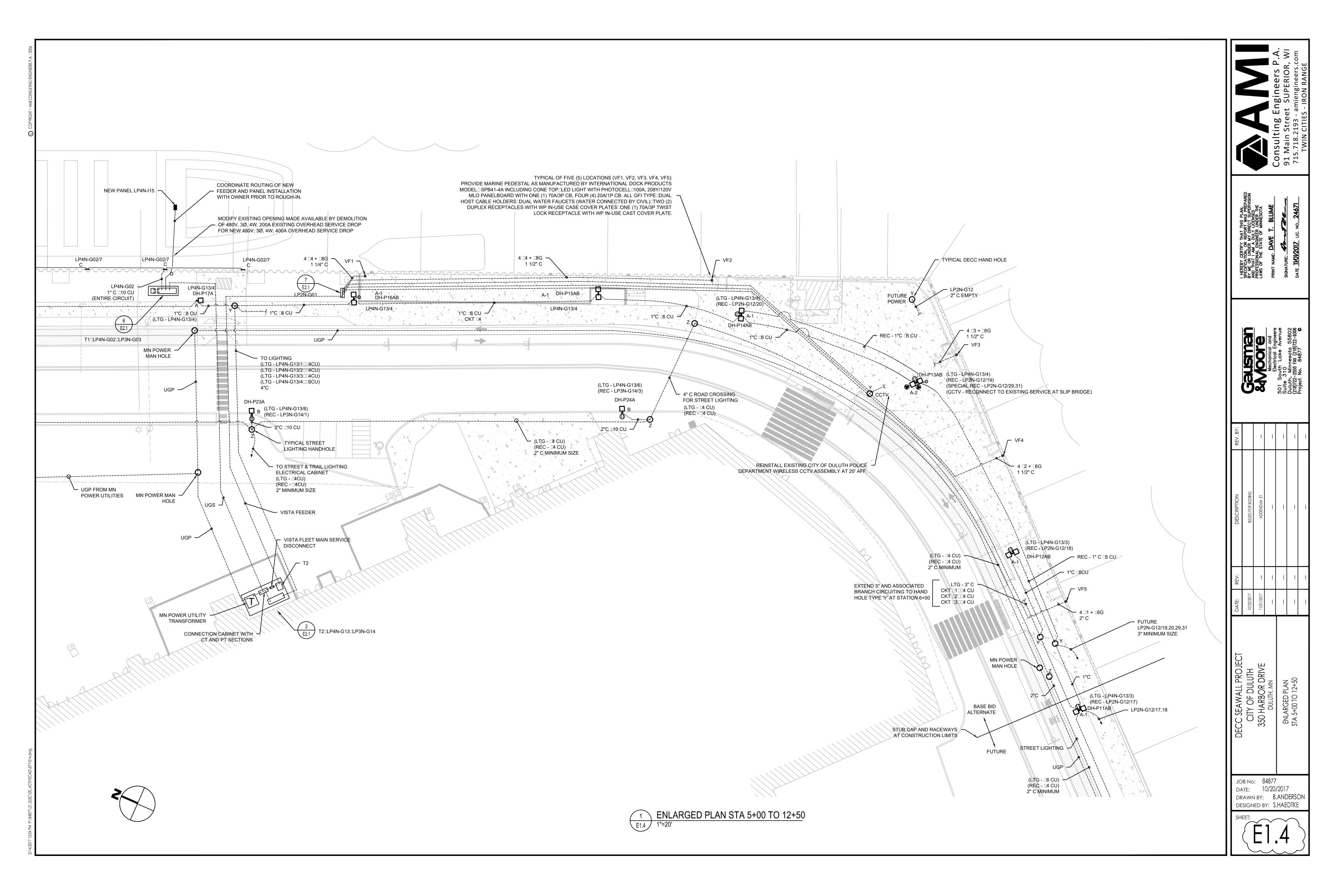
ACCHAINCE AND LANGE SUITE 310 DUILLY, MINNESOLA 55802 (218)22-2555 FIX (218)72-3306 PD 2555 NX (218)72-3555 PD 2555 NX (218)72-3555 PD 2555 NX (218)72-355 PD 2555 NX (218)72-355 PD 2555 NX (218)72-3

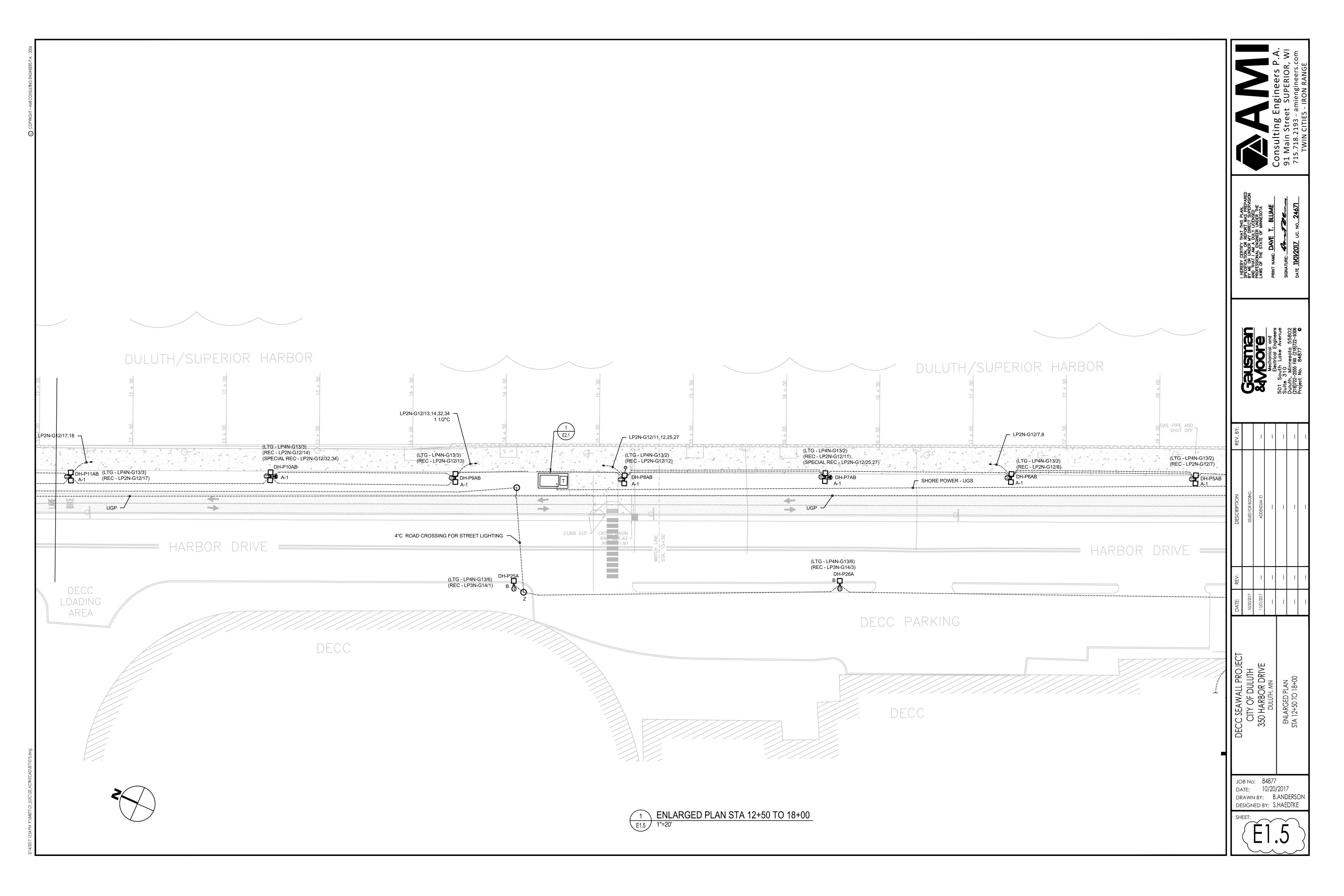
SECC SEAWALL PROJECTDATE:REV:DESCRIPTIONCITY OF DULUTH10/20/2017---ISSUED FOR BIDDING350 HARBOR DRIVE---------DULUTH, MN---------ERALL ELECTRICAL SITE PLAN---------

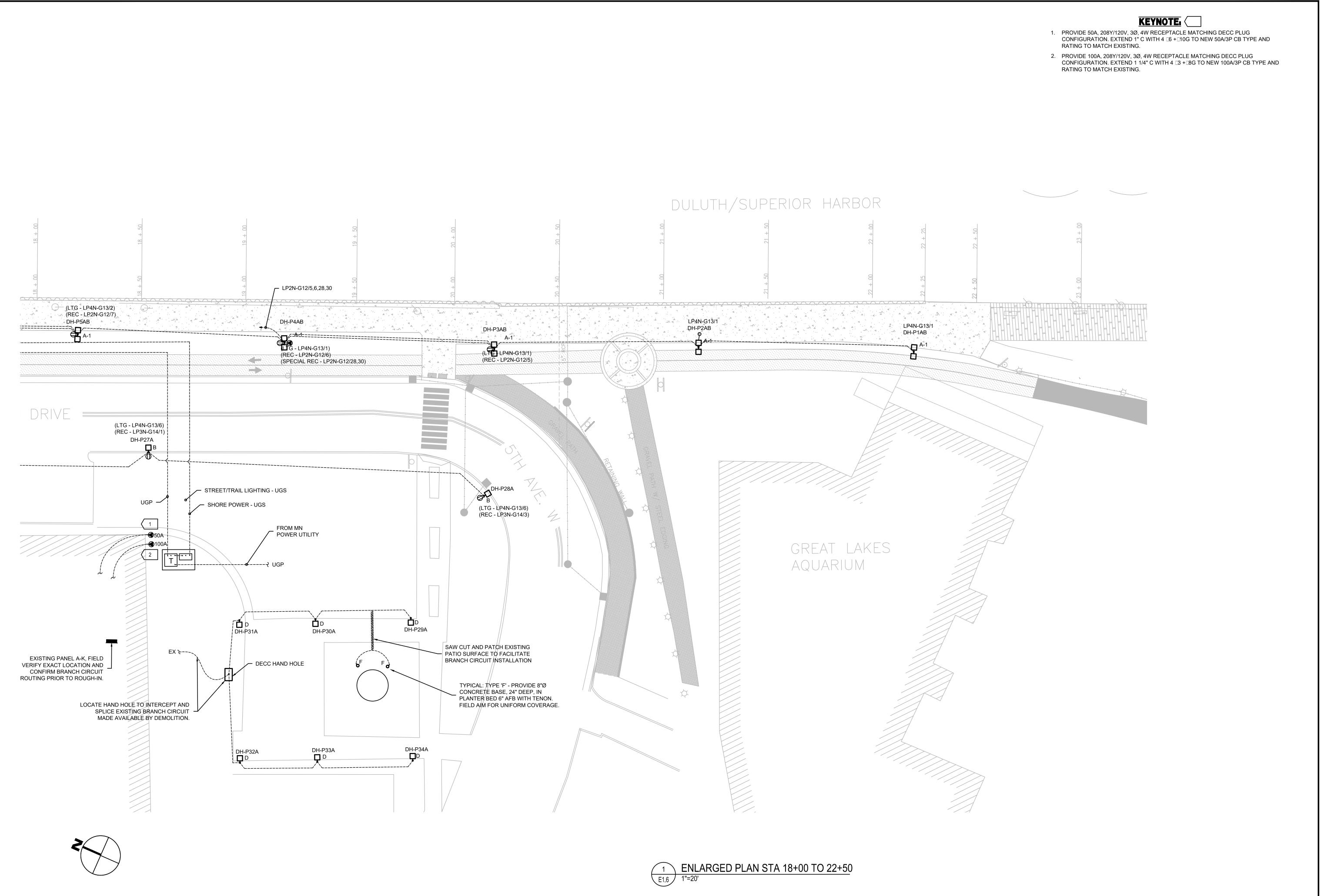
Job no: 84877 date: 10/20/2017 drawn by: B.ANDERSON designed by: S.HAEDTKE

E1.2









Sulting Engineers P.A.

NT NAME: DAVE T. BLUME

NATURE: AND OIZ TO NO 2467

Mechanical and Electrical Engineers 501 South Lake Avenue 5uite 310 Juluth, Minnesota 55802 218)722-2555 FXX (218)722-2555 FXX (218)722-3306 Project No. 84877

 DECC SEAWALL PROJECT
 DATE:
 REV:
 DESCRIPTION

 CITY OF DULUTH
 10/20/2017
 —
 ADDENDING

 350 HARBOR DRIVE
 —
 —
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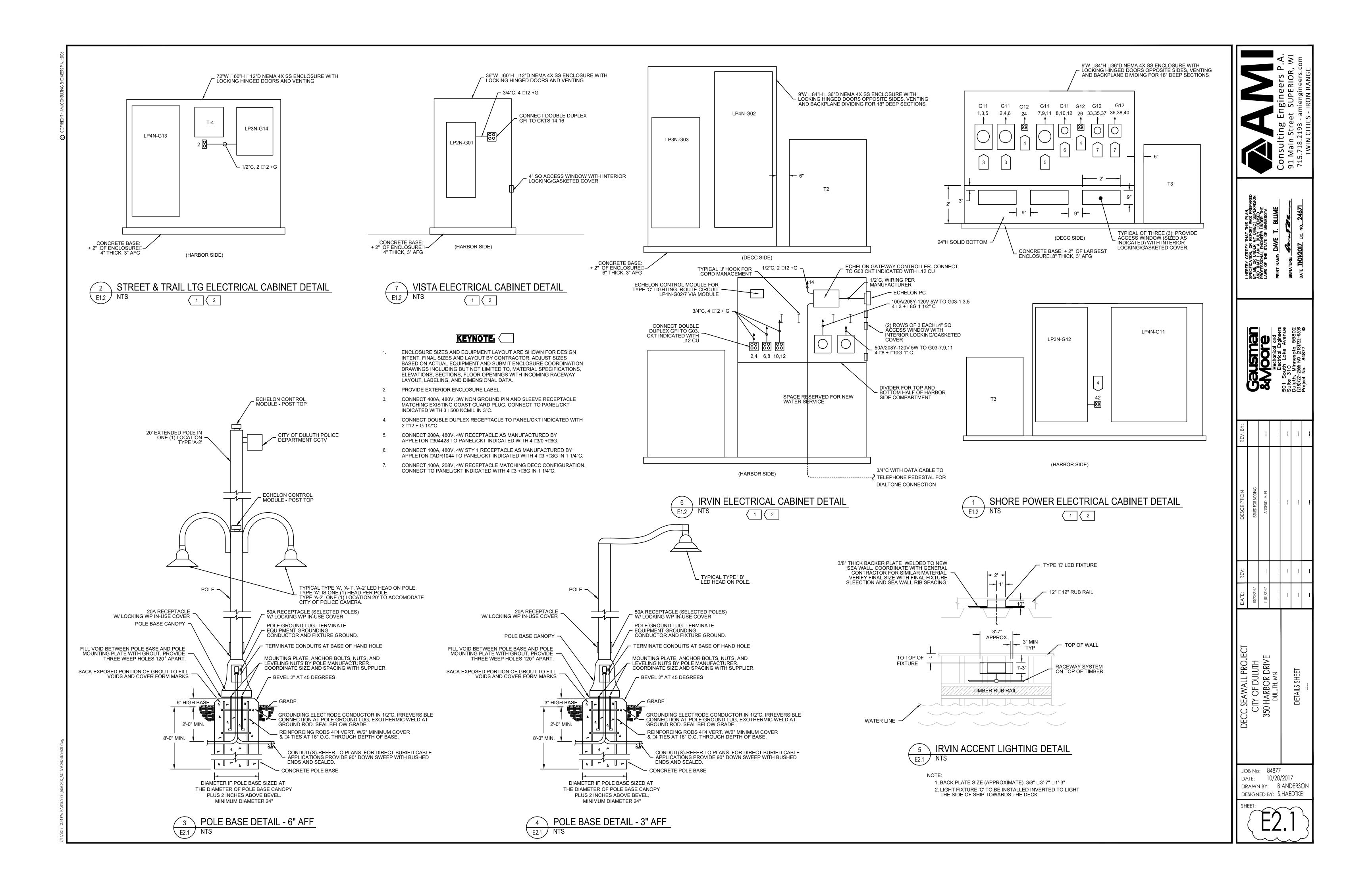
 DULUTH, MN
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 ENLARGED PLAN
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 —

 STA 18+00 TO 22+50
 —
 —
 —

JOB no: 84877 Date: 10/20/2017 Drawn by: B.ANDERSON DESIGNED by: S.HAEDTKE





								LP2N	I-G01								
P/	ANEL NAME:	LP2N-G01		VOLT:	208Y/120								BUS SIZE:	400 AMPS	REMARKS		
	LOCATION:	EXTERIOR		PH:	3							P	IAIN BRKR:	300 AMPS			
	MOUNTING:	SURFACE		WIRE:	4	V.A.	V.A.	V.A.	V.A.	V.A.	V.A.		GND BUS:	YES	GFI:	NO	
RM. NO.	LOAD DESC	RIPTION	TYPE	BRKR	ССТ							ССТ	BRKR	TYPE	LOAD DES	CRIPTION	RM. NO.
VF1	VESSEL PE	DESTAL			1	7766			7766			2			VESSEL PI	EDESTAL	VF2
				70/3	3		7766			7766		4	70/3				
				1 [5			7766			7766	6					
VF3	VESSEL PE	DESTAL			7	7766			7766			8			VESSEL PI	EDESTAL	VF4
				70/3	9		7766			7766		10	70/3				
				1	11			7766			7766	12					
VF5	FUTURE				13	10.00.10.10.10.10.10.10.11			1920			14	20/1	R	RECEPTA	DLE	CAB
				70/3	15					1920		16	20/1	R	RECEPTAG	DLE	CAB
				1	17							18			SPACE		
	SPACE				19							20			SPACE		
	SPACE				21							22			SPACE		
	SPACE				23							24			SPACE		
						PER PHAS	SE TOTAL V	/A	32984	32984	31064					11/1/2	017
						TOTAL CO	NNECTED	VA		97032	V.A.						
						TOTAL CO	NNECTED	AMPS		269	AMPS						
						3 x HIGHE	ST PHASE			98952	V.A						
						HIGHEST	AMPS			275	AMPS						

								LP4N	I-G11								
P	ANEL NAME:	LP4N-G11		VOLT:	480Y/277								BUS SIZE:	1200 AMPS	REMARKS:		
	LOCATION:	EXTERIOR		PH:	3							N	IAIN BRKR:	1200 AMPS		SERVICE RATE	:D
	MOUNTING:	SURFACE		WIRE:	4	V.A.	V.A.	V.A.	V.A.	V.A.	V.A.		GND BUS:	YES	GFI:	YES	
RM. NO.	LOAD DESC	RIPTION	TYPE	BRKR	CCT							CCT	BRKR	TYPE	LOAD DESC	CRIPTION	RM. N
	ALDER/MAC	KINAW			1	88640			88640			2			MACKINAV	//ALDER	
	-			*400/3	3		88640			88640		4	*400/3		-		
	-			1	5			88640			88640	6			-		
	LAKE GUAR	DIAN			7	44320			22160			8			R/V/KIYI		
	-			*200/3	9		44320			22160		10	*100/3		-		1
	-			1	11			44320			22160	12			-		
	XFRMR T-3		С		13	88640						14			SPARE		
	(LP3N-G12)	ı		400/3	15		88640					16	100/3		-		
	-			1	17			88640				18			-		
	SPACE				19							20			SPACE		
	SPACE				21							22			SPACE		
	SPACE				23							24			SPACE		
	SPACE				25							26			SPACE		
	SPACE				27							28			SPACE		
						PER PHAS	SE TOTAL V	/A	332400	332400	332400					11/1/2	.017
						TOTAL CO	NNECTED	VA		997200	V.A.						
						TOTAL CONNECTED AMPS			1199	AMPS							
						3 x HIGHE	ST PHASE			997200	V.A.						
						HIGHEST	AMPS			1199	AMPS						1

								LP3N	I-G14								
P/	NEL NAME:	LP3N-G14		VOLT:	208Y/120								BUS SIZE:	60 AMPS	REMARKS		
	LOCATION:	EXTERIOR		PH:	3							N	/IAIN BRKR:	20 AMPS			
	MOUNTING:	SURFACE		WIRE:	4	V.A.	V.A.	V.A.	V.A.	V.A.	V.A.		GND BUS:	YES	GFI:	GFI: NO	
RM. NO.	LOAD DESC	RIPTION	TYPE	BRKR	ССТ							ССТ	BRKR	TYPE	LOAD DESC	CRIPTION	RM. NO
ODD	STREET RE	C ODD POLES	R	20/1	1	1500			180			2	20/1	R	CAB REC		
EVEN	STREET RE	C EVEN POLES	R	20/1	3		1500			180		4	20/1	С	ECHELON	CONTROLS	
	SPACE				5							6			SPACE		
	SPACE				7							8			SPACE		
	SPACE				9							10			SPACE		
	SPACE				11							12			SPACE		
						PER PHAS	PER PHASE TOTAL VA		1680	1680	0					11/1/20	.017
						TOTAL CO	TOTAL CONNECTED VA			3360	V.A.						
						TOTAL CONNECTED AMPS			9	AMPS							
						3 x HIGHE	ST PHASE			5040	V.A.						
						HIGHEST.	AMPS			14	AMPS						

						LP4	N-I15								
PANEL NAME:	LP4N-l15	VO	T: 480Y/277								BUS SIZE:	400 AMPS	REMARKS	s:	
LOCATION:	IRVIN	ı	Н: 3								MAIN BRKR:	400 AMPS		SERVICE RATE	D
MOUNTING:	SURFACE	WI	RE: 4	V.A.	V.A.	V.A.	V.A.	V.A.	V.A.		GND BUS:	YES	GFI	: NO	\top
RM. NO. LOAD DESC	RIPTION	TYPE BRK	R CCT							ССТ	BRKR	TYPE	LOAD DES	CRIPTION	RM. N
BACKFEED	TO EXISTING		1	55460						2			SPARE		1
-		200	3 3		55460					4	20/3		-		
-			5			55460				6			-		T
SPACE			7							8			SPARE		
SPACE			9							10	50/3		-		
SPACE			11							12			-		
SPACE			13							14	20/1		SPARE		
SPACE			15							16	20/1		SPARE		
SPACE			17							18	20/1		SPARE		
SPACE			19							20	20/1		SPARE		
SPACE			21							22	20/1		SPARE		
SPACE			23							24	20/1		SPARE		
SPACE			25							26			SPACE		
SPACE			27							28			SPACE		
SPACE			29							30			SPACE		
SPACE			31							32			SPACE		
SPACE			33							34			SPACE		
SPACE			35							36			SPACE		
SPACE			37							38			SPACE		
SPACE			39							40			SPACE		
SPACE			41							42			SPACE		
				PER PHAS			55460	55460	55460					11/1/20)17
					NNECTED			166380	V.A.						-
					NNECTED			200	AMPS						-
				HIGHEST	ST PHASE			166380 200	V.A. AMPS						+

								LP4N	I-G02								
P	ANEL NAME:	LP4N-G02		VOLT:	480Y/277								BUS SIZE:	600 AMPS	REMARKS	:	
	LOCATION:	EXTERIOR		PH:	3							М	AIN BRKR:	600 AMPS		SERVICE RATED)
	MOUNTING:	SURFACE		WIRE:	4	V.A.	V.A.	V.A.	V.A.	V.A.	V.A.		GND BUS:	YES	GFI	: NO	
RM. NO.	LOAD DESC	RIPTION	TYPE	BRKR	ССТ							ССТ	BRKR	TYPE	LOAD DES	CRIPTION	RM. NO.
	XFMR T-2		С		1	27700			88640			2		NC	IRVIN		
	(LP3N-G03))		125/3	3		27700			88640		4	*400/3				
	-			1	5			27700			88640	6	1				
	IRVIN HULL	. LIGHTING	L	*20/1	7	756						8			SPARE		
	SPARE			20/1	9							10	100/3				
	SPARE			20/1	11							12	1				
	SPACE				13							14			SPACE		
	SPACE				15							16			SPACE		
	SPACE				17							18			SPACE		
	SPACE				19							20			SPACE		
	SPACE				21							22			SPACE		
	SPACE				23							24			SPACE		
	SPACE				25							26			SPACE		
	SPACE				27							28			SPACE		
	* GFCI TYPI	ECB				PER PHAS	SETOTAL V	Ά	117096	116340	116340					11/1/20	17
						TOTAL CO	NNECTED	VA		349776	V.A.						
						TOTAL CO	NNECTED	AMPS		421	AMPS						
						3 x HIGHE	ST PHASE			351288	V.A.						
						HIGHEST	AMPS			423	AMPS						

								LP2N	I-G12							
P/	ANEL NAME:	LP2N-G12		VOLT:	120/240								BUS SIZE:	600 AMPS	REMARKS:	
	LOCATION:	EXTERIOR		PH:	3							ı	MAIN BRKR:	MLO	HIGH LEG SYS	TEM
	MOUNTING:	SURFACE		WIRE:	4	V.A.	V.A.	V.A.	V.A.	V.A.	V.A.		GND BUS:	YES	GFI: NO	
RM. NO.	LOAD DESC	RIPTION	TYPE	BRKR	CCT							CCT	BRKR	TYPE	LOAD DESCRIPTION	RM. NO.
P1	RECEPTAC	LE	R	20/1	1	1920			1920			2	20/1	R	RECEPTACLE	P2
	HIGH LEG E	O NOT USE			3							4	-		HIGH LEG DO NOT USE	
P3	RECEPTAC	LE	R	20/1	5			1920			1920	6	20/1	R	RECEPTACLE	P4
P5	RECEPTAC	LE	R	20/1	7	1920			1920			8	20/1	R	RECEPTACLE	P6
	HIGH LEG E	O NOT USE			9							10	-		HIGH LEG DO NOT USE	
P7	RECEPTAC	LE	R	20/1	11			1920			1920	12	20/1	R	RECEPTACLE	P8
P9	RECEPTAC	LE	R	20/1	13	1920			1920			14	20/1	R	RECEPTACLE	P10
	HIGH LEG E	O NOT USE			15							16	-		HIGH LEG DO NOT USE	
P11	RECEPTAC	LE	R	20/1	17			1920			1920	18	20/1	R	RECEPTACLE	P12
P13	RECEPTAC	LE	R	20/1	19	1920			1920			20	20/1	R	RECEPTACLE	P14
	HIGH LEG E	O NOT USE			21							22	-		HIGH LEG DO NOT USE	
P13	DPD CCTV		R	20/1	23			800			180	24	20/1	R	HARBOR S. CABINET	REC
P7	RECEPTAC	LE	R	*50/2	25	6000			180			26	20/1	R	HARBOR S. CABINET	REC
	-			00,2	27		6000			6000		28	*50/2	R	RECEPTACLE	P4
P13	RECEPTAC	LE	R	*50/2	29			6000			6000	30	00/2		-	
	-			00/2	31	6000			6000			32	*50/2	R	RECEPTACLE	P10
	RECEPTAC	LE	R		33		11085			6000		34	00/2		-	
	-			100/3	35			11085			11085	36		R	RECEPTACLE	
					37	11085			11085			38	100/3		-	
	SPARE			20/1	39					11085		40			-	
	SPARE			20/1	41						180	42	20/1	R	DECC S. CAB	REC
	*GFCITYPE	СВ				PER PHAS	SE TOTAL V	/A	55710	40170	46850				11/1/2	2017
						TOTAL CO	NNECTED	VA		142730	V.A.					
						TOTAL CO	NNECTED	AMPS		343	AMPS					
						3 x HIGHE	ST PHASE			167130	V.A.					
						HIGHEST	AMPS			402	AMPS					

							LP3N	I-G03								
PANEL NAME:	LP3N-G03		VOLT:	208Y/120								BUS SIZE:	200 AMPS	REMARKS	:	
LOCATION:	EXTERIOR		PH:	3							ı	MAIN BRKR:	200 AMPS			
MOUNTING:	SURFACE		WIRE:	4	V.A.	V.A.	V.A.	V.A.	V.A.	V.A.		GND BUS:	YES	GFI	NO	
RM. NO. LOAD DESCR	RIPTION	TYPE	BRKR	CCT							CCT	BRKR	TYPE	LOAD DES	CRIPTION	RM. NO.
PEDESTAL R	RECEPTACLE	R		1	9627			1920			2	20/1	R	PEDESTA	RECEPTACLE	
-			*100/3	3		9627			1920		4	20/1	R	PEDESTA	RECEPTACLE	
-				5			9627			1920	6	20/1	R	PEDESTA	RECEPTACLE	
PEDESTAL R	RECEPTACLE			7	4814			1920			8	20/1	R	PEDESTA	RECEPTACLE	
-			*50/3	9		4814			1920		10	20/1	R	PEDESTA	RECEPTACLE	
-				11			4814			1920	12	20/1	R	PEDESTA	RECEPTACLE	
SPARE				13				200			14	20/1		ECHELON	CONTROLS	
-			*50/3	15							16	20/1		SPARE		
-				17							18	20/1		SPARE		
SPACE				19							20	20/1		SPARE		
SPACE				21							22	20/1		SPARE		
SPACE				23							24	20/1		SPARE		
SPACE				25							26			SPACE		
SPACE				27							28			SPACE		
SPACE				29							30			SPACE		
SPACE				31							32			SPACE		
SPACE				33		2010 10 10 10 10 10 10 10 10 10 10 10 10					34			SPACE		
SPACE				35							36			SPACE		
SPACE				37							38			SPACE		
SPACE				39							40			SPACE		
SPACE				41							42			SPACE		
*GFCITYPE	СВ				PER PHAS	ETOTAL V	4	18481	18281	18281					11/1/20	17
					TOTAL CO	NNECTED	VA		55043	V.A.						
					TOTAL CO	NNECTED	AMPS		153	AMPS						
					3 x HIGHE	ST PHASE			55443	V.A.						
					HIGHEST.	AMPS			154	AMPS						

								LP4N	N-G13								
P#	ANEL NAME:	LP4N-G13		VOLT:	480Y/277								BUS SIZE:	100 AMPS	REMARKS:		
	LOCATION:	EXTERIOR		PH:	3						M	AIN BRKR:	100 AMPS		SERVICE RATED		
	MOUNTING:	SURFACE		WIRE:	4	V.A.	V.A.	V.A.	V.A.	V.A.	V.A.		GND BUS:	YES	GFI:	NO	
RM. NO.	LOAD DESC	RIPTION	TYPE	BRKR	CCT							CCT	BRKR	TYPE	LOAD DESC	CRIPTION	RM. NO.
	LIGHTING, F	P1-P4	L	20/1	1	784			784			2	20/1	L	LIGHTING,	P5-P8	
	LIGHTING, F	P9-P12	L	20/1	3		784			980		4	20/1	L	LIGHTING,	P13-P18	
	XFRMR T-4		С		5			3324			196	6	20/1	L	STREET LI	GHTING, P19-P28	8
	(LP1N-G14)			15/3	7	3324			580			8	20/1		SPARE		
					9		3324					10	20/1		SPARE		
	SPARE			20/1	11							12	20/1		SPARE		
	SPARE			20/1	13							14	20/1		SPARE		
	SPARE			20/1	15							16	20/1		SPARE		
	SPARE			20/1	17							18	20/1		SPARE		
	SPACE				19							20			SPACE		
	SPACE				21							22			SPACE		
	SPACE				23							24			SPACE		
						DED DUA	SE TOTAL V	Δ.	5472	5088	3520					11/1/20	17
							NNECTED		3472	14080	3320 V.A.					1171720	17
							NNECTED			14080	V.A. AMPS						
							ST PHASE	AIVIPS		16416							
						HIGHEST				20	AMPS						

HAND HOLE SCHEDULE										
TYPE	SIZE									
TIFE	W	L	D							
Z	12"	24"	24"							
X	PER CITY OF DULUTH/MNDOT STANI									
Y	24"	36"	36"							
W	12"	12"	12"							
N	OTE: REFER TO	SPECIFICATION	NS .							

		LUMINAIRE SCHEDULE						
				LAMP				
TYPE	MANUFACTURER	MODEL	VOLT	NO.	BALLAST	LUMINAIRE DESCRIPTION		
	WANT ACTORER	MODEL	VOL!	TYPE	_ DALLAGI	EOMINAINE DEGORIT HON		
				WATTAGE				
		GPLF2P2030KAS4RAL5015L2 PDA16S5J18P09RAL5015(2)RXXXY (2)SXXXY 1BC90R15FRAL5015-QSM (2)BA24B0H4RAL5015		1		GLASWERKS PRAGUE LED - FLAT GLASS, 3K, TYPE 2 DIST		
Α	GLASWERKS	FGIUS FG-50A FD-BLNKPLT AB RFD246835	277	LED		INTEGRAL PHOTOCELL		
	(NOTE 3,4)			98		SINGLE LAMP, DUAL BANNER, POWER SHEPHARD HOOK		
		CEMENT POLE BASE: REFER TO DETAIL 3/E2.1						
		GPLF2P2040KAS4RAL5015L2 PDA16S5J18P09RAL5015(2)RXXXY (2)SXXXY 2BC180R15CRAL5015-QSM (2)BA24BOH4RAL5015		1		GLASWERKS PRAGUE LED - FLAT GLASS, 3K, TYPE 2 DIST		
A-1 G	GLASWERKS	FGIUS FG-50A FD-BLNKPLT AB RFD239973	277	LED	DRIVER			
	(NOTE 3,4)			98		DUAL LAMP, DUAL BANNER, POWER SHEPHARD HOOK		
		CEMENT POLE BASE: REFER TO DETAIL 3/E2.1						
A-2		GPLF2P2030KAS4RAL5015L2 PDA16S5J18P09RAL5015(2)RXXXY (2)SXXXY 1BC90R15FRAL5015-QSM (2)BA24B0H4RAL5015		1		GLASWERKS PRAGUE LED - FLAT GLASS, 3K, TYPE 2 DIST WITH		
	GLASWERKS	FGIUS FG-50A FD-BLNKPLT AB SXXXY	277	LED	DRIVER	CCTV MOUNTED AT TOP OF POLE		
	(NOTE 3,4)			98	1	DUAL LAMP, DUAL BANNER, POWER SHEPHARD HOOK		
		CEMENT POLE BASE: REFER TO DETAIL 3/E2.1			1			
		GBLF2 P20 40K AS 4 B L2 BHDF13 200 BK		1		GLASWERKS LED BERN - FLAT GLASS, 3K, TYPE 2 DIST		
В	GLASWERKS	KWA22L5J20P08ABGXX AB-31-4 FGIUS FG-20A RXXXY RFD225830	277	LED	DRIVER	INTEGRAL PHOTOCELL		
	(NOTE 3,4)	BHC48/1MODU8698 RFD246830		58	1	SINGLE LAMP, DUAL BANNER, NO POWER, BOSTON HARBOR		
		CEMENT POLE BASE: REFER TO DETAIL 3/E2.1			1			
		SVLED2 PNL PK1 MVOLT 40K AMT BKSDP AO		1		SIGN VUE LED II, PNL OPTIC, PK1 4K PANEL MOUNT		
С	HALOPHANE		277	LED	DRIVER			
				54				
		PUCL2035HO4KASXXL2S PDA08S4C18P07XX AB-31-4 RFD239974		1		POST TOP AREA LIGHTING LED FULL CUT OFF		
D	HALOPHANE		277	LED	DRIVER			
	(NOTE 2)			45		SINGLE LAMP, DUAL BANNER, NO POWER		
		CEMENT POLE BASE: REFER TO DETAIL 4/E2.1						
		PSLED PK1 MVOLT NSP 40K 1 XXSDP 10KVMP NR		1		UP LIGHTING FOR STATUE		
F	HALOPHANE		MVOLT	LED	DRIVER			
	(NOTE 1)							

NOTE:

- 1. FIELD VERIFY EXISTING CIRCUIT VOLTAGE AND ADJUST DRIVER TAP SETTING RESPECTIVELY.
- TYPE D FIXTURES TO USE POLE BASE 14"Ø, 8' DEEP AND 3" ABOVE FINISHED GRADE.
 FIXTURE TYPES 'A','A-1','A-2' AND 'B' FURNISH WITH WIRELESS LIGHTING CONTROLS MATCHING CITY OF
- DULUTH REQUREMENTS AS MANUFACTURE BY ECHELON.
- 4. REFER TO SPECIFICATION SECTION 260943 NETWORK LIGHTING CONTROLS. PROVIDE 7-PIN RECEPTACLE TO FACILITATE INSTALLATION OF WIRELESS CONTROL MODULE AS MANUFACTURED BY ECHELON. AT CONTRACTOR AND MANUFACTURES OPTION THE 7-PIN RECEPTACLE MAY BE REPLACED WITH BASE MOUNTED CONTROL MODULE CPD3000 AS MANUFACTURED BY ECHELON.

AM	Consulting Engineers P.A. 91 Main Street SUPERIOR, WI 715.718.2193 - amiengineers.com
	Consultir 91 Main St 715.718.21

NT NAME: DAVE T. BLUME

NATURE: A CONTROL OF NO 24671

Mechanical and Electrical Engineers 21 South Lake Avenue 21te 310 21uth, Minnesota 55802 8)722-2555 FX (218)722-9306

 DECC SEAWALL PROJECT
 DATE:
 REV:
 DESCRIPTION
 REV

 CITY OF DULUTH
 10/20/2017
 -- LISSUED FOR BIDDING
 --

 350 HARBOR DRIVE DULUTH, MN
 - -- -- --

 SCHEDULES
 - - -- --

 SCHEDULES
 - -- -- --

JOB NO: 84877

DATE: 10/20/2017

DRAWN BY: B.ANDERSON

DESIGNED BY: S.HAEDTKE



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